











For nearly 60 years, Simpson Strong-Tie has steadfastly served the needs of its customers. During that time, our commitment to leading the construction industry in research and development, product testing and customer service has never wavered.

Today, that promise continues to be delivered worldwide – from our manufacturing facilities to the jobsite. Our commitment to maintaining the highest standards of quality and service can be seen when we help educate engineers on applications and specifications, or when we answer questions from contractors or building officials or help train them on a proper installation. We're there to ensure our dealers get their questions answered and to help their customers receive the best service possible.

You can expect Simpson Strong-Tie to be there to assist you. For our more than 2,000 employees, it's not about doing business, it's about our commitment to your success.



.TRICONTRUSS.CA

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BETTER BUILDING THROUGH EDUCATION



Simpson Strong-Tie is committed to training customers on the proper specification, installation and inspection of structural system solutions.

As part of this commitment, our regional training centres offer a selection of seminars for engineers, architects, dealers, contractors and inspectors. These dedicated training facilities offer opportunities for classroom instruction as well as chances for hands-on installation of Simpson Strong-Tie® products. Participants can earn professional development hours (PDH) through our registration with CSI, SEA, ICC, BIA, AIBO, ACIA and AIBD.

Simpson Strong-Tie is now a provider of IACET CEUs. We offer continuing education units to workshop participants that meet requirements.

To locate the Simpson Strong-Tie training centre nearest you and to obtain a schedule of seminars, call (800) 999-5099 o visit www.strongtie.com.

INTRODUCTION



22-27

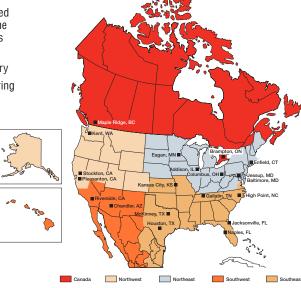
For nearly 60 years, Simpson Strong-Tie has focused on creating structural products that help people build safer and stronger homes and buildings. A leader in structural systems research and technology, Simpson Strong-Tie is one of the largest suppliers of structural building products in the world. The Simpson Strong-Tie commitment to product development, engineering, testing and training is evident in the consistent quality and delivery of its products and services. Simpson Strong-Tie® product lines include:

- Structural connectors for wood and cold-formed-steel construction
- · Strong-Wall® prefabricated shearwalls
- Strong Frame® moment frames
- Anchor Tiedown Systems (ATS) for multi-storey buildings
- Simpson Strong-Tie® anchors and fasteners for concrete and masonry
- · Simpson Strong-Tie Fastening Systems
- Connectors for Cold-Formed Steel Curtainwalls

For more information, visit the company's Web site at www.strongtie.com.

The Simpson Strong-Tie Company Inc. "No Equal" pledge includes:

- Quality products value-engineered for the lowest installed cost at the highest-rated performance levels
- Most thoroughly tested and evaluated products in the industry
- Strategically located manufacturing and warehouse facilities
- National code agency listings
- Largest number of patented connectors in the industry
- European locations with an international sales team
- In-house R&D, and tool and die professionals
- In-house product testing and quality control engineers
- Member of WWTA, OWTFA, QWTFA, AWTFA, WRLA, LBMAO, ABSDA, TPIC, PEO.



THE SIMPSON STRONG-TIE QUALITY POLICY

We help people build safer structures economically. We do this by designing, engineering and manufacturing "No Equal" structural connectors and other related products that meet or exceed our customers' needs and expectations. Everyone is responsible for product quality and is committed to ensuring the effectiveness of the Quality Management System.



GETTING FAST TECHNICAL SUPPORT

When you call for engineering technical support, we can help you quickly if you have the following information at hand. This will help us to serve you promptly and efficiently.

- Which Simpson Strong-Tie catalogue are you using? (See the front cover for the catalogue number)
- Which Simpson Strong-Tie product are you using?
- · What is your load requirement?
- What is the carried member's width and height?
- What is the supporting member's width and height?
- What is the carried and supporting members' material and application?



WE ARE ISO 9001-2008 REGISTERED

Simpson Strong-Tie is an ISO 9001-2008 registered company. ISO 9001-2008 is an internationally-recognized quality assurance system which lets our domestic and international customers know that they can count on the consistent quality of Simpson Strong-Tie® products and services.

USA and CANADA 800-999-5099 | www.strongtie.com

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CONNECTOR SELECTION KEY Products are divided into fifteen general

Fasteners &

Quik Drive® Systems

Products are divided into fifteen general categories, identified by tabs along the page's outer edge.





HTHMQ Heavy Multiple Truss Hanger

This heavy-duty hanger is designed to carry 2 or 3 trusses to enable a greater range of structural designs. Designed with versatility in mind, the HTHMQ will accommodate various lumber types and widths and multiple-ply trusses. The HTHMQ also facilitates right- or left-hand hips (at 30°-60° skews) and can be used for terminal hip trusses with or without the centre common jack.

See page 164 for more information.





HTHGQ Truss Girder Hanger

The HTHGQ is a high-capacity girder hanger designed to carry multi-ply trusses or composite lumber. It accommodates various widths for installation flexibility by enabling up to 5-ply girder trusses. The Strong-Drive® SDS screws that fasten the hanger also help transfer the load between the plies of the supporting girder when they penetrate all plies.

See page 168 for more information.





HHSUQ Heavy Severe-Skew Truss and SCL Hanger

The HHSUQ is a new high load capacity, face-mount, truss-to-truss hanger designed to accommodate severe skews (45° to 84°). This allowable skew range for hip trusses enables a greater range of installation applications. The HHSUQR model is skewed right and the HHSUQL version is skewed left.

See page 146 for more information.





HHRC Hip Ridge Connectors

The HHRC is a 12-gauge hip ridge connector that attaches hip roof beams to the end of a ridge beam. The HHRC is suitable for light commercial construction and may be sloped down a maximum of 35° (12:17 hip slope).

See page 96 for more information.

NEW PRODUCTS FOR 2015







Strong-Drive® 33° SCNR Ring-Shank Connector Nails

Simpson Strong-Tie® Strong-Drive® 33° SCNR Ring-Shank Connector Nails are designed to provide installers a pneumatically-driven alternative to bulk nails. This hot-dip galvanized nail is approved for use in many Simpson Strong-Tie® connector applications. These nails are available in 25-nail, paper-collated strips, and are compatible with a wide variety of popular pneumatic nailers.

See page 23 for more information.





PPBZ Porch Base

The new PPBZ porch base now offers a simplified, one-time installation designed to support permanent porch framing throughout all stages of construction. This design eliminates the need for temporary vertical support and streamlines the subcontractor scheduling process while still providing adequate safety to enable full access for installers/inspectors.

See page 60 for more information.





DU/DHU/DHUTF Drywall Hanger

The new DU/DHU face-mount and the DHUTF top-mount hangers are designed to carry joist floor loads to a wood stud wall through two layers of %" gypsum board (drywall). These hangers install after the drywall is in place. The hangers come in sizes that accommodate most joists used in multi-family construction including I-joists and trusses.

See page 150 for more information.





CPTZ Concealed Post Tie

This new concealed post base incorporates a knife plate with a standoff base. It achieves a clean, concealed look while providing a 1" standoff height above concrete. The CPTZ is installed with $\frac{1}{2}$ " diameter galvanized dowels (supplied). The standoff also reduces the potential for decay at post or column ends.

See page 59 for more information.





PCZ/EPCZ Post Caps

The new PCZ/EPCZ post caps enable one size to accommodate several post sizes. With post and header flanges now in-line, one PCZ or EPCZ model can accommodate several 4x or 6x post sizes for a variety of lighter-duty applications, including patio covers, trellises and interior framing.

See page 67 for more information.





JBA and LBA Top-Flange Hangers

The next-generation version of the JB and LB top-flange hangers, respectively, the JBA and LBA provide higher loads to accommodate 2x10, 2x12 and 2x14 members. New nail locations allow for use with nailers. The 14 gauge LBA may be welded to steel headers.

See page 85 for more information.



DISCONTINUED PRODUCTS



Products that will be discontinued or replaced in 2015

Simpson Strong-Tie is dedicated to continuously expanding our line of structural connectors with innovative new products that address the changing needs of our customers. As new connectors are introduced that improve upon older designs, it becomes necessary to discontinue the old versions in the name of efficiency and product-line simplicity.

The table below lists products that are no longer included in the *Wood Construction Connectors* catalogue as well as the products recommended to replace them. While technical information for discontinued products will be maintained on our website for a number of months, Simpson Strong-Tie asks that our customers begin to substitute the replacement products shown below in their designs and inventories. While it is hard to say when they will no longer be available from our distribution partners, production of some of these connectors ended in late 2014 and others will be phased out of production in 2015.

For the most current information on discontinued products visit **www.strongtie.com/discontinued**. If you have questions about any of the products shown below, please call (800) 999-5099 for assistance.



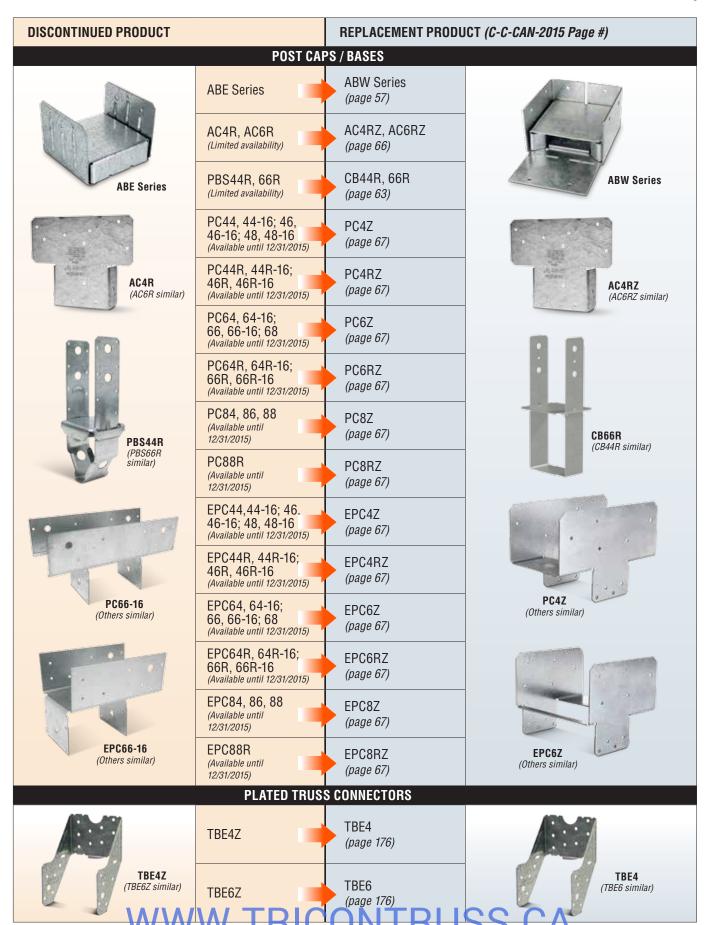
DISCONTINUED PRODUCTS



DISCONTINUED PRODUCT		REPLACEMENT PRODUCT (C-C-CAN-2015 Page #)		
	HANG	ERS		
70	JB210 (Limited availability)	JB210A (page 85)		
JB210 (JB212 and	JB212 (Limited availability)	JB212A (page 85)	JB214A (JB210A and	
JB214 similar)	JB214 (Limited availability)	JB214A (page 85)	JB212A similar)	
7	LB210 (Limited availability)	LB210A (page 85)		
LB210 (LB212 and	LB212 (Limited availability)	LB212A (page 85)	LB214A (LB210A and	
LB214 similar)	LB214 (Limited availability)	LB214A (page 85)	LB212A similar)	
V	ITS1.56/9.5 ITS1.56/11.88	LT159 LT151188 (page 118)	TPTT	
ITS2.56/11.25 (Others similar)	ITS2.56/9.25 (Available until 12/31/2015)	LBV2.56/9.25 (page 122)		
	ITS2.56/11.25 (No availability)	LBV2.56/11.25 (page 122)	LT159 LBV2.56/9.25 (LT151188 similar) (Others similar)	
7011	ITS3.56/9.25/11.25 (Available until 12/31/2015)	LBV3.56/9.25/11.25 (page 122)		
IUS2.56/9.25	IUS2.56/9.25 (No availability)	LF259 (page 107)		
HIT 35 Series	HIT 35 Series (No availability)	MIT 35 Series (page 118)	LF259 MIT 35 Series	
	TWIST	STRAP		
	MTS28C (Available until 12/31/2015)	MTS24C, 30C (page 186)		
MTS28C HTS28	HTS28 (Limited availability)	HTS24, 30 (page 186)	MTS24C (Others similar) HTS30 (Others similar)	

DISCONTINUED PRODUCTS





SIMPSON

<u>HOW TO USE THIS CATALOGUE</u>

NEW PRODUCTS

New products are shown with the symbol. There are also many new sizes within existing model series.

CHANGES IN RED

Significant changes from last year's catalogue are indicated in red.

HOW WE DETERMINE FACTORED RESISTANCES

Factored resistances in this catalogue are determined using calculations and/or one or more of the following methods:

- a minimum of 3 static load tests in wood assemblies;
- a minimum of 3 static load tests in steel jigs;
- a minimum of 3 static load tests of products embedded in concrete or masonry.

Some tests include only portions of a product such as purlin anchor tests — only the embedded hook is tested, not the nailed or bolted section of the strap, which is calculated. Testing to determine factored resistances in this catalogue is not done on connection systems in buildings. Testing is conducted under the supervision of an independent laboratory.

For detailed information regarding how Simpson Strong-Tie tests specific products, contact Simpson Strong-Tie.

WEINEER ED

VALUE ENGINEERED

This icon indicates a product that is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.



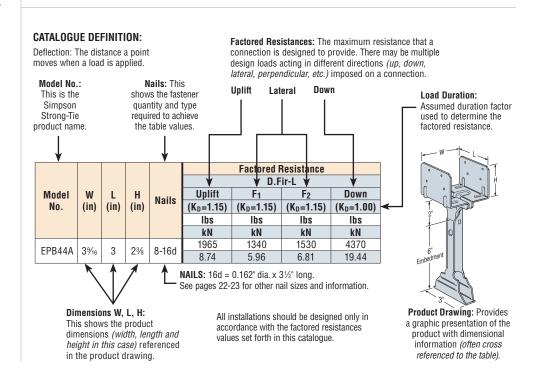
EXTRA CORROSION PROTECTION

This icon identifies products that are available with additional corrosion protection ($ZMAX^{\circ}$, Hot-Dip Galvanized, stainless steel or the SDS double-barrier coating). Other products may also be available with additional protection, contact Simpson Strong-Tie for options. The end of the product name will indicate what type of extra corrosion protection is provided (Z = ZMAX, HDG = Hot-Dip Galvanized or SS = stainless steel). See pages 14-17 for information on corrosion, and visit our website www.strongtie.com/info</code> for more technical information on this topic.



STRONG-DRIVE® SD SCREW COMPATIBLE

This icon identifies products approved for installation with the Simpson Strong-Tie Strong-Drive® SD Connector screw. See page 24 for more information.



CORROSION INFORMATION

Understanding the Corrosion Issue

Many environments and materials can cause corrosion including ocean salt air, fire-retardants, fumes, fertilizers, preservative-treated wood, de-icing salts, dissimilar metals and more. Metal connectors, fasteners and anchors could corrode and lose load-carrying capacity when installed in corrosive environments or when installed in contact with corrosive materials.

The many variables present in a building environment make it impossible to accurately predict if, or when, corrosion will begin or reach a critical level. This relative uncertainty makes it crucial that specifiers and users are knowledgeable of the potential risks and select a product suitable for the intended use. It is also prudent that regular maintenance and periodic inspections are performed especially for outdoor applications.

It is common to see some corrosion in outdoor applications. Even stainless steel can corrode. The presence of some corrosion does not mean that load capacity has been affected or that failure is imminent. If significant corrosion is apparent or suspected, then the wood, fasteners and connectors should be inspected by a qualified engineer or qualified inspector. Replacement of affected components may be appropriate.

Some wood-preservative chemicals and fire retardant chemicals and retentions pose increased corrosion potential and are more corrosive to steel connectors and fasteners than others. Testing by Simpson Strong-Tie has shown that ACQ-Type D is more corrosive than Copper Azole Type C, Micronized Copper Azole, and CCA-C. At the same time, others have shown that the inorganic boron treatment chemicals, specifically SBX-DOT, is less corrosive than CCA-C.

Due to the many different chemical treatment formulations, chemical retention levels, moisture conditions and regional formulation variants, selection of fasteners has become a complex task. We have attempted to provide basic knowledge on the subject here, but it is important to fully educate yourself by reviewing our technical bulletins on the topic (www.strongtie.com/info) and also by reviewing information, literature and evaluation reports published by others.

Corrosion Information



Treatment Use Categories and Exposure Conditions

The American Wood Protection Association (AWPA) and the Canadian Standards Association (CSA) identify several Use Category designations (UC) for wood treatment chemicals that are based on protection of the wood material; the Use Categories are based on service conditions and environments and agents of deterioration. At the same time, the building codes require specific corrosion resistance for fasteners that are in contact with chemically treated wood, and the corrosion resistance is independent of the service environments and treatments that are the basis of the AWPA and CSA Use Categories. From the building code perspective, fastener corrosion resistance is provided by hot-dip galvanization applied following ASTM A153, Class D or a corrosion resistant base metal, such as stainless steel, silicon bronze or copper regardless of exposure.

The International Code Council – Evaluation Service (ICC-ES) implemented AC257 as a method to evaluate alternate corrosion resistance mechanisms for fasteners used in wood construction where hot-dip galvanization (ASTM A153, Class D) is used as the benchmark performance. Under AC257, fastener corrosion resistance is qualified for one or more of four exposure conditions with no salt exposure: (1) treated wood in dry-service; (2) clean wood in a salt air dry-service environment; (3) treated wood in a wet-service condition; with no salt exposure; and (4) general use with no limitations.

The Use Category system (UCS) employed in the CAN/CSA 080 Series of Standards is based on the UCS developed by AWPA, with minor differences that account for treated wood production and use patterns in Canada. Some of the categories listed by AWPA are not included in CAN/CSA 080 due to regional use and/or other factors.

Use Category System (UCS)			
CAN/CSA 080.1 AWPA U1-13			
UC1	UC1		
UC2	UC2		
UC3.1	UC3A		
UC3.2	UC3B		
UC4.1	UC4A		
UC4.2	UC4B		
UC5A	UC5A		
UCF.1	UCFA		

COATINGS AVAILABLE

Not all products are available in all finishes. Contact Simpson Strong-Tie for product availability, ordering information and lead times.

Finish/Material	Description	Level of Corrosion Resistance
Gray Paint	Water-based paint intended to protect the product while it is warehoused and in transit to the jobsite.	Low
Powder Coating	Baked on paint finish that is more durable than our standard paint and produces a better looking finished product.	Low
Standard G90 Zinc Coating	Zinc galvanized coating containing 0.90 oz. of zinc per square foot of surface area (total both sides).	Low
Electrocoating (E-Coat™)	Electrocoating utilizes electrical current to deposit the coating material on the fastener. After application, the coating is cured in an oven. Electrocoating provides a minimum amount of corrosion protection and is recommended for dry, non-corrosive applications only.	Low
TIMAX . G185	Galvanized (G185) 1.85 oz. of zinc per square foot of surface area (hot-dip galvanized per ASTM A653 total both sides). These products require hot-dip galvanized fasteners (fasteners which meet the specifications of ASTM A153).	Medium
H <mark>OTDIPD (G</mark> GALVANIZED®	Products are hot-dip galvanized after fabrication (14 ga. and thicker). The coating weight increases with material thickness. The minimum average coating weight is 2.0 oz./ft² (per ASTM A123 total both sides). These products require hot-dip galvanized fasteners (fasteners which meet the specifications of ASTM A153). Anchor bolts are hot-dip galvanized per ASTM F2329.	Medium
Type 410 Stainless Steel with Protective Top Coat	Carbon martensitic grade of stainless steel which is inherently magnetic, with an added protective top coat. This material can be used in mild atmospheres and many mild chemical environments.	Medium
Mechanically-Galvanized Coating, Class 55	Simpson Strong-Tie Strong-Drive® SD Connector screw is manufactured with a mechanically-applied zinc coating in accordance with ASTM B695, Class 55 with a supplemental overcoat. These fasteners are compatible with painted and zinc-coated (G90 and ZMAX) connectors.	Medium
Double-Barrier Coating	Simpson Strong-Tie Strong-Drive SDS Heavy-Duty Connector screw is manufactured with two different finishes that together provide a level of corrosion protection that equals that provided by the previous HDG coating.	Medium
STATULO STAINLESS STEEL®	Connectors are manufactured from Type 316L stainless steel, and provide greater durability against corrosion. Stainless-steel nails are required with stainless-steel products, and are available from Simpson Strong-Tie.	High/Severe

See Corrosion Information for more specific performance and application information on these finishes.

CORROSION INFORMATION



Simpson Strong-Tie General Recommendations

Simpson Strong-Tie has evaluated the AWPA Use Categories (AWPA U1-13) and the ICC-ES, AC257 Exposure Conditions and developed from that evaluation a set of Corrosion Resistance Recommendations. These recommendations address the coating systems and materials used by Simpson Strong-Tie for connector and fastener products.

Dry-service (or damp-service) environments lead to wood moisture contents less than or equal to 19%. The corrosion potential, even in chemically treated wood, is reduced in these conditions. These conditions are typical of AWPA UC1 and UC2 for wood treatment and AC257 Exposure Condition 1. See the Corrosion Resistance Classification Table for the Simpson Strong-Tie assessment of corrosion needs in these conditions. The AC257 Exposure Condition 2 reflects the presence of air-borne salt in a dry-service environment and corrosion hazard to exposed metal surfaces; it does not include effects of treatment chemicals.

Outdoor environments are generally more corrosive to steel either because the moisture exposure is elevated (greater than 19%) and/or the treatment chemical retention level is higher than for interior service. The AWPA classifies exterior above ground treatments as Use Categories UC3 (A and B) depending on moisture run-off; and for ground-contact levels of protection, it has Use Categories UC4 (A-C). ICC-ES considers the exterior exposure to be limited by the type of chemicals and retention level of the chemicals in the qualification testing and whether the exposure includes salt exposure. In general, The AC257 Exposure Condition 3 includes AWPA Use Categories UC1 (interior dry) to UC4A (exterior ground contact, general use).

Types 316/305/304 stainless steel, copper, silicon bronze and hot-dip galvanized (Class-C) are the most effective protection against corrosion risk, where Type 316 is the best choice for salt marine and chloride containing environments

regardless of treatment chemicals or wood species. If you choose to use hot-dip galvanized (Class-D), mechanically galvanized (C3, N2000, or Class 55), double-barrier or Quik Guard coated fasteners on outdoor projects (e.g., a deck), you should periodically inspect the fasteners or have a professional inspection performed, and regular maintenance is a good practice. See the Corrosion Resistance Classifications Table for the Simpson Strong-Tie assessment of the corrosion resistance associated with materials and coatings and an appropriate level of corrosion resistance for various environments.

Due to the many variables involved, Simpson Strong-Tie cannot provide estimates of service life of connectors and fasteners. We suggest that all users and specifiers obtain recommendations on corrosion from the treated wood supplier or for the type of wood used. As long as Simpson Strong-Tie recommendations are followed, Simpson Strong-Tie stands behind its product performance and our standard warranty applies (page 21).

Simpson Strong-Tie does not recommend painting stainless steel fasteners or hardware. The reason behind this recommendation is that sometimes painting can facilitate corrosion. Stainless steel is "stainless" because it forms a protective chromium oxide film on the surface by passive oxidation with air. The paint film on the stainless steel surface may be imperfect or it can be injured during service, and in either case the metal may be exposed. Microscopic-sized film imperfections and scratches facilitate collection of dirt and water that can be stagnant and degrade or block the passive formation of the protective chromium oxide film. When this happens, crevice corrosion can initiate. Crevice corrosion eventually becomes visible as a brown stain or as red rust. This is the reason that painting usually does not improve corrosion resistance of stainless steel.

Guidelines for Selecting Corrosion-Resistant Connectors and Fasteners

Evaluate the Application

Consider the importance of the connection.

Evaluate the Exposure

Consider these moisture and treatment chemical exposure conditions:

- Dry service: Generally INTERIOR applications and includes wall and ceiling cavities, raised floor applications in enclosed buildings that have been designed to prevent condensation and exposure to other sources of moisture. Prolonged exposure during construction should also be considered, as this may constitute a Wet Service or Elevated Service Condition.
- Wet Service: Generally EXTERIOR construction in conditions other than Elevated Service. These include Exterior Protected and Exposed and General Use Ground Contact as described by the AWPA UC4A.
- Elevated Service: Includes fumes, fertilizers, soil, some preservative-treated wood (AWPA UC4B and UC4C), industrial zones, acid rain and other corrosive elements.

- Uncertain: Unknown exposure, materials, or treatment chemicals.
- Ocean/Water Front: Marine environments that include airborne chlorides and some splash. Environments with de-icing salts are included.
- Treatment Chemicals: See AWPA Use Category Designations.
 The preservative-treated wood supplier should provide all of the pertinent information about the wood being used. The information should include Use Category Designation, wood species group, wood treatment chemical, and chemical retention. See appropriate evaluation reports for corrosion effects of treatment chemicals and fastener corrosion resistance recommendations.

Use the Simpson Strong-Tie® Corrosion Classification Table

If the treatment chemical information is incomplete, Simpson Strong-Tie recommends the use of a 300 series stainless steel product. Also if the treatment chemical is not shown in the Corrosion Classification Table, then Simpson Strong-Tie has not evaluated it and cannot make any recommendations other than the use of coatings and materials in the Severe category. Manufacturers may independently provide test results of other product information; Simpson Strong-Tie expresses no opinion regarding such information.

CORROSION INFORMATION

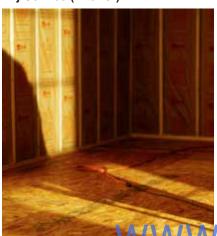


CORROSION RESISTANCE RECOMMENDATIONS						
Low Medium High Severe						
	FASTI	ENERS				
Phosphate (gray, black), Clear (bright) zinc (ASTM F1941), Heavy electro-galvanized (ASTM A641-Class 1), Yellow zinc (ASTM F1941), Electrocoat (E-coat), Type 410 stainless steel	Mechanically galvanized (AS 3566.2-C3, N2000, ASTM B695-Class 55), Quik Guard® coating, Hot-dip galvanized (ASTM A153-Class D), Double-barrier coating, Type 410 stainless steel with protective top coat	Type 304 stainless steel, Type 305 stainless steel	Type 316 stainless steel, Hot-dip galvanized (ASTM A153-Class C), Silicon bronze, Copper			
CONNECTORS						
Simpson Strong-Tie® gray paint Powder coating Standard G90 zinc coating	ZMAX® (G185) Hot-dip galvanized (ASTM A153 - Class D)	Type 316L stainless steel	Type 316L stainless steel			

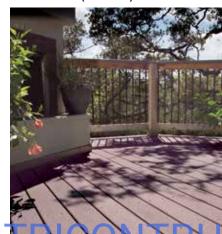
CORROSION RESISTANCE CLASSIFICATIONS							
			Mate	rial To Be Fastened			
Environment	Untreated		Prese	rvative-Treated Woo	d		
Environment Untreated Wood or Other Material		SBX-DOT Zinc Borate	Chemical Retention ≤ AWPA, UC4A	Chemical Retention > AWPA, UC4A	ACZA	Other or Uncertain	FRT Wood
Dry Service	Low	Low	Low	High	High	High	Med
Wet Service	Med	N/A	Med	High	High	High	High
Elevated Service	High	N/A	Severe	Severe	High	Severe	N/A
Uncertain	High	High	High	Severe	High	Severe	High
Ocean/Water Front	Severe	N/A	Severe	Severe	Severe	Severe	N/A

- 1. These are general guidelines that may not consider all application criteria. Refer to product specific information for additional guidance.
- 2. Type 316/305/304 stainless steel products are recommended where preservativetreated wood used in ground contact has a chemical retention level equal to or greater than those for AWPA UC4A: CA-C, 0.1 pcf; CA-B, 0.21 pcf; micronized CA, 0.20 pcf; ACQ-Type D (or C), 0.40 pcf.
- 3. Testing by Simpson Strong-Tie following ICC-ES AC257 showed that mechanical galvanization (ASTM B695, Class 55), Quik Guard coating, and Double Barrier coating will provide corrosion resistance equivalent to hot-dip galvanization (ASTM A153, Class D) in contact with chemically treated wood in dry service and wet service exposures (AWPA UC1-UC4A, ICC-ES AC257 Exposure Conditions 1 and 3) and will perform adequately subject to regular maintenance and periodic inspection.
- 4. Mechanical galvanizations C3 and N2000 should not be used in conditions that would be more corrosive than AWPA UC3A (exterior, above ground, rapid water
- 5. If uncertain about Use Category, treatment chemical, or environment, use Types 316/305/304 stainless steel, silicon bronze or copper.
- 6. Some treated wood may have excess surface chemicals making it potentially more corrosive than lower retentions. If this condition is suspected, use Type 316/305/304 stainless steel, silicon bronze, or copper fasteners.
- 7. Type 316 stainless steel, silicon bronze, and copper fasteners are the best recommendation for ocean salt-air and other chloride-containing environments. Hot-dip galvanized fasteners with at least ASTM A153, Class C protection can also be an alternate for some applications in environments with ocean air and/or elevated wood moisture content.

Dry Service (Interior)



Wet Service (Exterior)



Ocean/Water Front





CODES

Simpson Strong-Tie® connectors are recognized by most code agencies. Agencies that recognize some or all of our products include CCMC, ICC-ES; the City of Los Angeles, California; State of Florida; and IAPMO Evaluation Service.

The factored resistances shown in this catalogue comply with the National Building Code of Canada (NBC 2010 and NBC 2015).

Department of State Architecture, State of California: The DSA of California is no longer issuing Product Acceptance Reports on wood to wood connections – joist hangers. AC25-2, a new acceptance criteria drafted by the DSA, specifically states, companies with current ICBO code reports and/or third party witnessed testing will be recognized as acceptable structural connections for DSA specified projects. Further to AC25-2, the DSA is requiring that those companies who are supplying the aforementioned products to State specified projects be ISO9001 certified.

Call Simpson Strong-Tie or visit the code agencies' web sites for the current evaluation reports if recognition or approval is to be based on the report. Specific reductions and restrictions may be required by other code agencies. **CCMC**—Canadian Construction Materials Centre: Nos. CCMC 12862-R, 12863-R

International Code Council:

NER-209, 393, 413, 432, 443, 499, 694.

ER—1211, 4935, 5313, 5349, 5357, 5655, 5672, 5708, 5709, 5952. **ESR**—1622, 1866, 2105, 2203, 2236, 2330, 2549, 2551, 2552, 2553, 2554,

2555, 2604, 2605, 2606, 2607, 2608, 2611, 2613, 2614, 2615, 2616, 2877, 2920, 3046.

City of Los Angeles, CA—Nos. RR 25711, RR 25712, RR 25713, RR 25714, RR 25716, RR 25718, RR 25719, RR 25720, RR 25725, RR 25726, RR 25800, RR 25801, RR 25802, RR 25803, RR 25804, RR 25806, RR 25807, RR 25814, RR 25818, RR 25827, RR 25828, RR 25851

State of Florida—FL9589, 10441, 10444, 10446, 10447, 10456, 10531, 10655, 10667, 10849, 10852, 10856, 10854, 10860, 10861, 10864, 10865, 10866, 11166, 11169, 11468, 11470, 11473, 11478, 11496, 12708, 13326, 13628, 13904, 13975, 14101.

IAPMO Evaluation Service:

ER-112, 130, 143, 192

TERMS & CONDITIONS OF SALE

PRODUCT USE

Products in this catalogue are designed and manufactured for the specific purposes shown, and should not be used with other connectors not approved by a qualified Designer. Modifications to products or changes in installation procedures should only be made by a qualified Designer. The performance of such modified products or altered installation procedures is the sole responsibility of the Designer.

INDEMNITY

Customers or Designers modifying products or installation procedures, or designing non-catalogue products for fabrication by Simpson Strong-Tie Company Inc. shall, regardless of specific instructions to the user, indemnify, defend, and hold harmless Simpson Strong-Tie Company Inc. for any and all claimed loss or damage occasioned in whole or in part by non-catalogue or modified products.

NON-CATALOGUE AND MODIFIED PRODUCTS

Consult Simpson Strong-Tie Company Inc. for applications for which there is no catalogue product, or for connectors for use in hostile environments, with excessive wood shrinkage, or with abnormal loading or erection requirements.

Non-catalogue products designed by the customer will be fabricated by Simpson Strong-Tie in accordance with customer specifications.

Simpson Strong-Tie cannot and does not make any representations regarding the suitability of use or load-carrying capacities of non-catalogue products. Simpson Strong-Tie provides no warranty, express or implied, on non-catalogue products.

F.O.B. Shipping Point unless otherwise specified.

WARNING

Simpson Strong-Tie Company Inc. structural connectors, anchors, and other products are designed and tested to provide specified design loads. To obtain optimal performance from Simpson Strong-Tie Company Inc. products and achieve maximum allowable design load, the products must be properly installed and used in accordance with the installation instructions and design limits provided by Simpson Strong-Tie Company Inc. To ensure proper installation and use, Designers and installers must carefully read the following General Notes, General Instructions for the Installer and General Instructions tor the Designer, as well as consult the applicable catalog pages for specific product installation instructions and notes.

Proper product installation requires careful attention to all notes and instructions, including these basic rules:

- 1. Be familiar with the application and correct use of the connector.
- 2. Follow all installation instructions provided in the applicable catalog, website, Installer's Pocket Guide or any other Simpson Strong-Tie publications.
- 3. Install all required fasteners per installation instructions provided by Simpson Strong-Tie Company Inc.: a) use proper fastener type; b) use proper fastener quantity; c) fill all fastener holes; d) do not overdrive or underdrive nails, including when using gun nailers; and e) ensure screws are completely driven.
- 4. Only bend products that are specifically designed to be bent. For those products that required bending, do not bend more than once.
- 5. Cut joists to the correct length, do not "short-cut". The gap between the end of the joist and the header material should be no greater than 1/6" unless otherwise noted.

In addition to following the basic rules provided above as well as all notes, warnings and instructions provided in the catalog, installers, designers, engineers and consumers should consult the Simpson Strong-Tie Company Inc. website at **www.strongtie.com** to obtain additional design and installation information, including:

 Instructional builder/contractor training kits containing an instructional video an instructor guide and a student guide in both English and Spanish

- Installer's Pocket Guide (form S-INSTALL) which is designed specifically for installers and uses detailed graphics and minimal text in both English and Spanish to explain visually how to install many key products;
- Information on workshops Simpson Strong-Tie conducts at various training centers throughout the country;
- · Product specific installation videos;
- · Specialty catalogs;
- Code reports Simpson Strong-Tie® Code Report Finder software;
- Technical fliers and bulletins:
- · Master format specifications;
- · Material safety data sheets;
- · Corrosion information;
- Connector selection guides for engineered wood products (by manufacturer):
- Simpson Strong-Tie Connector Selector™ software;
- · Simpson Strong-Tie Autocad menu;
- $\bullet \ \, \text{Simpson Strong-Tie Strong-Wall} \\ ^{\circledcirc} \ \, \text{Selector software};$
- Simpson Strong-Tie Anchor Tiedown System Selector and anchor-related software; and
- · Answers to frequently asked questions and technical topics.

Failure to follow fully all of the notes and instructions provided by Simpson Strong-Tie Company Inc. may result in improper installation of products. Improperly installed products may not perform to the specifications set forth in this catalog and may reduce a structure's ability to resist the movement, stress, and loading that occurs from gravity loads as well as impact events such as earthquakes and high velocity winds.

Simpson Strong-Tie Company Inc. does not guarantee the performance or safety of products that are modified, improperly installed or not used in accordar ce viith the design and load limits set forth in this catalog.



FACTORED RESISTANCE DETERMINATION METHOD

The factored resistance is the maximum factored static load that can be imposed on a connection. Factored resistances in this catalogue are determined using calculations and/or one or more of the following methods: static load tests in wood assemblies; static load tests in steel jigs; static load tests of products embedded in concrete or masonry. Some tests include only portions of a product such as purlin anchor tests, where only the embedded hook is tested, not the nailed or bolted section of the strap, which is calculated.

Testing to determine factored resistances in this catalogue is not done on connection systems in buildings. Testing is conducted under the supervision of an independent laboratory. Some factored resistances are determined using calculations without testing. Tested and calculated factored resistances are determined in accordance with the appropriate material design standards, including CSA 086-14, CSA S16-14, CSA S136-12 and CSA A23.3-14.

For detailed information regarding how Simpson Strong-Tie tests specific products, contact your Simpson Strong-Tie representative or the company.

GENERAL NOTES

These general notes are provided to ensure proper installation of Simpson Strong-Tie Company Inc. products and must be followed fully.

- Simpson Strong-Tie Company Inc. reserves the right to change specifications, designs, and models without notice or liability for such changes.
- b. Steel used for each Simpson Strong-Tie® product is individually selected based on the product's steel specifications, including strength, thickness, formability, coating, and weldability. Contact Simpson Strong-Tie for steel information on specific products.
- c. Unless otherwise noted, dimensions are in inches, resistances are in pounds.
- d. Unless otherwise noted, bolts and nails cannot be combined. 8d (0.131x2½"), 10d (0.148x3") and 16d (0.162x3½") specify common nails that meet the requirement of CSA B111. When a shorter nail is specified, it will be noted (for example 8dx1½"). Refer to page 22 for more nail info.
- e. Unless otherwise noted, factored resistances are for Douglas Fir-Larch under continuously dry conditions (K_S =1.00). Factored resistances for other species or conditions must be adjusted according to CSA 086-14.
 - The following material properties were used to generate the resistances in this catalogue in accordance with CSA 086-14. For LVL and other engineered wood products verify with the manufacturer that their material properties meet or exceed the values shown in the table below.

Species	φ F _{CP}	Specific Gravity
Douglas Fir-Larch (D.Fir-L)	812 psi (5.60 MPa)	0.49
Spruce-Pine-Fir (S-P-F)	615 psi (4.24 MPa)	0.42
Hem-Fir (HF)	533 psi (3.68 MPa)	0.46
D.Fir-L Glulam	812 psi (5.60 MPa)	0.49
Spruce-Pine Glulam	672 psi (4.64 MPa)	0.44
LVL	1092 psi (7.53 MPa)	0.50
Parallam® PSL	1092 psi (7.53 MPa)	0.50
LSL (E=1.3x10 ⁶)	992 psi (6.84 MPa)	0.50
LSL (E>1.5x10 ⁶)	1092 psi (7.53 MPa)	0.50

- f. Simpson Strong-Tie Company Inc. will manufacture non-catalogue products provided prior approval is obtained and an engineering drawing is included with the order. Steel specified on the drawings as 1/6", 3/6", and 1/4" will be 11 gauge (0.120"), 7 gauge (0.179"), and 3 gauge (0.239"), respectively. The minimum yield and tensile strengths are 33 ksi and 52 ksi, respectively.
- g. All references to bolts or machine bolts (MBs) are for structural quality through bolts equal to or better than American Society of Testing and Materials ASTM Standard A307, Grade A or Society of Automotive Engineers standard SAEJ429, Grade 2. RFB is A307, Grade C; SSTB is ASTM A36.
- h. Unless otherwise noted, bending steel in the field may cause fractures at the bend line. Fractured steel will not carry load and must be replaced.
- i. A fastener that splits the wood will not take the factored load. Evaluate splits to determine if the connection will perform as required. Dry wood may split more easily and should be evaluated as required. If wood tends to split, consider pre-boring holes with diameters not exceeding 0.75 of the nail diameter. Use a 1/22" bit for Simpson Strong-Tie® Strong-Drive® SDS Heavy-Duty Connector screws and a 1/22" bit for Strong-Drive SD9/SD10 Connector screws.

- Wood shrinks and expands as it loses and gains moisture, particularly perpendicular to its grain. Take wood shrinkage into account when designing and installing connections. Simpson Strong-Tie manufactures products to fit common dry lumber dimensions. If you need a connector with dimensions other than those listed in this catalogue, Simpson Strong-Tie may be able to vary connector dimensions; contact Simpson Strong-Tie. The effects of wood shrinkage are increased in multiple lumber connections, such as floor-to-floor installations. This may result in the vertical rod nuts becoming loose, requiring post-installation tightening.
- k. Top flange hangers may cause unevenness. Possible remedies should be evaluated by a professional and include using a face mount hanger, and routering the beam or cutting the subfloor to accommodate the top flange thickness.
- Built-up lumber (multiple members) must be fastened together to act as one unit to resist the applied load (excluding the connector fasteners). This must be determined by the Designer/Engineer of Record.
- m. Do Not Overload. Do not exceed catalogue factored resistances, which would jeopardize the connection.
- n. Some model configurations may differ from those shown in this catalogue. Contact Simpson Strong-Tie for details.
- o. Hanger Options some combinations of hanger options are not available. In some cases, combinations of these options may not be installable. Horizontal loads induced by sloped joists must be resisted by other members in the structural system. A qualified Designer must always evaluate each connection, including carried and carrying member limitations, before specifying the product. Fill all fastener holes with fastener types specified in the tables, unless otherwise noted. Hanger configurations, height, and fastener schedules may vary from the tables depending on joist size, skew and slope. See the tabulated factored resistance for the non-modified hanger, and adjust as indicated. Gauge may vary from that specified depending on the manufacturing process used. U and W hangers normally have single stirrups; occasionally, the seat may be welded. B, GLT, HGLT, HW, LBV, W and WNP hangers for sloped seat installations are assumed backed. To order a custom non-backed hanger, contact the Simpson Strong-Tie.
- p. Simpson Strong-Tie will calculate the net height for a sloped seat. The customer must provide the H1 joist height before slope.
- q. Truss plates shown are the responsibility of the truss designer.
- r. Do not weld products listed in this catalogue unless this publication specifically identifies a product as acceptable for welding or unless specific approval for welding is provided in writing by Simpson Strong-Tie. Some steels have poor weldability and a tendancy to crack when welded. Cracked steel will not carry load and must be replaced.
- s. Unless noted otherwise, all references to standard cut washers refer to Type A plain washers (W) conforming to the dimensions shown in ASME B18.22.1 for the appropriate rod size. Some products require SAE narrow washers (N) to fit in a tight space and are noted accordingly.



GENERAL INSTRUCTIONS FOR THE DESIGNER

- a. Factored resistances for hangers are determined by a static load test resulting in not more than a ½" (3mm) deflection of the joist relative to the header.
- b. Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows:
 - Factored Uplift/Factored Uplift Resistance + Factored Parallel to Plate / Factored Parallel to Plate Resistance + Factored Perpendicular to Plate / Factored Perpendicular to Plate Resistance < 1.0.

The three terms in the unity equation are due to the three possible directions that exist to generate force on a hurricane tie. The number of terms that must be considered for simultaneous loading is at the sole discretion of the Designer and is dependant on the method of calculating wind forces and the utilization of the connector within the structural system.

- Factored resistances are based on CSA 086-14 unless otherwise specified.
- d. Load Duration Factor, K_D as specified by CSA 086-14 is as follows: **Standard term** ($K_D = 1.00$) – applies to all roof and floor factored resistances and is designated as "Normal" in tables.
 - **Short term** ($K_D = 1.15$) applies to all wind and seismic factored resistances. Other factored resistance values, based on load durations or special conditions, may govern in certain geographic areas and may be used where applicable, up to the maximum tabulated factored resistance. Load duration increases are only applied if the factor of safety can be maintained.
- e. Wood shear is not considered in the factored resistances given; reduce factored resistances when wood shear is limiting.
- f. Simpson Strong-Tie strongly recommends the following addition to construction drawings and specifications: "Simpson Strong-Tie® connectors are specifically required to meet the structural calculations of plan. Before substituting another brand, confirm factored resistances based on reliable published testing data or calculations. The Engineer/

- Designer of Record should evaluate and give written approval for substitution prior to installation."
- g. Verify that the dimensions of the supporting member are sufficient to receive the specified fasteners, and develop the top flange bearing length.
- h. Some catalogue illustrations show connections that could cause tension stresses perpendicular to grain or bending of the wood during loading if not sufficiently reinforced. In this case, mechanical reinforcement should be considered.
- Simpson Strong-Tie recommends that hanger height be at least 60% of joist height for stability.
- j. The term "Designer" used throughout this catalogue is intended to mean a licensed/certified building design professional, a licensed professional engineer, or a licensed architect.
- k. For holdowns, anchor bolt nuts should be finger-tight plus ½ to ½ turn with a hand wrench, with consideration given to possible future wood shrinkage. Care should be taken to not over-torque the nut. Impact wrenches should not be used as they may preload the holdown.
- I. Holdown and Tension Tie capacities are based on installations with an anchor rod length of 6" from the concrete to top of holdown seat. These products may be raised to any height with consideration of the increased deflection due to additional rod elongation. For cases where the anchor rod is offset, Simpson Strong-Tie offers recommendations, subject to the approval of the Designer, which permit holdowns to be raised up to 18" maximum with a corresponding horizontal anchor rod offset of 1½". See "General Instructions for the Installer" (page 21 note q).
- m. Throughout the catalogue there are installation drawings showing the load transfer from one element in the structure to another. Additional connections may be required to safely transfer the loads through the structure. It is the Designer's responsibility to specify and detail all necessary connections to ensure that a continuous load path is provided as required by the building code.

GENERAL INSTRUCTIONS FOR THE INSTALLER

These general instructions for the installer are provided to ensure proper selection and installation of Simpson Strong-Tie Company Inc. products and must be followed carefully. These general instructions are in addition to the specific installation instructions and notes provided for each particular product, all of which should be consulted prior to and during installation of Simpson Strong-Tie Company Inc. products.

- a. All specified fasteners must be installed according to the instructions in this catalogue. Incorrect fastener quantity, size, placement, type, material, or coating may cause the connection to fail. Prior to using a particular fastener, please consult the Fastener Guide in this catalogue.
 - 16d fasteners are common nails (0.162" dia. x 3½" long) and cannot be replaced with 16d sinkers (0.148" dia. x 3¼" long) for full load value unless otherwise specified.
 - Screws may not be used to replace nails in connectors unless approved and recommended by the Designer/Engineer of Record.
 Unless stated otherwise, Simpson Strong-Tie cannot and does not make any representations regarding the suitability of use or load-carrying capacities of connectors with screws replacing nails.
 - When using stainless-steel connectors, use stainless-steel fasteners.
 When using ZMAX®/HDG galvanized connectors, use fasteners that meet the zinc coating specifications of ASTM A153.
- b. Fill all fastener holes as specified in the installation instructions for that product. Refer to Simpson Strong-Tie Fastener Guide for the requirements of the various shaped fastener holes.
- c. Do not overdrive nails. Overdriven nails reduce shear capacity.
- d. Use the materials specified in the installation instructions. Substitution
 of or failure to use specified materials may cause the connection to fail.
- e. Do not add fastener holes or otherwise modify Simpson Strong-Tie Company Inc. products. The performance of modified products may be substantially weakened. Simpson Strong-Tie will not warrant or guarantee the performance of such modified products.
- f. Install products in the position specified in the catalogue.

- g. Do not alter installation procedures from those set forth in this catalogue.
- h. The proper use of certain products requires that the product be bent.
 For those products, installers must not bend the product more than one time (one full cycle).
- i. Bolt holes shall be at least a minimum of $\frac{1}{2}$ (1 mm) and no more than a maximum of $\frac{1}{16}$ (2 mm) larger than the bolt diameter (per 12.4.1.2 CSA 086-14).
- j. Install all specified fasteners before loading the connection.
- k. Some hardened fasteners may have premature failure if exposed to moisture. These fasteners are recommended to be used in dry interior applications.
- I. Use proper safety equipment.
- m. Welding galvanized steel may produce harmful fumes; follow proper welding procedures and safety precautions. Welding should be in accordance with CSA W59. Unless otherwise noted Simpson Strong-Tie connectors cannot be welded.
- n. Pneumatic or powder-actuated fasteners may deflect and injure the operator or others. Pneumatic nail tools may be used to install connectors, provided the correct quantity and type of nails (length and diameter) are properly installed in the nail holes. Tools with nail hole-locating mechanisms should be used. Follow the manufacturer's instructions and use the appropriate safety equipment. Overdriving nails may reduce allowable loads. Contact Simpson Strong-Tie. Powder-actuated fasteners should not be used to install connectors.
- o. Joist shall bear completely on the connector seat, and the gap between the joist end and the header shall not exceed 1/8" (3 mm) per ASTM D17c1 and ASTM D7147 lest standards.



GENERAL INSTRUCTIONS FOR THE INSTALLER (cont.)

- p. For holdowns, anchor bolt nuts should be finger-tight plus ½ to ½ turn with a hand wrench, with consideration given to possible future wood shrinkage. Care should be taken to not over-torque the nut. Impact wrenches should not be used as they may preload the holdown.
- q. Holdowns and Tension Ties may be raised off the sill as dictated by field conditions to accommodate an anchor mislocated no more than 1½". The holdown shall be raised off the sill at least 3" for every ¼" that the anchor is offset from the model's centreline (as defined on pages 49 to 56 to maximum of 18"). Anchor bolt slope shall be no greater than 1:12 (or 5 degrees). Contact the Designer if the holdown anchor is offset more then 1½". Raised holdown height is measured from the top of concrete to the top of the holdown bearing plate.
- r. Strong-Drive® Screws are permitted to be installed through metal truss plates as approved by the Truss Designer (pre-drilling required through the plate using a maximum of a 5/22" bit).
- For cold-formed steel applications, all screws shall be installed in accordance with the screw manufacturer's recommendations. All screws

- shall penetrate and protrude through the joined materials a minimum of 3 full exposed threads per AISI Standard for Cold Formed Steel Framing General Provisions, section D1.3, if applicable.
- Nuts shall be installed such that the end of the threaded rod or bolt is at least flush with the top of the nut.
- When installing hurricane ties on the inside of the wall special considerations must be taken to prevent condensation on the inside of the completed structure in cold climates.
- Unless otherwise noted, connectors shown in this catalogue have been designed to be installed at the time the framing members are installed. Contact Simpson Strong-Tie for retrofit suitability of specific connectors including those manufactured in accordance with the hanger options section of this catalogue.
- w. When bolts are used, standard cut washers must be placed between the wood and the nut or the wood and the head when there is no steel between. (see 12.2.2.4 – CSA 086-14)

LIMITED WARRANTY

Simpson Strong-Tie Company Inc. warrants catalogue products to be free from defects in material or manufacturing. Simpson Strong-Tie Company Inc. products are further warranted for adequacy of design when used in accordance with design limits in this catalogue and when properly specified, installed, and maintained. This warranty does not apply to uses not in compliance with specific applications and installations set forth in this catalogue, or to non-catalogue or modified products, or to deterioration due to environmental conditions.

Simpson Strong-Tie® connectors are designed to enable structures to resist the movement, stress, and loading that results from impact events such as earthquakes and high velocity winds. Other Simpson Strong-Tie products are designed to the load capacities and uses listed in this catalogue. Properly-installed Simpson Strong-Tie products will perform in accordance with the specifications set forth in the applicable Simpson Strong-Tie catalogue. Additional performance limitations for specific products may be listed on the applicable catalogue pages.

Due to the particular characteristics of potential impact events, the specific design and location of the structure, the building materials used,

the quality of construction, and the condition of the soils involved, damage may nonetheless result to a structure and its contents even if the loads resulting from the impact event do not exceed Simpson Strong-Tie catalogue specifications and Simpson Strong-Tie connectors are properly installed in accordance with applicable building codes.

All warranty obligations of Simpson Strong-Tie Company Inc. shall be limited, at the discretion of Simpson Strong-Tie Company Inc., to repair or replacement of the defective part. These remedies shall constitute Simpson Strong-Tie Company Inc.'s sole obligation and sole remedy of purchaser under this warranty. In no event will Simpson Strong-Tie Company Inc. be responsible for incidental, consequential, or special loss or damage, however caused.

This warranty is expressly in lieu of all other warranties, expressed or implied, including warranties of merchantability or fitness for a particular purpose, all such other warranties being hereby expressly excluded. This warranty may change periodically – consult our website www.strongtie.com for current information.

Conversion Charts

Metric Conversion

Imperial	Metric
1 in	25.40 mm
1 ft	0.3048 m
1 lb	4.448N
1 Kip	4.448 kN
1 psi	6895 Pa

Bolt Diameter

in	mm	
3/8	9.5	
1/2	12.7	
5/8	15.9	
3/4	19.1	
7/8	22.2	
1	25.4	

If Common Rafter

Slope
5°
10°
14°
18°
23°
27°
30°
34°
37°
40°
42°
45°

Then Hip/Valley Rafter Roof Pitch becomes...

Rise/Run	Slope
1/17	3°
2/17	7°
3/17	10°
4/17	13°
5/17	16°
6/17	19°
7/17	22°
8/17	25°
9/17	28°
10/17	30°
11/17	33°
12/17	35°

US Standard Steel Gauge Equivalents in Nominal Dimensions

Min. Ga Thick.		Approximate Dimensions		Decimals (in)		
ua	(mils)	in	mm	Uncoated Steel	Galvanized Steel (G90)	ZMAX (G185)
3	229	1/4	6.0	0.239		1
7	171	3/16	4.5	0.179	0.186	_
10	118	9/64	3.5	0.134	0.138	0.140
11	111	1/8	3.1	0.120	0.123	0.125
12	97	7/64	2.7	0.105	0.108	0.110
14	68	5/64	2.0	0.075	0.078	0.080
16	54	1/16	1.6	0.060	0.063	0.065
18	43	3/64	1.3	0.048	0.052	0.054
20	33	1/32	1.0	0.036	0.040	0.042
22	27	1/32	1.0	0.030	0.033	0.035

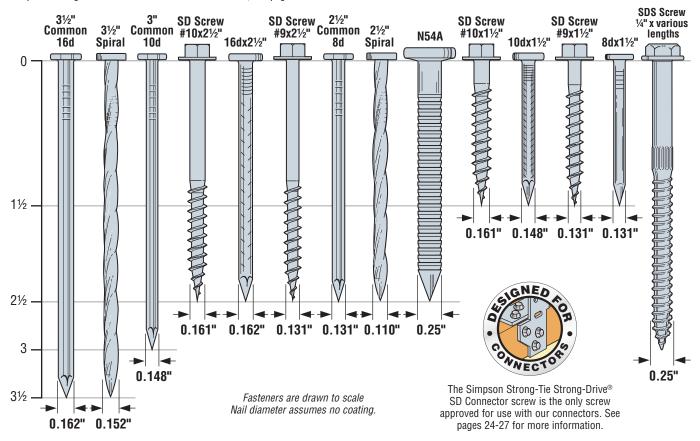
Use these Roof Pitch to Hip/Valley Rafter Roof Pitch conversion tables only for hip/valley rafters that are skewed 45° right or left. All other skews will cause the slope to change from that listed.

WWW.TRICONTRUSS.CA

Fastener Types and Sizes Specified for Simpson Strong-Tie® Connectors

Many Simpson Strong-Tie connectors have been designed and tested for use with specific types and sizes of fasteners. The specified quantity, type and size of fastener must be installed in the correct holes on the connector to achieve published values. Other factors such as fastener material and finish are also important. Incorrect fastener selection or installation can compromise connector performance and could lead to failure.

Simpson Strong-Tie does not offer all of these fasteners, see page 23 for more information.



NAIL DESIGN INFORMATION

In some cases it is desirable to install Simpson Strong-Tie face mount joist hangers and straight straps with nails that are a different type or size than what is called out in the load table. In these cases these reduction factors must be applied to the factored resistances listed for the connector.

Resistance Adjustment Factors for Optional Nails Used with Face Mount Hangers and Straight Straps

Specified Catalogue Nail	Replacement Nail	Face Mount Hangers	Straight Straps	
104 (0.100 01/)	10d common (0.148"x3")	0.00	0.00	
16d common (0.162"x3½")	12d common (0.148"x31/4")	0.83	0.83	
16d common (0.162"x31/2")	16dx2½" (0.162"x2½")	1.00	1.00	
16d common (0.162"x3½")	10dx1½" (0.148"x1½")	0.64	0.77	
16d common (0.162"x31/2")	16d spiral (0.152"x3½")	0.91	0.91	
16d common (0.162"x3½")	10d spiral (0.122"x3")	0.61	0.61	
100 common (0.102 x372)	12d spiral (0.122"x31/4")	0.01	0.01	
10d common (0.148"x3")	10dx2½" (0.148"x2½")	0.85	1.00	
10d common (0.148"x3")	8d common (0.131"x2½")	0.80	0.80	
10d common (0.148"x3")	10dx1½" (0.148"x1½")	0.77	0.92	
10d common (0.148"x3")	10d spiral (0.122"x3")	0.74	0.74	
8d common (0.131"x2½")	8dx1½" (0.131"x1½")	0.85	0.98	
8d common (0.131"x2½")	8d spiral (0.110"x2½")	0.64	0.75	



Double shear nailing should use full length common nails



Shorter nails may not be used as double shear nails

- 1. Resistance adjustment factors shown in the table are based on calculated reduction factors and are applicable for all face mount hangers and straight straps throughout this catalogue, except as noted in the footnotes below.
- 2. Some products have been tested specifically with alternate fasteners and have reduced capacities published on the specific product page which may differ from the values calculated using this table.
- 3. This table does not apply to hangers modified per the Hanger Options described on pages 230-231, or steel thicker than 10 gauge.
- 4. Unless noted otherwise, $10dx1\frac{1}{2}$ ", $10dx2\frac{1}{2}$ " or $16dx2\frac{1}{2}$ " nails may not be substituted for joist nails in double-shear hangers (i.e. LUS, HUS, HHUS, HGUS). For applications involving pneumatic nails, refer to specific tool manufacturer technical bulletins.
- 5. Do not substitute 10dx1½" nails for face nails on slope and skew combinations or skewed only LSU and LSSU
- straps installed over sheathing use a 2½" long nail minimum.

always fill, unless

noted otherwise.



Obround Holes Purpose: to make

fastening a connector in a tight location easier.

Fill Requirements: always fill.



Hexagonal Holes

Purpose: to fasten a connector to concrete or masonry.

Fill Requirements: always fill when fastening a connector

to concrete or masonry.



Triangular Holes

specifies Max nailing

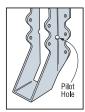
Purpose: to increase a connector's strength or to achieve Max strength.

Fill Requirements:
when the Designer



Diamond Holes

Purpose: to temporarily fasten a connector to make installing it easier. **Fill Requirements:** none.



Pilot Holes

Tooling holes for manufacturing purposes. No fasteners required.



Speed ProngsUsed to temporarily position and secure the connector for easier

and faster installation.



Positive Angle Nailing (PAN)

Provided when wood splitting may occur, and to speed installation.



Dome Nailing

This feature guides the nail into the joist and header at a 45° angle. U.S. Patent 5,603,580



Double-Shear Nailing

The nail is installed into the joist and header, distributing the load through two points on each joist nail for greater strength.



ITS Strong-Grip

(IUS Similar)

The Strong-Grip™ seat allows the I-joist to "snap" in securely without the need for joist nails.

STRONG-DRIVE® CONNECTOR NAILS

Simpson Strong-Tie nails and structural fasteners have been developed as the optimum fasteners for connector products. Special lengths afford economy of purchase and installation, and depth compatibility with framing members.

For pneumatic nail use, see Instructions to the Installer, pages 20-21 and visit **www.strongtie.com** for technical bulletins.

Nails Sold by the Pound

Nail	Simpson Model No.	Dimensions	Wire Gauge	Finish
8dx1½"	N8	0.131" x 1½"	101/4	HDG
OUX 1 72	SSN8	(3.3mm x 38.1mm)	1074	SS
8d Common	SS8D	0.131" x 2½" (3.3mm x 63.5mm)	101/4	SS
10dv11/"	N10	0.148" x 1½"	9	HDG
10dx1½"	SSN10	(3.8mm x 38.1mm)		SS
10d Common	10DHDG	0.148" x 3"	9	HDG
Tou Common	SS10D	(3.8mm x 76.2mm)	9	SS
16dx2½"	N16	0.162" x 2½" (4.1mm x 63.5mm)	8	Bright
16d Common	16DHDG	0.162" x 3½"	8	HDG
16d Common	SS16D	(4.1mm x 88.9mm)		SS
N54A	N54A	0.250" x 2½"	3	Bright
INDAA	N54AHDG	(6.4mm x 63.5mm)		HDG

- 1. HDG = hot-dip galvanized; SS = stainless steel; Bright = no finish.
- 2. For pneumatic fastener info, request additional technical information.
- 3. Use HDG nails with ZMAX® and HDG products.
- 16d sinker with GV finish is not acceptable for ZMAX or HDG applications.
- or HDG applications.

 5. HDG nails sold by Simpson Strong-Tie meet the specifications of ASTM A153. Stainless-steel nails are type 316 stainless.

Retail Packaging



1 lb. Retail Tub

Catalogue C-C-CAN2015 © 2015 SIMPSON STRONG-TIE COMPANY INC



5 lb. Retail Bucket

Simpson Strong-Tie hot-dip galvanized nails are packed in 1 lb. and 5 lb. plastic retail containers for easy handling.

STRONG-DRIVE® 33° SCNR RING-SHANK CONNECTOR NAILS

Simpson Strong-Tie® Strong-Drive® 33° SCNR Ring-Shank Connector Nails are designed to provide installers a pneumatically-driven alternative to hand-driven nails. This hot-dip galvanized nail is approved for use in many Simpson Strong-Tie® connector applications. These nail are available in 25-nail, paper-collated strips, and are compatible with a wide variety of popular pneumatic nailers.

MATERIAL: Heat-treated carbon steel
FINISH: Hot-dip galvanized, stainless steel
INSTALLATION: • Use all specified fasteners;
see General Notes.

- Follow the manufacturer's instructions and use the appropriate safety equipment.
- Tools with nail hole-locating mechanisms should be used.
- Overdriving nails may reduce capacities.
- Compatible with a wide variety of popular pneumatic nailers. For more information, visit www.strongtie.com/SCN.
- For applications involving pneumatic nails, refer to technical bulletin T-PNEUMATIC.

	Model No.	Nominal Size	Diameter (in.)	Length (in.)
	Hot	-Dip Galva	nized	
愈	N8HDGPT4000	8d	0.131	11/2
靊	8DHDGPT2500	8d	0.131	21/2
靊	N10HDGPT3000	10d	0.148	11/2
鬡	N10DHDGPT2500	10d	0.148	21/2
鬡	N16HDGPT2000	16d	0.162	21/2
		Bright		
靊	N8BRPT4000	8d	0.131	11/2
靊	8BRPT2500	8d	0.131	21/2
E	N10BRPT3000	10d	0.148	11/2
4	N TODDD DT 2500	104	0.140	21/



fastener capacities achieved by the SD9 and SD10 exceed those of typical 10d common or 16d common nails, respectively. In addition, the galvanized coating makes the Strong-Drive SD Connector screw ideal for interior and most exterior conditions.

The Strong-Drive SD Connector screw features an optimized shank which is specifically designed to be compatible with the fastener holes in Simpson Strong-Tie connectors. The hex head virtually eliminates cam-out and helps avoid stripping of the head during installation. The sharp point of the screw enables fast starts, and the patented serrated threads reduce torque for improved drivability.

FEATURES:

- Tested and approved for use in many of our best-selling connectors for both interior and most exterior applications
- The single-fastener steel-side-plate capacity of the SD9 exceeds the capacity of a 10d common nail, while the single-fastener capacity of the SD10 exceeds that of the 16d common nail
- Ideal for use in tight spaces where using a hammer is inconvenient
- · Optimized heat-treating for ductility and strength
- Mechanically-galvanized coating meets ASTM B695 Class 55. is recommended for use with certain preservative-treated woods (see pages 14-17)
- 1/4" hex drive
- · Head identification

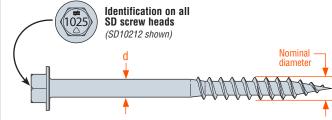
MATERIAL: Heat-treated carbon steel

FINISH: Mechanically galvanized (ASTM Class 55)



Product Information

Model No.	Shank Size (d)	Nominal Dia. (in)	Length (in)
SD9112R100			
SD9112R500			1½
SD9112MB	#9	0.177"	
SD9212R100	(0.131")	0.177	
SD9212R500			21/2
SD9212MB			
SD10112R100			
SD10112R500			1½
SD10112MB	#10 (0.161")	0.200"	
SD10212R100		0.200	
SD10212R500			21/2
SD10212MB			



Strong-Drive® SD10 CONNECTOR Screw

(Strong-Drive SD9 similar) U.S. Patent 7,101,133



Since testing of the Strong-Drive® SD Connector screw is ongoing, Simpson Strong-Tie continues to add connectors to the approvedconnector list. For the most current list of approved connectors, load values and applications, visit www.strongtie.com/strongdrive.

					Factored	Lateral Re	sistance ($K_D = 1.00$			
				D.F	ir-L		S-P-F				
		Thread		Side Plate Side Pla							
Model No.	Size (in)	Length	Wo	ood	Sto	eel	Wo	od	Sto	eel	
110.	(,	(in)	1/2"	11/2"	20 ga	12 ga	1/2"	11½"	20 ga	12 ga	
			lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	
			kN	kN	kN	kN	kN	kN	kN	kN	
SD9112	#9x1½	1	150	_	195	295	130	_	175	280	
309112	#9X172	ľ	0.67	_	0.87	1.31	0.58	_	0.78	1.25	
SD9212	#9x2½	1	240	230	285	390	205	195	250	360	
309212	#3X272	l	1.07	1.02	1.27	1.73	0.91	0.87	1.11	1.60	
SD10112	#10x1½	1	165	_	220	340	140	_	200	320	
3010112	# IUX I 72	1	0.73	_	0.98	1.51	0.62	_	0.89	1.42	
SD10212	#10x2½	w .	270	265	325	445	230	225	290	395	
3010212	# IUXZ72	\ \	1.20	1.18	1.45	1.98	1.02	1.00	1.29	1.76	

- 1. Factored resistances shown have been developed in accordance with 12.11 CSA 086-14. Apply the adjustment factors KD, KSF and KT as per 12.11.4.1 CSA 086-14 to the tabulated values shown when applicable. Resistances assume full penetration into the main member.
- 2. Factored resistances shown assume steel
- side plates with F_U = 45,000 psi (310 MPa). 3. Factored resistances shown for ½" wood side plates is applicable to structural panel side members (OSB, DFP and CSP) as per 12.11.4.2 CSA 086-14.
- Widrawal values for Strong-Drive® SD9 and SD10 Connector screws may be calculated in accordance with 12.11.5.2 CSA 086-14.
- 5. Minimum spacing edge and end distances ccordance with 12.9.2.1 CSA

STRONG-DRIVE® SDS HEAVY-DUTY CONNECTOR Screw

The Simpson Strong-Tie® Strong-Drive® SDS Heavy-Duty Connector screw is a $\frac{1}{4}$ " diameter structural wood screw ideal for various connector installations as well as wood-to-wood applications. It installs with no predrilling and has been extensively tested in various applications. The Strong-Drive SDS Heavy-Duty Connector screw is improved with a patented

The #8x1¼" SD8 Wafer-Head screw is ideal for miscellaneous fastening applications. The needle point ensures fast starts and deep #2 Phillips drive reduces cam-out and stripping.

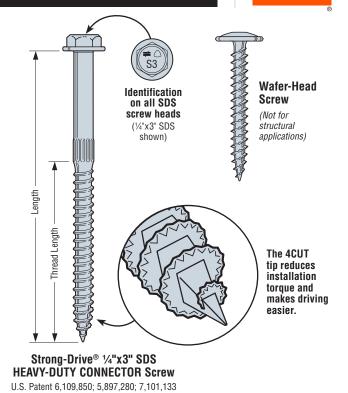
easy driving 4CUT™ tip and a corrosion resistant double-barrier coating.

STRONG-DRIVE® FEATURES:

- The patented 4CUT tip has a square core and serrated threads to reduce installation torque and make driving easier with no predrilling and minimal wood splitting.
- A double-barrier coating finish provides corrosion resistance equivalent to hot-dip galvanization. Now one screw can handle interior, exterior and certain pressure-treated wood applications (see Corrosion Information on pages 14-17 for more information).
- %" hex washer head is stamped with the No-Equal sign and fastener length for easy identification after installation.

MATERIAL: Heat-treated carbon steel, Type-316 stainless steel
FINISH: SDS—Double-barrier coating. Strong-Drive SDS Heavy-Duty
Connector screws may also be available yellow zinc dichromate or
HDG (Not all sizes are available in all coatings – Contact Simpson
Strong-Tie for product availability and ordering information);
SD8x1.25—Electro Galvanized.

WARNING: Industry studies show that hardened fasteners can experience performance problems in wet or corrosive environments. Accordingly, the SD8 should be used in dry, interior, and noncorrosive environments only.



📄 These products feature additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

					D.Fir-L							S-P-F					
				Factor	ed Latera	al Resista	ance (K _D	=1.00)	Factored	Factor	ed Later	al Resista	ance (K _D	= 1.00)	Fastanad		
		Thread	Fasteners			Side Plate	9		Factored Withdrawal		Side Plate		Factored Withdrawal				
Model	Size	Length	per	Wo	od		Steel		Resistance	Wo	od		Steel		Resistance		
No.	(in)	(in)		Carton	1½"	1¾" SCL	14 ga	10 ga	3 ga	(K _D = 1.15)	1½"	1¾" SCL	14 ga	10 ga	3 ga	$(K_D = 1.15)$	
				lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs		
				kN	kN	kN	kN	kN	kN	kN	kN	kN	kN	kN	kN		
SD8x1.25	5/32 X 11/4	_		_	_				_	_	_				_		
	732 A 1 /4			_	_				_		_				_		
SDS25112	½ x 1½	1	1500	_	_	340	465	545	280	_		315	435	435	215		
SDS25112SS	/4 X 1 /2	<u>'</u>	1300	_	_	1.51	2.07	2.42	1.25	_		1.40	1.94	1.94	0.96		
SDS25200	½ x 2	11/4	1300	_	_	400	530	655	355	_	_	370	455	455	270		
SDS25200SS	74 X Z	1 74	1 /4	1300	_	_	1.78	2.36	2.91	1.58	_	_	1.65	2.02	2.02	1.20	
SDS25212	1/4 x 21/2	11/2	1100	_	_	465	590	825	425	_	_	420	550	590	320		
SDS25212SS	74 X Z 72	I 1/2	1 /2	1 /2	1100	_		2.07	2.62	3.67	1.89	_	_	1.87	2.45	2.62	1.42
SDS25300	½ x 3	2	950	370	_	525	655	840	565	320		475	590	590	430		
SDS25300SS	74 / 0	3 2	2	330	1.65	_	2.34	2.91	3.74	2.51	1.42		2.11	2.62	2.62	1.91	
SDS25312	1/4 x 31/2	21/4	900	435	435	585	715	840	635	370	370	525	590	590	485		
SDS25312SS	/4 X 3 /2	Z /4	300	1.94	1.94	2.60	3.18	3.74	2.82	1.65	1.65	2.34	2.62	2.62	2.16		
SDS25412	1/4 x 41/2	23/4	800	475	510	585	720	840	775	420	450	530	590	590	590		
30323412	/4 X 4 /2	274	000	2.11	2.27	2.60	3.20	3.74	3.45	1.87	2.00	2.36	2.62	2.62	2.62		
SDS25500	1/4 x 5	23/4	500	475	510	585	720	840	775	420	450	530	590	590	590		
30323300	74 X J	274	300	2.11	2.27	2.60	3.20	3.74	3.45	1.87	2.00	2.36	2.62	2.62	2.62		
SDS25600	1/4 x 6	31/4	600	475	510	585	720	840	915	420	450	530	590	590	700		
30323000	74 X U	J 74	000	2.11	2.27	2.60	3.20	3.74	4.07	1.87	2.00	2.36	2.62	2.62	3.11		
SDS25800	1/4 x 8	31/4	400	475	510	585	720	840	915	420	450	530	590	590	700		
00023000	/4 X U	0 /4	700	2.11	2.27	2.60	3.20	3.74	4.07	1.87	2.00	2.36	2.62	2.62	3.11		

- 1. Factored resistances shown have been developed in accordance with 12.11 CSA 086-14. Apply the adjustment factors K_D , K_{SF} and K_T as per 12.11.4.1 CSA 086-14 when applicable.
- 2. Factored lateral resistances shown assume steel side plates with a minimum $F_{IJ} = 45,000 \; \text{psi} \; (310 \; \text{MPa}).$
- 3. Factored lateral resistances shown assume full penetration into the main member.

 4. Factored withdrawal resistances shown are applicable to short term loads, reduce
- Factored withdrawal resistances shown are applicable to short term loads, reduce for other load durations where applicable.
- 5. Factored withdrawal resistances shown assume the entire threaded portion of the screw is installed into the main member. Where the penetration into the main member is less than the length of the thread, the factored resistance may be calculated by multiplying the length of penetration of the threads x 2.0 bs/in
- (49 N/mm) for D.Fir-L and 215 lbs/in (38 N/mm) for S-P-F).
- Factored withdrawal resistances shown are for penetration into the main member. Head pull through resistance may govern and must be calculated in accordance with 12.11.5.3 CSA 086-14 using a washer diameter d_w = 0.480°.
- 7. LSL wood-to-wood applications that require 4½", 5", 6" or 8" Strong-Drive SDS Heavy-Duty Connector screws are limited to interior-dry use only.
- Minimum spacing, edge and end distances shall be in accordance with 12.9.2.1 CSA 086-14 using a fastener diameter of 0.250" (6.4 mm).
- 9. Screws may be provided with the 4CUT or Type 17 tip.
- 0- Strong-Drive® SDS Heavy-Duty Connector screws install best with a low speed 1/8" drill with a '6" hex head driver.

STRONG-DRIVE® SDW TRUSS-PLY & EWP-PLY Screws

The Simpson Strong-Tie® Strong-Drive® SDW Truss-Ply and EWP-Ply screws are the 0.22" diameter, high-strength structural wood screws specifically designed for fastening together multi-ply wood members, such as plated trusses, engineered-lumber products and solid-sawn lumber. Strong-Drive SDW Truss-Ply and EWP-Ply screws install easily with no pre-drilling and are available in optimized lengths for fastening 2, 3 and 4-ply trusses or 1¾" structural composite lumber (SCL). The Strong-Drive SDW Truss-Ply and EWP-Ply screws enable single-side fastening, while still allowing concurrent loading on both sides of the assembly.

- Low-profile head for reduced interference during handling or installation of hardware on the assembly
- · High shear values enable wider screw spacing
- Bold thread design firmly cinches plies together to close gaps in multi-ply assemblies
- Optimal screw lengths provide maximum penetration

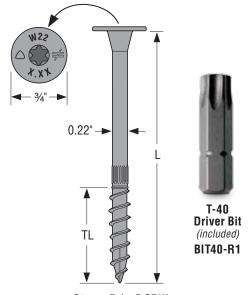
MATERIAL: Heat-treated carbon steel

FINISH: Black E-coat™ and double-barrier coating

WARNING: Industry studies show that hardened fasteners can experience performance problems in wet or corrosive environments. Accordingly, the Strong-Drive SDW Truss-Ply and EWP-Ply screws should only be used in dry, interior and non-corrosive environments.

INSTALLATION: • See General Notes.

- Strong-Drive Truss-Ply and EWP-Ply screws install best with a low-speed ½" drill and a T-40 6-lobe bit. The matched bit included with the screws is recommended for best results.
- Pre-drilling is typically not required. Strong-Drive SDW Truss-Ply and EWP-Ply screws may be installed through metal truss plates as approved by the Truss Designer (pre-drilling required through the plate using a maximum of ⁵/₂₂" bit).
- Screw heads that are countersunk flush to the wood surface are acceptable
 if the screw has not spun out.



Strong-Drive® SDW TRUSS-PLY and EWP-PLY Screw

U.S. Patents 5,897,280; 7,101,133 and patent pending

Product Information

Model No. ^{2,3}	Head Stamp Length	Nominal Length (L) (in)	Typical Application ¹	Thread Length (TL) (in)	Retail Box³ Quantity (1 Bit)	Retail Boxes/ Carton	Mini-Bulk Bucket Quantity ² (1 Bit)	Bulk Bucket Quantity (2 Bits)
SDW22300	3.00	2 ¹⁵ / ₁₆	2x/Truss	1 ½16	50	6	250	950
SDW22338	3.37	3%	SCL	1%16	50	6	250	900
SDW22458	4.62	45/8	2x/Truss	17/16	50	4	200	600
SDW22500	5.00	5	SCL/3x2PCT	1%6	50	4	200	600
SDW226004	6.00	6	2x/Truss	1 ½16	50	4	200	500
SDW22638 ⁴	6.37	6%	2x/Truss	1 ½16	50	4	200	500
SDW22634	6.75	6¾	SCL/4x2PCT	1%6	50	4	200	500

- Typical screw application key:
 2x/Truss = Solid-sawn dimensional lumber and plated wood trusses.
 SCL = 1%* plies of structural-composite lumber.
- SCL/3x2PCT = 1¾" plies of structural-composite lumber or double 3x2 parallel-chord trusses. SCL/4x2PCT = 1¾" or 3½" plies of structural-composite lumber or double 4x2 parallel-chord trusses.
- 2. To order mini-bulk buckets add the letters MB to the model number, e.g. SDW22458MB.
- To order retail pack boxes add "-R50" to the model number, e.g. SDW22458-R50.
- 4. If assembly is less than or equal to 63/16" thick, use the Strong-Drive SDW22600.

				D.Fir-L		S-P-F			
	Nominal	Thread		d Lateral (K _D = 1.00)	Factored Withdrawal		l Lateral (K _D = 1.00)	Factored Withdrawal	
Model No.	Length	Length	Wood Side Member		Resistance	Wood Sid	Resistance		
NU.	(in)	(in)	1½"	1¾" SCL	$(K_D = 1.15)$	1½"	1¾" SCL	$(K_D = 1.15)$	
			lbs	lbs	lbs	lbs	lbs	lbs	
			kN	kN	kN	kN	kN	kN	
SDW22300	3	1 ½16	335	_	485	290	_	370	
3DW22300	3	1716	1.49	_	2.16	1.29	_	1.65	
SDW22338	3%	1%6	335	390	530	290	325	405	
3DW22330	378	I 716	1.49	1.73	2.36	1.29	1.44	1.80	
SDW22438	43/8	1 ½6	455	_	485	405	_	370	
3DW22430	478	1716	2.02	_	2.16	1.80	_	1.65	
SDW22458	45/8	1 ½16	455	_	485	405	_	370	
3DW22430	478	1 /16	2.02	_	2.16	1.80	_	1.65	
SDW22500	5	1 %16	455	495	530	405	430	405	
3DW22300	3	1716	2.02	2.20	2.36	1.80	1.91	1.80	
SDW22600	6	1 7/16	455	_	485	405	_	370	
3DW22000	0	1716	2.02	_	2.16	1.80	_	1.65	
SDW22638	63%	17/16	455	_	485	405	_	370	
3DW22030	U78	I /16	2.02	_	2.16	1.80	_	1.65	
SDW22634	63/4	1%6 🔼	455	495	530	405	430	405	
3DW22034	094	1 716	2.02	2.20	2.36	1.30	1.91	1.80	

See pages 152-153 for specific multi-ply lamination details using Strong-Drive SDW Truss-Ply screws.

- 1. Factored resistances shown have been developed in accordance with 12.11 CSA 086-14 based on testing per ICC-ES AC233. Apply the adjustment factors $K_D,\,K_{SF}$ and K_T as per 12.11.4.1 when applicable.
- 2 Factored withdrawal resistances shown are applicable to short term loads, reduce where other load durations govern.
- 3. Factored withdrawal resistances shown assume the entire threaded portion of the screw is installed into the main member. Where the penetration into the main member is less than the length of the thread, the factored resistances may be calculated by multiplying the length of penetration of the threads x 280 lbs/in (49 N/mm) for D.Fir-L and 215 lbs/in (38 N/mm) for S-P-F.
- Minimum spacing, edge and end distances shall be in accordance with 12.9.2.1 CSA 086-14 using a diameter value of 0.30".

QUIK DRIVE® FASTENERS AND ATTACHMENTS





QUIK DRIVE® AUTO-FEED SCREW DRIVING SYSTEMS

The systems offer several easy-to-use attachments bringing speed and reliability to applications that require the fastening power of screws. Our attachments provide tough, reliable performance in specific fastening applications.

FASTENERS FOR QUIK DRIVE® SYSTEMS

Featuring patented collation technology, these fasteners are designed to meet or exceed industry standards for strength and longevity while offering easy-to-load, tangle-free strips for efficient performance in auto-feed systems.



ANCHORING SYSTEMS



Concrete Anchors Designed to Meet Annex D of CSA A23.3



Building Codes Have Changed – New Anchor Designs Now Required

Most Canadian provinces have adopted either the 2009 or 2014 version of the CSA Standard A23.3 – Design of Concrete Structures. As a result, designers are increasingly being required to specify anchors designed and tested to meet the requirements of CSA A23.3 Annex D, utililizing a true Limit States Design methodology. This affects how and when post-installed concrete anchors are specified and what products are suitable for use.

When designing concrete anchorages, designers must consider whether conditions exist that may cause the concrete to crack. If it's determined such conditions do exist, anchors designed and tested for cracked concrete must be specified. If determined that there is no risk of concrete cracking, the designer may choose to specify anchors approved for use in uncracked concrete. In either case, CSA A23.3 Annex D requires anchors to be tested in accordance with ACI 355.2 or ACI 355.4 and no longer supports designs soft converted from Working Stress Design (Allowable Stress Design) methodology.

Common conditions that cause cracking of concrete:

- Concrete in tension such as in the underside of a slab
- Concrete elements located in areas prone to seismic activity
- Other factors that contribute to cracking include:
 - External short term loads (such as high winds)
 - Temperature variations
 - Shrinkage during curing

Software Tools to Help You Select the Right Products



Anchor Designer™



Adhesive Cartridge Estimator

For more information, visit www.strongtie.com/software

Simpson Strong-Tie has, for years, been at the forefront of developing and testing anchors for use in both cracked and uncracked concrete. In fact, one of our test labs was the first lab in North America to be accredited in testing for cracked concrete. With our technical expertise and support you can rely on us to be the trusted source for concrete anchors.



ANCHORING SYSTEMS For Cracked-Concrete Applications



SET-XP® High-Strength Anchoring Adhesive





A two-part, high-strength epoxy anchoring adhesive system formulated for threaded rod and rebar anchoring into concrete (cracked/uncracked) and masonry, SET-XP is a teal color when mixed, providing easy post-installation identification.

FEATURES:

- Qualified under ICC-ES AC308 regarding elevated temperature and long-term sustained loading conditions
- Code-listed for cracked and uncracked concrete per ICC-ES ESR-2508
- Code-listed for masonry per IAPMO UES ER-265
- Suitable for use under static and seismic loading conditions in cracked and uncracked concrete
- Multiple DOT listings; refer to www.strongtie.com/DOT
- · Manufactured in the USA using global materials

CODES: ICC-ES ESR-2508 (concrete); IAPMO UES ER-265 (masonry); City of L.A RR25744 (concrete); City of L.A. RR25965 (masonry); Florida FL15730 (concrete), FL16230 (masonry); ASTM C 881 (Type I and IV, Grade 3, Class C); NSF/ANSI Standard 61 (216 in²/1000 gal)

AT-XP® High-Strength, Fast-Cure, All-Weather Anchoring Adhesive





Formulated for high-strength anchorage of threaded rod and rebar into concrete (cracked/uncracked) and masonry under a wide range of conditions, AT-XP dispenses easily in cold or warm environments and, when mixed, is a dark teal color for easy post-installation identification.

FEATURES:

- Qualified under ICC-ES AC308 regarding reduced temperature, elevated temperature, and long-term sustained loading conditions
- Code-listed for cracked and uncracked concrete per IAPMO UES ER-263
- Code-listed for masonry per IAPMO UES ER-281
- Fully cures in temperatures as low as 14°F (-10°C)
- Suitable for use under static and seismic loading conditions in cracked and uncracked concrete
- · Manufactured in the USA using global materials

CODES: IAPMO UES ER-263 (concrete), ER-281 (masonry); City of L.A. RR25960 (concrete), RR25966 (masonry); Florida FL 16230, NSF/ANSI Standard 61 (43.2 in²/1000 gal)

ET-HP® Anchoring Adhesive





A two-part epoxy anchoring adhesive system formulated for threaded rod and rebar anchoring into concrete (cracked/uncracked) and masonry.

FEATURES:

- Qualified under ICC-ES AC308 regarding elevated temperature and long-term sustained loading conditions
- Code-listed for cracked and uncracked concrete per ICC-ES ESR-3372
- Code-listed for uncracked masonry per IAPMO UES ER-241
- Suitable for use under static and seismic loading conditions in cracked and uncracked concrete
- Multiple DOT listings; refer to www.strongtie.com/DOT
- · Manufactured in the USA using global materials

CODES: ICC-ES ESR-3372 (concrete); ICC-ES ESR-3638 (unreinforced masonry); IAPMO UES ER-241 (masonry); City of L.A. RR25120 (unreinforced masonry); ASTM C 881 (Type I, II, IV and V, Grade 3, Class B and C, except gel time)

STRONG-BOLT® 2 Wedge Anchor





A wedge-type expansion anchor designed for optimum performance in cracked and uncracked concrete and uncracked masonry, the Strong-Bolt 2 is available in carbon-steel (¼" through 1" diameter) and Type 316 stainless steel (¼" through ¾" diameter).

FEATURES:

- Code-listed for cracked and uncracked concrete per ICC-ES ESR-3037
- Code-listed for masonry per IAPMO UES ER-240
- Qualified for static and seismic loading conditions

CODES: ICC-ES ESR-3037(concrete); IAPMO UES ER-240 (carbon steel in CMU); City of L.A. RR25891 (concrete), RR25936 (carbon steel in CMU); Florida FL 15731; UL File Ex3605; FM 3043342 and 3047639; meets the requirements of Federal Specifications A-A-1923A, Type 4

TORQ-CUT[™] Self-Undercutting Anchor



A heavy-duty, high-capacity, self-undercutting anchor designed for cracked and uncracked concrete applications, the Torq-Cut has a built-in cutting ring that provides superior load-carrying capacity.

FFATURES

- Code-listed for cracked and uncracked concrete per ICC-ES ESR-2705
- Provides higher load-carrying capacity that conventional mechanical anchors
- Excellent for resisting seismic (SDC A-F) and vibratory loads
- No special installation tools required

CODES: ICC-ES ESR-2705 (concrete); City of L.A. RR25946 (concrete); Florida FL15731





TITEN HD® Concrete Screw





The original, patented, high-strength screw anchor that offers industry-leading performance in cracked and uncracked concrete and uncracked masonry, the Titen HD installs with low installation torque for maximum efficiency.

FEATURES

- Code-listed in accordance with ICC-ES AC193 for cracked and uncracked concrete per ICC-ES ESR-2713; includes Titen HD® Rod Hanger (models THD37212RH and THD50234RH only)
- Code-listed in accordance with ICC-ES AC106 for masonry per ICC-ES ESR-1056
- Qualified for static and seismic loading conditions
- Standard fractional sizes; no special drill bits required
- · Removable; ideal for temporary anchoring applications like formwork or bracing

CODES: ICC-ES ESR-2713 (concrete), ICC-ES ESR-1056 (masonry); City of L.A. RR25741 (concrete), RR25560 (masonry); Florida FL 15730; FM 3017082, 3035761 and 3043442

ANCHORING SYSTEMS For General-Purpose Applications

SET® Anchoring Adhesive



A non-shrink, epoxy based anchoring system, SET is formulated for anchoring threaded rod and rebar in a wide range of base materials.

FEATURES:

- Code-listed for URM per ICC-ES ESR-1772
- Excellent as a pick-proof sealant around doors, windows and fixtures
- CalTrans and multiple DOT listings; refer to www.strongtie.com/DOT
- Manufactured in the USA using global materials

CODES: ICC-ES ESR-1772 (URM); City of L.A. RR25279 (masonry); Florida FL 15730; ASTM C 881 (Type I, II and IV, Grade 3, Class B and C); NSF/ANSI Standard 61 (216 in²/1000 gal) – except SET 1.7KTA

AT Fast-Cure, All-Weather Anchoring Adhesive



An acrylic-based adhesive anchoring system, AT is formulated for use as a high-strength anchoring material for threaded rod and rebar in a wide range of temperatures and base materials.

FFATURES.

- Code-listed for URM per ICC-ES ESR-1958
- Fully cures in temperatures as low as 0°F (-18°C)
- Multiple DOT listings; refer to www.strongtie.com/DOT
- Manufactured in the USA using global materials

CODES: ICC-ES ESR-1958 (URM); Florida FL 14832; ASTM C 881 (Type I and IV, Grade 3, Class A, B and C – except AT is a non-epoxy formulated for fast cure time); NSF/ANSI Standard 61 (2.2 in²/1000 gal)

WEDGE-ALL® Wedge Anchor



A non-bottom bearing, wedge-style expansion anchor for use in solid concrete or grout-filled masonry, the Wedge-All is available in carbon steel, mechanically galvanized, and Types 303/304/316 stainless steel.

FEATURES:

- Code-listed for CMU per ICC-ES ESR-1396
- · One-piece, wrap-around clip ensures uniform holding capacity
- · Chamfered thread end for ease of starting nut
- · Wide range of diameters and lengths for added versatility

CODES: ICC-ES-ESR-1396 (CMU); City of L.A. RR24682 (CMU); Florida FL 15730; FM 3017082 and 3131136; UL File Ex3605; meets the requirements of Federal Specifications A-A-1923A, Type 4.

Note: The Tie-Wire Anchor is not code-listed.

Titen HD® Heavy-Duty Screw Anchor for Cracked and Uncracked Concrete

The Titen HD® anchor is a patented, high-strength screw anchor for concrete and masonry. It is designed for optimum performance in both cracked and uncracked concrete. The high strength, easy to install Titen HD anchor has been tested and shown to provide outstanding performance in cracked and uncracked concrete under both static and seismic loading conditions. The self-undercutting, non-expansion characteristics of the Titen HD anchor make it ideal for structural applications, even at reduced edge distances and spacings. Recommended for permanent dry, interior non-corrosive environments or temporary outdoor applications.

PERFORMANCE FEATURES:

- Tested per AC193 to ensure outstanding performance in both cracked and uncracked concrete.
- Higher load capacity and vibration resistance: Threads along the length of the anchor undercut the concrete and efficiently transfer the load to the base material.
- Vibration and shock resistance: The mechanical interlock of the threads and the ratchet teeth on the underside of the head help prevent the anchor from loosening in vibratory conditions. The Titen HD anchor has been tested to 12.6 million vibratory cycles with no performance reductions.
- Specialized heat treating process: Creates superior surface hardness at the tip to facilitate cutting, while at the same time not compromising ductility within the anchor body.
- Less spacing and edge distance required: The anchor does not exert expansion forces on the base material.
- Easy post-installation inspection: The head is stamped with the Simpson Strong-Tie[®] "≠" sign
 and the anchor length in inches.

INSTALLATION FEATURES:

- No special drill bit needed: Designed to install using standard sized ANSI tolerance drill bits.
- Installs with 50% less torque: Testing shows that when compared to competitors, the Titen HD requires 50% less torque to be installed in concrete.
- Hex-washer head: Requires no separate washer and provides a clean installed appearance.*
- Removable: Ideal for temporary anchoring (e.g. formwork, bracing) or applications where fixtures
 may need to be moved. Re-use of the anchor to achieve listed load values is not recommended.

MATERIAL: Carbon steel, heat treated

FINISH: Zinc plated or mechanically galvanized

CODES: ICC-ES ESR-2713 (concrete); ICC-ES ESR-1056 (CMU); City of L.A. RR25741(concrete); City of L.A. RR25560 (CMU); Florida FL 11506.7; Factory Mutual 3017082, 3035761 and 3043442.

TEST CRITERIA: The Titen HD® anchor has been tested in accordance with ICC-ES AC193, ACI 355.2 and ICC-ES AC106 for the following:

- · Static tension and shear loading in cracked and uncracked concrete
- · Seismic and wind loading in cracked and uncracked concrete
- · Performance in uncracked masonry

Anchor Fatigue Testing: Tested in accordance with ASTM E 488 for the effects of fatigue. 25% of the average ultimate load was applied to the anchor for 2 million cycles at a frequency of 15 Hz. Subsequent load tests showed no reduction in ultimate tension capacity.

Vibratory Load Testing: A 150 lb. concrete block was suspended from a %" diameter anchor embedded at 1½" and vibrated for 12.6 million cycles at a frequency of 30 Hz and an amplitude of 0.0325 inches. Subsequent load test showed no reduction in ultimate tension capacity. Field Testing: For guidance on field testing see technical bulletin T-SAS-THDINSP.

INSTALLATION: Holes in metal fixtures to be mounted should be 1/6" to 3/16" larger than the anchor diameter.

- Caution: Oversized holes in the base material will reduce or eliminate the mechanical interlock of the threads with the base material and will reduce the anchor's load capacity. Use a Titen HD screw anchor one time only. Installing the anchor multiple times may result in excessive thread wear and reduce load capacity.
- Drill a hole in the base material using a carbide drill bit the same diameter as the nominal diameter of the anchor to be installed. Drill the hole to the specified embedment depth plus ½" minimum to allow the thread tapping dust to settle and blow it clean using compressed air. Overhead installations need not be blown clean. Alternatively, drill the hole deep enough to accommodate embedment depth and dust from drilling and tapping.
- Insert the anchor through the fixture and into the hole.
- Tighten the anchor into the base material until the hex washer head contacts the fixture.
- Do not use impact wrenches to install into hollow CMU.

SUGGESTED SPECIFICATIONS: Screw anchors shall have 360-degree contact with the base material and shall not require oversized holes for installation. Fasteners shall be manufactured from carbon steel, and are heat-treated. Anchors shall be zinc plated in accordance with ASTM B633 or mechanically galvanized in accordance with ASTM B695. Anchors are not to be reused after initial installation. Screw anchors shall be Titen HD® anchors from Simpson Strong-Tie, Pleasanton, CA. Anchors shall be installed per the Simpson Strong-Tie instructions for the Titen HD anchor.

*Some jurisdictions require an additional square plate washer for sill plate applications See current Anchor Systems catalogue C-SAS for more information







Serrated teeth on the tip of the Titen HD® screw anchor facilitate cutting and reduce installation torque.



Titen HD® Screw Anchor U.S. Patent 5.674.035 &

6.623.228

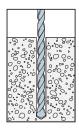


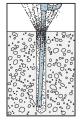
Suitable for use in place of code anchor bolts.

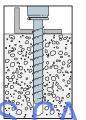


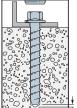
Longer ½" diameter
Titen HD anchors achieve
sufficient embedment
depth to develop tension
capacities equal to many
Simpson Strong-Tie
holdowns that specify a
% " diameter anchor.
Testing has been
conducted to assure
compatibility of these
holdowns' anchor holes
with the ½" Titen HD
screw anchor.

Installation Sequence









‡ 1⁄⁄2" mir

Titen® screws are 3/16" and 1/4" diameter hardened screws for attaching all types of components to concrete and masonry. Available in hex and phillips head designs in three colors. Use with appropriately sized Titen drill bits included with each box.

Warning: Industry studies show that hardened fasteners can experience performance problems in wet or corrosive environments. Accordingly, use these products in dry, interior and non-corrosive environments only.

MATERIAL: Heat-treated carbon steel

FINISH: Zinc plated with a baked on ceramic coating

INSTALLATION:

Caution: Industry studies show that hardened fasteners can experience performance problems in wet or corrosive environments. Steps must be taken to prevent inadvertent sustained loads above the listed allowable loads. Overtightening and bending moments can initiate cracks detrimental to the hardened screw's performance. Use the Simpson Strong-Tie installation tool kit as it has a bit that is designed to reduce the potential for overtightening the screw.

Caution: Oversized holes in the base material will reduce or eliminate the mechanical interlock of the threads with the base material and will reduce the anchor's load capacity.

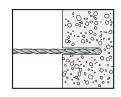
- · Drill a hole in the base material using the appropriate diameter carbide drill bit as specified in the table. Drill the hole to the specified embedment depth plus 1/2" to allow the thread tapping dust to settle and blow it clean using compressed air. Overhead installations need not be blown clean. Alternatively, drill the hole deep enough to accommodate embedment depth and dust from drilling and tapping.
- Position fixture, insert screw and tighten using drill and installation tool fitted with a hex socket or phillips bit.

Preservative-treated wood applications: Suitable for use in non-ammonia formulations of CCA, ACQ-C, ACQ-D, CA-B, SBX/DOT and zinc borate. Use in dry, interior environments only. Use caution not to damage ceramic barrier coating during installation. Recommendations are based on testing and experience at time of publication and may change. Simpson Strong-Tie cannot provide estimates on service life of screws. Contact Simpson Strong-Tie for additional information.

See current Anchor Systems catalogue C-SAS for more information.

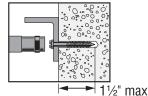


Installation Sequence









Titen® Tension and Shear Load Values in Normal-Weight Concrete

Titen	Drill Bit	Embed.	Critical	Critical	Tension Load				Shear Load		
Dia. in.	Dia. in.	Depth in.	Spacing in.	Edge Dist.		000 psi 1) Concrete	f' ≥ 4000 psi (27.6 MPa) Concrete		f' ≥ 2000 psi (13.8 MPa) Concrete		
(mm)		(mm)	(mm)	in. (mm)	Ultimate lbs. (kN)	Allowable lbs. (kN)	Ultimate lbs. (kN)	Allowable lbs. (kN)	Ultimate lbs. (kN)	Allowable lbs. (kN)	
3/16	5/32	1	2 1/4	1 ½	500	125	640	160	1,020	255	
(4.8)		(25.4)	(57.2)	(28.6)	(2.2)	(0.6)	(2.8)	(0.7)	(4.5)	(1.1)	
3/16	5/32	1 ½	2 ½	1 1/8	1,220	305	1,850	460	1,670	400	
(4.8)		(38.1)	(57.2)	(28.6)	(5.4)	(1.4)	(8.2)	(2.0)	(7.4)	(1.8)	
1/4	3/16	1	3	1 ½	580	145	726	180	900	225	
(6.4)		(25.4)	(76.2)	(38.1)	(2.6)	(0.6)	(3.2)	(0.8)	(4.0)	(1.0)	
1/ ₄	3⁄16	1 ½	3	1 ½	1,460	365	2,006	500	1,600	400	
(6.4)		(38.1)	(76.2)	(38.1)	(6.5)	(1.6)	(8.9)	(2.2)	(7.1)	(1.8)	

- 1. The tabulated allowable loads are based on a factor of safety of 4.0.
- 2. Maximum anchor embedment is 1 1/2" (38.1 mm).
- 3. Concrete must be minimum 1.5 x embedment.
- 4. Loads shown are for Allowable Stress Design methodology.

Titen® Phillips head screw available in white and standard blue

Titen® Tension and Shear Load Values in Face Shell of Hollow and Grout-Filled CMU

Titen Dia.	Drill Bit Dia.	Embed. Depth	Critical Spacing	Critical Edge			8" Lightweight, Normal-Weight CMU		
in.	in.	in.	in.	Dist.	Tension Load		Shear Load		
(mm)	,	(mm)	(mm)	in. (mm)	Avg. Ult. Ibs. (kN)	Allow. lbs. (kN)	Avg. Ult. Ibs. (kN)	Allow. lbs. (kN)	
3/16 (4.8)	5/32	1 (25.4)	2½ (57.2)	1 1/8 (28.6)	542 (2.4)	110 (0.5)	1,016 (4.5)	205 (0.9)	
1/ ₄ (6.4)	3⁄16	1 (25.4)	3 (76.2)	1 ½ (38.1)	740 (3.3)	150 (0.7)	1,242 (5.5)	250 (1.1)	

- 1. The tabulated allowable loads are based on a safety factor of 5.0.
- Maximum anchor embedment is 1 ½ (38.1 mm).
- 3. Loads shown are for Allowable Stres

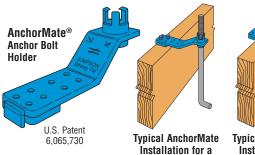
ANCHORMATE® Anchor Bolt Holders

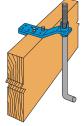
These reusable anchor bolt holders are designed to hold the anchor in place before the concrete pour, as required in some jurisdictions. The gripping section secures the bolt in place without a nut for quicker set up and tear down. It also protects the threads from wet concrete and simplifies trowel finishing.

- Built-in 2x4 and 2x6 stops eliminate measuring.
- Color-coded for easy size identification.
- Use the %" and %" AnchorMate to secure the SSTB to the formboard before the concrete pour. Alignment arrows (left or right) match the SSTB bolt head arrow.

MATERIAL: Nylon

Model No.	Dia. (in)	Color
AM½	1/2	Yellow
AM%	5/8	Blue
AM¾	3/4	Red
AM7/8	7/8	Green
AM1	1	Black





Typical AnchorMate Installation for a 2x4 Mudsill

ABS Anchor Bolt Stabilizer

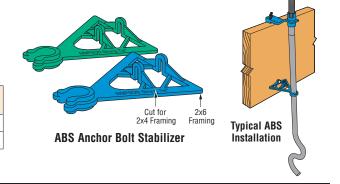
The ABS stabilizes the anchor bolt to prevent it from being pushed against the form during the concrete pour.

FEATURES:

- Supports the bolt approx. 8" below the top of the concrete.
- Model ABS% is for the %" SSTB and ABS% is for the %" SSTB.
- Thin section limits the effect of a cold joint
- · Sized for 2x4 and 2x6 mudsills.

MATERIAL: Engineered Composite Plastic.

Model No.	Dia. (in)	Color
ABS%	5/8	Blue
ABS%	7/8	Green



2x6 Mudsill

STRAPMATE® Strap Holder

The StrapMate is designed to keep the STHD and LSTHD straps vertically aligned during the concrete pour to minimize possibility of spalling. The friction fit allows for quick and easy installation.

- The StrapMate is reusable.
- · Works with STHD and LSTHD.

MATERIAL: Engineered Composite Plastic.

- Designed to fit ¾" plywood forms up to 1¾" LVL forms and larger.
- The strap is positioned off the front edge of the form board.

SM1 2-8d Duple	X



ABL Anchor Bolt Locator

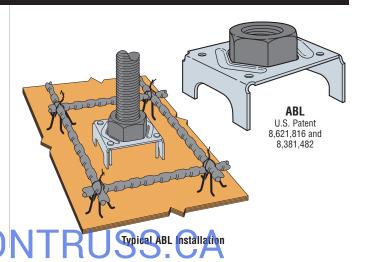
The ABL enables the accurate and secure placement of anchor bolts on concrete-deck forms prior to concrete placement. The structural heavy-hex nut is attached to a pre-formed steel "chair", which eliminates the need for an additional nut on the bottom of the anchor bolt. Electro-galvanized versions available for HDG anchor bolts. Order ABL-OST when using HDG anchor bolts.

FEATURES:

- Designed for optimum concrete flow
- Installed with nails or screws
- Meets code requirement for 1" stand off. Also available with 1½" standoff. Order ABLXX-1.5.
- PAB anchors are not designed for use with the ABL. Contact Simpson Strong-Tie for pre-assembled anchor solutions to be used with ABL.

MATERIAL: Nut - Heavy hex, Chair - Steel FINISH: Nut - None or Electro-galvanized; Chair - G90; ABL-OST - Hag

Model No.	Anchor Bolt Dia. (in)
ABL4-1	1/2
ABL5-1	5/8
ABL6-1	3/4
ABL7-1	7/8
ABL8-1	1
ABL9-1	11/8
/ A DI 10-1	11/



Bearing plates give greater bearing surface than standard cut washers, and help distribute the load at these critical connections.

The BPS and LBPS are 3"x3" bearing plates that offer increased flexibility. The slotted hole allows for adjustability to account for bolts that are not in the middle of the sill plate.

The BP%SKT uses ¼"x1½" Strong-Drive® SDS Heavy-Duty Connector screws to provide lateral resistance when 5%" diameter sill holes are overdrilled (screws are provided). The shear capacity of the connection and the sill/anchor bolt shall be determined by the Designer for each installation.

MATERIAL: See table

FINISH: LBP, LBPS & BP5/S—Galvanized; BP7/s-2—Zinc Plated; BPS, BP—None. BP's and BPS's may be ordered HDG; LBP and LBPS products may be ordered ZMAX®; contact Simpson Strong-Tie. Refer to pages 14-17 for Corrosion Information.

INSTALLATION: See General Notes.

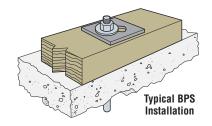
These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

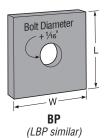
Model	Thickness	Dimens	ions (in)	Bolt Dia.
No.	Tillckiless	W	L	(in)
LBP½	%4	2	2	1/2
LBP5/8	9/64	2	2	5/8
LBPS½	9/64	3	3	1/2
LBPS%	%4	3	3	5/8
BPS½-3	3 ga	3	3	1/2
BPS ½-6	3 ga	3	41/2	1/2
BPS%-3	3 ga	3	3	5/8
BPS %-6	3 ga	3	41/2	5/8
BP%-2	3/16	2	2	3/8
BP½	3/16	2	2	1/2
BP½-3	3 ga	3	3	1/2
BP5/8-2	3/16	2	2	5/8
BP%SKT	3 ga	4	2	5/8
BP5/8	1/4	21/2	21/2	5/8
BP%-3	3 ga	3	3	5/8
BP¾	5/16	2¾	2¾	3/4
BP¾-3	3 ga	3	3	3/4
BPS¾-3	3 ga	3	3	3/4
BPS ¾-6	3 ga	3	41/2	3/4
BP7/8-2	3/8	1 15/16	21/4	7/8
BP%	5/16	3	3	7/8
BP1	3/8	3½	3½	1

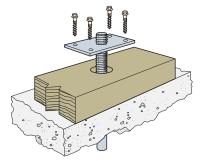
- 1. BP%SKT sold as a kit.
- 2. Standard cut washer required with BPS½-3, BPS ½-6, BPS¾-3, BPS ⅓-6, BPS¾-3, and BPS ¾-6 (not provided).



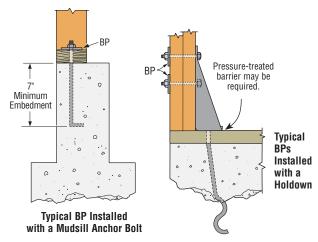








The BP%SKT is used when 5/8" diameter sill bolt holes are overdrilled



KST Speed Wall Ties (Kwik Strip)

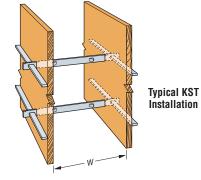
MATERIAL: 16 gauge FINISH: none

Model No.	Wall Thickness W (in)	Length L (in)
KST6	6	101//8
KST8	8	121/8
KST10	10	141/8
KST12	12	161//8





Formwork designer to specify tie spacing and concrete pour rate to ensure that Factored Resistances are not exceeded.



CNW/HSCNW Coupler Nuts

SIMPSON
Strong-Tie

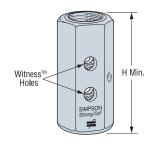
Simpson Strong-Tie® coupler nuts are a tested and load-rated method to join threaded rod and anchor bolts. "Witness" holes in the nut provide a means to verify when rods are properly installed. The positive stop feature helps ensure even threading into each end of the nut. CNWs meet and exceed the tensile capacity of corresponding ASTM A36 bolts and threaded rod. HSCNWs meet and exceed the tensile capacity of corresponding ASTM A449 bolts and threaded rod. Contact Simpson Strong-Tie for other coupler nut sizes.

FINISH: Zinc Plated

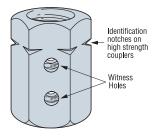
INSTALLATION:

- Tighten the two rods until each all-thread rod is visible in the witness hole.
- For non-hot-dip galvanized all-thread rod only.
- 5%" and 7%" diameter couplers available with oversized threads for installation to hot-dip galvanized bolts (order CNW5%-5% OST and CNW7%-7% OST).

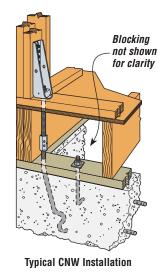
Model No.	Rod Diameter (in)	H Min. (in)		
CNW½	0.500	1½		
CNW%	0.625	17/8		
CNW¾	0.750	21/4		
CNW%	0.875	2½		
CNW1	1.000	2¾		
CNW11/4	1.250	3		
HSCNW¾	0.750	21/4		
HSCNW1	1.000	2¾		
Transition Couplers				
CNW5/8 -1/2	0.625 to 0.500	1½		
CNW¾ - 5/8	0.750 to 0.625	1¾		
CNW7/8 - 5/8	0.875 to 0.625	2		
CNW1-7/8	1.000 to 0.875	21/4		

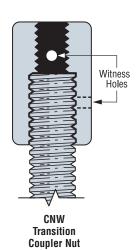


CNW
Allows fast visual check for correct all thread rod installation



HSCNW High Strength Coupler Nut





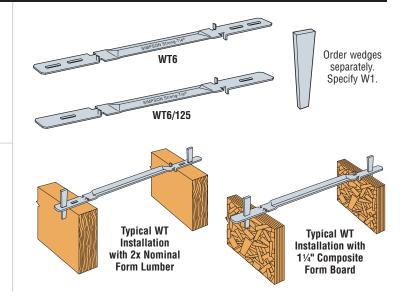
WT Wedge Form Ties

Designed for low foundation wall applications. 5%" wide formed "V" design for rigidity allows accurate form spacing and support. Sizes now available for composite form board.

MATERIAL: Wedges—14 gauge, WT—18 gauge FINISH: Galvanized INSTALLATION:

- Use two 31/2" long wedges for each tie.
- Not recommended for wall pours greater than 4' high.
- Wall thickness from 6" to 12".
- · Refer to technical bulletin T-WT for recommended spacing.

Model No.	Form Board	Wall Thickness (in)	
WT6		6	
WT8	2x Solid	8	
WT10	Sawn	10	
WT12		12	
WT6/125	11/4"	6	
WT8/125	Composite	8	



L-BOLT Anchor Bolts

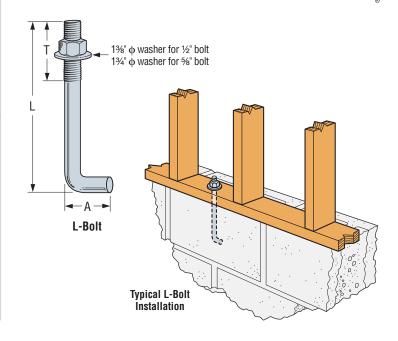
The L-Bolt anchor bolts are used to attach sill plates to concrete or masonry foundations, provide anchorage for light weight post bases and for general anchorage to concrete. The L-Bolt anchor bolts meet the prescriptive requirements of article 9.23.6 of the National Building Code of Canada 2015 (NBC 2015).

MATERIAL: ASTM F1554 Grade 36

FINISH: Unfinished, available in HDG (per ASTM A153)

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

Model No.	Dimensions (in)			
Model No.	Diameter	L	T	Α
LB0LT50600	1/2	6	1½	1½
LB0LT50800	1/2	8	1½	1½
LB0LT50100	1/2	10	1½	1½
LB0LT50120	1/2	12	11/2	11/2
LBOLT62600HDG	5/8	6	3	1%
LB0LT62800	5/8	8	3	1%
LB0LT62100	5/8	10	3	11//8



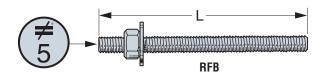
RFB Retrofit Bolts

RFBs are clean, oil free, pre-cut threaded rod, supplied with nut and washer. Offers a complete engineered anchoring system when used with Simpson Strong-Tie® adhesive. Inspection is easy; the head is stamped with rod length and "No Equal" symbol for identification after installation.

MATERIAL: ASTM F1554 Grade 36,

Type 316 stainless steel (RFB#5X8SS only)

FINISH: Zinc Plated (unless otherwise noted), available in HDG (per ASTM A153)



These products feature additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

Model No.	Length L (in)	Bolt Diameter (in)
RFB#4X4	4	1/2
RFB#4X5	5	1/2
RFB#4X6	6	1/2
RFB#4X7	7	1/2
RFB#4X10	10	1/2
RFB#4x8HDG-R	8	1/2
RFB#5X5	5	5/8
RFB#5X8	8	5/8
RFB#5X10	10	5/8
RFB#5X12HDG-R	12	5/8
RFB#5X16	16	5/8
RFB#6X10.5	10½	3/4

- 1. RFB#4X8HDG-R and RFB#5X12HDG-R are only available with a hot-dip galvanized coating. They are retail packaged and are sold 10 per carton.
- 2. Washer provided on all RFB (except RFB#5x8SS).

RP6 Retro Plate

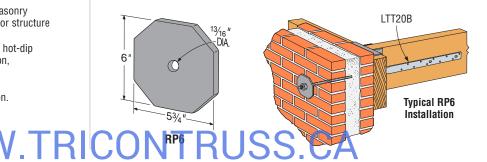
The RP6 retrofit plate fits on the outside of masonry buildings, and helps tie the walls to the roof or floor structure with a 3/4" diameter rod.

 $\textbf{FINISH} : \textbf{Simpson Strong-Tie} \\ \textbf{gray paint. Optional hot-dip}$ galvanized coating; see Corrosion Information, pages 14-17 and specify HDG.

MATERIAL: 3/8" Steel

Available with additional corrosion protection.

Check with Simpson Strong-Tie. INSTALLATION: Use a 3/4" diameter rod.



FAP/FJA/FSA Foundation Anchors

This series is for retrofit or new construction. These products may be used together as a system or in individual applications, designed and tested for earthquake and high wind conditions.

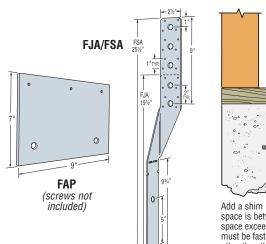
The FAP Plate connects the mudsill to the foundation. Designed to provide lateral load resistance.

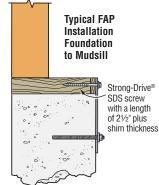
FJA Foundation Joist Anchor nails or bolts directly into floor joist, and provides a direct connection between the foundation and joist. It provides uplift and lateral resistance. FSA Foundation Stud Anchor nails or bolts to floor joist, or nails to stud. Plywood shearwall may require notching with stud-to-foundation installation.

MATERIAL: FAP—7 gauge; all others—12 gauge FINISH: Galvanized; may be ordered HDG, contact Simpson Strong-Tie. See Corrosion Information, pages 14-17.

INSTALLATION: • Use all specified fasteners; see General Notes.

- · Select and install concrete anchor bolts in accordance with the manufacturer's recommendations.
- See Acrylic-Tie® adhesive, page 30 and RFB, page 36.
- Spacing to be specified by the Designer.
- FSA may be bent along bend line up to 20° to accommodate installation. Bend one time only.





Add a shim between plate and sill when space is between 3/16" and 11/2". When space exceeds 11/2" use the UFP. The shim must be fastened to the mudsill by means other than the FAP Strong-Drive® SDS Heavy-Duty Connector screw.

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

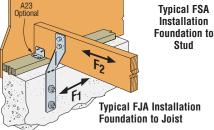
			Fas	teners	F	actored	l Resist	ance (K	D = 1.15)
	Madal	Anch	or Bolt			D.Fir-L		S-P-F		
	Model No.		D:-	Stud/Joist/	Uplift	F ₁	F ₂	Uplift	F ₁	F ₂
	140.	Qty.	Dia. (in)	Plate	lbs	lbs	lbs	lbs	lbs	lbs
L			(,		kN	kN	kN	kN	kN	kN
	FAP	2	1/2	3-1/4" x 21/2" SDS	_	2035	690		1610	520
	IAF	۷	72	3-74 X Z 72 3D3	_	9.05	3.07	_	7.16	2.31
				8-10dx1½	2085	_	_	1480	_	_
	FJA	2	1/2	0-10UX 1 72	9.27	_	_	6.58	_	_
7	FJA	2	72	2-1/2" MB	1805	_	_	1425	_	_
				Z-72 IVID	8.02	_	_	6.33	_	_
					1790	_	_	1270	_	_
	FSA	2	1/2	0-10UX172	7.96			5.65	_	_
	I JA	FSA 2	72	2-1/2" MB	960	_	_	760	60 — —	_
				Z-72 IVID	4.27			3.38	_	_

- 1. Factored resistances have been increased 15% for earthquake or wind loading with
- no further increase allowed; reduce where other load durations govern.

 2. Use the RFB#4x6 with Acrylic-Tie® for the anchorage system.

 3. FAP uses a minimum Strong-Drive® SDS Heavy-Duty Connector screw length of 2½" plus the shim thickness.
- 4. The shim must be fastened to the mudsill by means other than the FAP Strong-Drive SDS screw.
- 5. See page 25 for Strong-Drive SDS Heavy-Duty Connector screw information.
- 6. NAILS: $10dx1\frac{1}{2} = 0.148$ " dia. x $1\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.

Typical FSA Installation Foundation to Joist Typical FSA



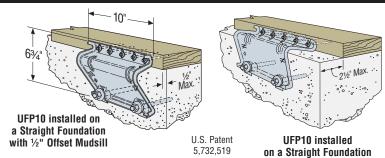
UFP Universal Foundation Plate

The UFP10 Retrofit Foundation Plate cuts installation time in half. Designed to connect when the mudsill is offset from the foundation up to $2\frac{1}{2}$ " or extended beyond the foundation up to $\frac{1}{2}$ ". MATERIAL: 14 gauge

FINISH: Galvanized. May be ordered HDG, contact Simpson Strong-Tie. See Corrosion Information, pages 14-17.

INSTALLATION: • Use all specified fasteners; see General Notes.

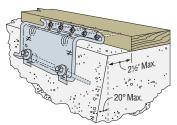
- Capacities are based on Simpson Strong-Tie® 1/4"x3" Strong-Drive® SDS Heavy-Duty Connector screw's factored lateral resistance, which are supplied with the UFP10.
- · Alternate lag screws will not achieve published values.



These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

	Fasteners			Factored Resistance Parallel to Plate (K _D = 1.15)			
Model	Anchor Bolt		D.Fir-L S-P-F		S-P-F		
No.	04	Dia.	Plate	lbs	lbs		
	Qty.	(in)		kN	kN		
UFP10-SDS3	2	1/2	5-1/4"x3" SDS	2110	1525		
UFF 10-3D33		72	3-74 X3 3D3	9.39	6.78		

1. Factored resistances have been increased 15% for earthquake or wind loading, with no further incre allowed. Reduce where other



UFP10 installed on _a frapezoid Foundation The SB%x24 anchor bolt offers a load-tested anchorage solution that exceeds the capacity of all of our holdowns that call for a %" dia. anchor. Similarly, the SB1x30 covers holdowns utilizing a 1" diameter anchor that exceed the capacity of our SSTB bolts. The SB%x24 is designed to maximize performance with minimum embedment for holdowns utilizing a %" dia. anchor.

Special Features:

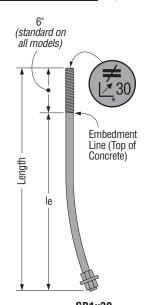
- Indentification on the bolt head showing embedment angle and model
- · Sweep geometry to optimize position in form
- Rolled thread for higher tensile capacity
- · Hex nuts and plate washer fixed in position
- Available in HDG for additional corrosion resistance

MATERIAL: ASTM F1554 Grade 36

FINISH: None. May be ordered HDG. Contact Simpson Strong-Tie.

INSTALLATION:

- SB is only for concrete applications poured monolithically, except where noted.
- Top nuts and washers for holdown attachment are not supplied with the SB; install standard nuts, couplers and/or washers as required.
- On HDG SB anchors, chase the threads to use standard nuts or couplers or use overtapped products in accordance with ASTM A563, for example Simpson Strong-Tie® NUT5/8-OST, NUT7/8-OST and NUT1-OST.
- Install SB before the concrete pour using AnchorMates®.
 Install the SB per the plan view detail.
- . Minimum concrete compressive strength is 20 MPa.
- · When rebar is required it does not need to be tied to the SB.

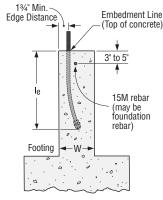


SB1x30 (Other models similar)

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

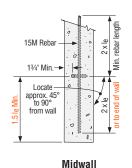
SB Bolts at Stemwall

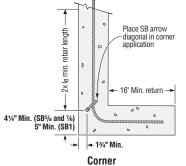
		Dimer	nsions			Fa	ctored Tens	ile Resistar	le Resistance			
		(i	n)		Wind/Seismic $I_EF_aS_a(0.2) < 0.35$			Seismic $I_EF_aS_a(0.2) \ge 0.35$				
Model No.				Min.	Midwall	Corner	End Wall	Midwall	Corner	End Wall		
	Stemwall Width	Dia.	Length	Embed.	lbs	lbs	lbs	lbs	lbs	End Wall Ibs kN 7600 33.81 7600 33.81 8045		
				(I _e)	kN	kN	kN	kN	kN	kN		
SB%x24	6	5/8	24	18	8915	8915	8915	8915	7600	7600		
3078824	U	78	24	10	39.66	39.66	39.66	39.66	33.81	33.81		
SB7/8x24	8	7/8	24	10	15560	13895	10135	11670	10420	7600		
3D78X24	0	78	24	18	69.22	61.81	45.08	51.91	46.35	33.81		
CD1v20	Q	1	30	24	20285	13895	10730	15215	10420	8045		
SB1x30	8	1	30		90.24	61.81	47.73	67.68	46.35	35.79		

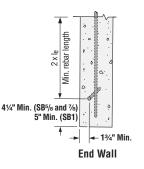


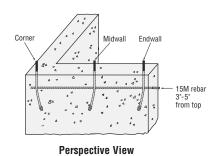
Typical SB Installation

1. See page 39 for notes to the Designer.

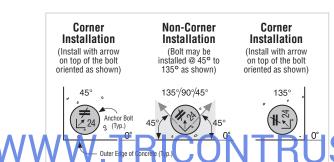








STEMWALL PLAN VIEWS



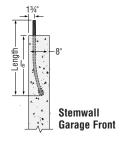
Plan View of SB Placement in Concrete

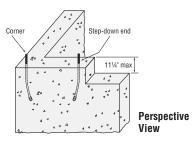
SB Anchor Bolt

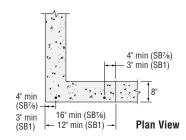


SB Bolts at Stemwall: Garage Front

		Dimer	nsions			Factored Tens	ile Resistance		
Model		(i	n)		Wind/Seismic I _E	$F_aS_a(0.2) < 0.35$	Seismic I _E F _a S	$S_a(0.2) \ge 0.35$	
Model No.	Ctomusell			Min.	Step-Down End	Corner	Step-Down End	Corner	
wo. Stemwar	Stemwall	Dia.	Length	Embed	lbs	lbs	lbs	lbs kN	
	wiutii			(l _e)	kN	kN	kN	kN	
SB%x24	0	7/8	24	18 -	10735	11385	8050	8540	
SB78X24	0	78	24		47.75	50.65	35.81	37.99	
SB1x30	0	4	30		16790	14550	12595	10910	
301830	0		30		74.69	64.72	56.03	48.53	

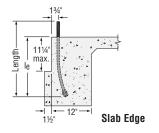


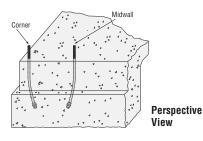


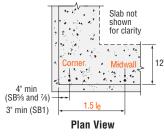


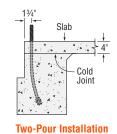
SB Bolts at Slab on Grade: Edge

		Dimer	nsions		bed. lbs lbs lbs kN kN kN kN 8915 8915 8915 7600 39.66 39.66 39.66 33.81 18220 18025 16345 13520 81.05 80.18 72.71 60.14 23900 23150 23580 17360			
Model		(i	n)		Wind/Seismic I _E	$F_aS_a(0.2) < 0.35$	Seismic I _E FaS	$S_a(0.2) \ge 0.35$
No.	Footing			Min.	Midwall	Corner	Midwall	Corner
NU.	Footing Width	Dia.	Length	Embed.	lbs	lbs	lbs	lbs
	wiutii	714111		(l _e)	kN	kN	kN	kN
SB%x24	12	5/8	24	10	8915	8915	8915	7600
3D78X24	12	78	24	10	39.66	39.66	39.66	33.81
SB7/sx24	12	7/8	24	10	18220	18025	16345	13520
3D78X24	12	78	24	18	81.05	80.18	72.71	60.14
SB1x30	12	1	30	24	23900	23150	23580	17360
SDIXSU	12	ļ	30		106.32	102.98	104.89	77.22



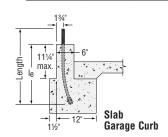


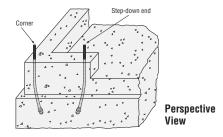


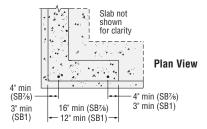


SB Bolts at Slab on Grade: Garage Curb

		Dimer	sions			Factored Tens	ile Resistance			
Model		(iı	n)		Wind/Seismic I _E	$F_aS_a(0.2) < 0.35$	Seismic I _E F _a S	$S_a(0.2) \ge 0.35$		
No.	Curb 5.			Min.	Step-Down End	Corner	Step-Down End	Corner		
Widt		Dia.	Length	Embed.	lbs	lbs	lbs	lbs		
	Wiath			(le)	kN	kN	kN	kN		
SB%x24	6	7/8	24	10	13630	16685	10225	12515		
3D78X24	O	78	24	18	60.63	74.22	45.48	55.67		
SB1x30	6	1	20	24	23150	23150	17360	17360		
301330	U	1	30 24	30	30 24	24	102.98	102.98	77.22	77.22







Notes to the Designer:

- 1. Rebar is required at top of stemwall foundations but is not required for Slab-on-Grade Edge and Garage Curb, or Stemwall Garage Front installations.

 2. Minimum end distances for SB bolts are as shown in graphics.

 3. Factored resistances have been developed based on testing per ICC AC399 in uncracked concrete using the corresponding adjustment factors from CSA A23.3-14 Annex D.
- 5. Midwall loads apply when ancho

Special Features:

- Identification on the bolt head showing embedment angle and model
- · Offset angle reduces side bursting, and provides more concrete cover
- · Rolled thread for higher tensile capacity
- · Stamped embedment line aids installation
- · Available in HDG for additional corrosion resistance

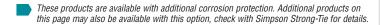
MATERIAL: ASTM F1554 Grade 36

FINISH: None. May be ordered HDG; contact Simpson Strong-Tie. INSTALLATION:

- SSTB is suitable for monolithic and two-pour concrete applications.
- Nuts and washers for holdown attachment are not supplied with the SSTB; install standard nuts, couplers and/or washers as required.
- On HDG SSTB anchors, chase the threads to use standard nuts or couplers or use overtapped products in accordance with ASTM A563, for example Simpson Strong-Tie® NUT%-OST or NUT%-OST.
- Install SSTB before the concrete pour using AnchorMates®.
 Install the SSTB per the plan view detail.
- . Minimum concrete compressive strength is 20 MPa.
- . When rebar is required it does not need to be tied to the SSTB.
- Order SSTBL Models *(example: SSTB16L)* for longer thread length (16L= $5\frac{1}{2}$ ", 20L = $6\frac{1}{2}$ ", 24L = 6", 28L = $6\frac{1}{2}$ "). SSTB and SSTBL tensile capacities are the same. SSTB34 and SSTB36 feature $4\frac{1}{2}$ " and $6\frac{1}{2}$ " of thread respectively and are not available in "L" versions.

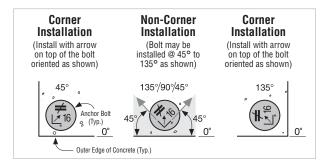
REINFORCED CONCRETE BLOCK

- Before concrete pour, install diagonally at approximately 45° in the cell.
- Grout all cells with coarse grout per CSA A179. Vibrate the grout per Code.
- See typical SSTB Installation in Grouted Concrete Block detail on page 42.



51/2 Identification 31/2" on the bolt head showing 11/2' 11/2 embedment angle and model. Embedment Line (Top of Embedment Line (Top of Length Length le le SSTB16L SSTB16 (other models similar) (other models similar)

See pages 41-42 for additional installation details.

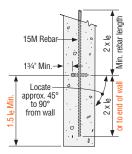


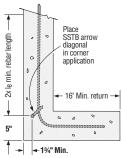
Plan View of SSTB Placement in Concrete

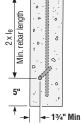
SSTB Bolts at Stemwall

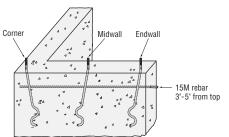
			Dimensions				Factored Tens	ile Resistance		
Madal			(in)		Wind/Seismic $I_EF_aS_a(0.2) < 0.35$			Seismic $I_EF_aS_a(0.2) \ge 0.35$		
Model No.	Ctomusul			Min.	Midwall	Corner	End Wall	Midwall	Corner	End Wall
110.	Stemwall Width	Dia.	Length	Embed.	lbs	lbs	lbs	lbs	lbs	lbs
	witti			(le)	kN	kN	kN	kN	kN	kN
SSTB16	6	5/8	17 % (16L = 19%)	12%	5365	5365	5365	3380	3380	3380
331010	0	78	17 78 (IOL = 1978)	1278	23.87	23.87	23.87	15.04	15.04	15.04
SSTB20	6	5/8	21 % (20L = 24%)	16%	6415	6005	6005	4170	3895	3895
331020	0	78	2198 (20L = 2498)	10 78	28.54	26.71	26.71	18.55	17.33	17.33
SSTB24	6	5/8	25% (24L = 28½)	20%	7470	6645	6645	4960	4410	4410
331B24	0	78	2378 (24L = 2078)	2078	33.23	29.56	29.56	22.06	19.62	19.62
SSTB28	8	7/8	29% (28L = 32%)	24 %	14710	12940	11315	11035	9705	8485
331020	0	78	2978 (20L = 3278)	24 78	65.44	57.56	50.33	49.09	43.17	37.74
SSTB34	8	7/8	34%	207/	14710	12940	11315	11035	9705	8485
001004	U	78	J478	28%	65.44	57.56	50.33	49.09	43.17	37.74
SSTB36	8	7/8	36%	28%	14710	12940	11315	11035	9705	8485
331030	O	78	3078	2078	65.44	57.56	50.33	49.09	43.17	37.74

1. See page 42 for notes to the Designer.









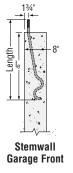
Midwall

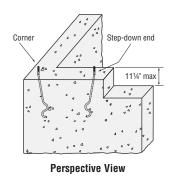


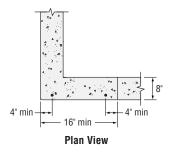
Concrete Connectors & Anchors

SSTB Bolts at Stemwall: Garage Front

		Dimer	nsions			Factored Tens	ile Resistance	
		(i	n)		Wind/Seismic I _E	$F_aS_a(0.2) < 0.35$	Seismic I _E F _a S	$S_a(0.2) \ge 0.35$
Model No.				Min.	Step-Down End	Corner	Step-Down End	Corner
	Stemwall Width	Dia.	. Length	Embed. (l _e)	lbs	lbs	lbs	lbs
					kN	kN	kN	kN
SSTB28	0	8 %	29%	24 1/8	10425	10470	7820	7850
331020	8		2978		46.37	46.57	34.79	34.92

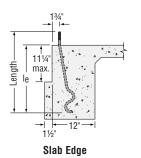


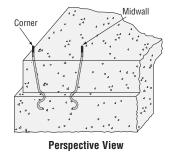




SSTB Bolts at Slab on Grade: Edge

		Dime	nensions		Factored Tensile Resistance				
		(i	n)		Wind/Seismic I _E	Wind/Seismic $I_EF_aS_a(0.2) < 0.35$ Seismic $I_EF_aS_a(0.2) \ge 0.35$			
Model No.				Min.	Midwall	Corner	Midwall	Corner	
	Footing Width	Dia.	Length	Embed.	lbs	lbs	lbs	lbs	
				(I _e)	kN	kN	kN	kN	
SSTB16	12	5/8	17%	105/	7955	7955	5015	5015	
551010	12	78	17 78	12%	35.39	35.39	22.31	22.31	
SSTB20	12	5/8	21%	16%	8915	8915	6345	6345	
331020	12	78	2178		39.66	39.66	28.23	28.23	
SSTB24	12	5/8	25%	20%	8915	8915	7680	7680	
331024	12	78	2378	20%	39.66	39.66	34.16	34.16	
SSTB28	12	7/8	29%	24%	18220	18220	14670	15400	
551020	12	78	2978	24 78	81.05	81.05	65.26	68.51	
CCTD24	12	7/8	34%	007/	18220	18220	14670	15400	
SSTB34	12	78	34 78	28 1/8	81.05	81.05	65.26	68.51	
CCTDOC	12 % 36% 28	007/	18220	18220	14670	15400			
SSTB36	12	78	30 78	28%	81.05	81.05	65.26	68.51	





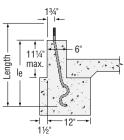


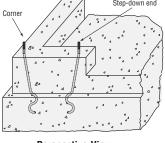
Plan View

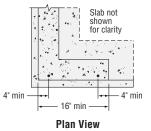
Strong-Tie

SSTB Bolts at Slab on Grade: Garage Curb

		Dimei	nsions			Factored Tens	ile Resistance	
		(in)			$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$			
Model No.				Min.	Step-Down End	Corner	Step-Down End	Corner
	Curb Width	Dia.	Length	Embed. (I _e)	lbs	lbs	lbs	lbs
					kN	kN	kN	kN
SSTB28	6	6 %	207/	29% 24%	15255	18220	11440	13785
331020			29 1/8		67.86	81.05	50.89	61.32







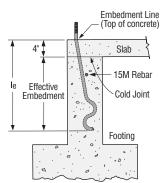
Slab Garage Curb

Perspective View

SSTB Bolts in 8" CMU Wall

		Dimensions (in)		Factored Tensile Resistance			
Model No.			Min.	Midwall	End Wall		
NU.	Dia.	Length	Embed.	lbs	lbs		
			(I _e)	kN	kN		
SSTB16	5/8	17% (16L = 19%)	12%	5715	2340		
331010	78	1778 (IOL = 1978)	1278	25.42	10.41		
CCTDOO	5/	21% (20L = 24%)	16%	5715	2340		
331020	SSTB20 % 21% (20	2178 (20L = 2478)	10 78	25.42	10.41		
SSTB24	5/8	25% (24L = 28%)	20%	5715	2340		
331024	78	2378 (24L = 2078)	20%	25.42	10.41		
SSTB28	7/8	29% (28L = 32%)	247/8	8030	5760		
331020	78	2978 (20L = 3278)	2478	35.72	25.62		
SSTB34	7/8	347/8	287/8	8030	5760		
001004	/8	J478	2078	35.72	25.62		
SSTB36	7/8	267/	287/8	8030	5760		
	-/8	36%	20 78	35.72	25.62		

- 1. Factored resistances shown are based on testing per CSA A370-14.
- 2. Reinforced concrete masonry units shall have a minimum specified compressive strength of 15 MPa per CSA S304.1-14 using Type N mortar and filled solid using coarse grout per CSA A179-14.
- 3. Minimum end distance required to achieve Midwall resistance is 1.5 le.
- 4. Minimum end distance required to achieve End Wall resistance is 41/4".
- 5. See installation detail for minimum reinforcing requirements.



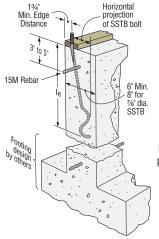
For two-pour (4" slab) installation loads:

- When using the SSTB20, use the equivalent loads of the SSTB16.
- · When using the SSTB24, use the equivalent loads of the SSTB20.
- · When using the SSTB34 or 36, use the equivalent loads

f the SSTB28.

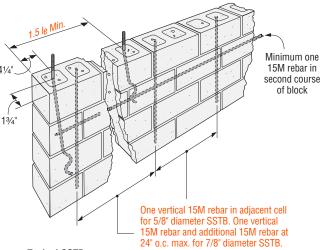
Notes to the Designer:

- 1. Rebar is required at top of stemwall foundations but is not required for Slab-on-Grade Edge and Garage Curb, or Stemwall Garage Front installations.
- 2. Minimum end distances for SSTB bolts are as shown in graphics.
- 3. Factored resistances have been developed based on testing per ICC AC399 in uncracked concrete using the corresponding adjustment factors from CSA A23.3-14 Annex D.
- 4. Factored resistances for seismic $I_EF_aS_a(0.2) \ge$ 0.35 applications assumes ductile yielding in the attachment. See D.4.3.5.3 CSA A23.3-14 for more information.
- 5. See ESR-2611 for additional information.
- 6. Midwall capacities apply when anchor is 1.5 $\ensuremath{I_{\mbox{\scriptsize e}}}$ or greater from the end. For bolts acting in tension simultaneously, the minimum bolt centre-to-centre spacing is 3 le.



Typical SSTB Installation in Concrete Foundation

Maintain minimum rebar cover, per CSA A23.1-14 requirements



Typical SSTB Installation in Grouted Concrete Block

(2 total vertical rebars for endwall, 3 total vertical rebars for midwall)

Length

'High Strength"

standard-steel models)

designation

(blank on

Concrete Connectors & Anchors

PAB Pre-Assembled Anchor Bolt

The PAB anchor bolt is a versatile new cast-in-place anchor bolt ideal for high-tension-load applications. It features a plate washer at the embedded end sandwiched between two fixed hex nuts and a head stamp for easy identification after the pour.

- Available in diameters from ½" to 1¼" in lengths from 12" to 36" (in 6" increments)
- · Available in standard and high-strength steel
- Head stamp contains the No Equal sign, diameter designation and an "HS" on high-strength rods

MATERIAL: Standard Steel – ASTM F1554 Grade 36, A36 or A307 – Fu = 58 ksi High-Strength Steel (up to 1" dia.) – ASTM A449 – Fu = 120 ksi

High-Strength Steel $(1\frac{1}{8}$ " and $1\frac{1}{4}$ " dia.) – ASTM A193 B7 or F1554 Grade 105 – F_u = 125 ksi

FINISH: None



The Simpson Strong-Tie® Anchor Designer Software™ analyzes and suggests anchor solutions using the CSA A23.3 Annex D Limit States Design methodology. It provides cracked and uncracked-concrete anchorage solutions for numerous Simpson Strong-Tie Anchor Systems® mechanical and adhesive anchors as well as the PAB anchor. With its easy-to-use graphical user interface, the software makes it easy for the Designer to identify anchorage solutions without having to perform time-consuming calculations by hand.

PAB Anchor Bolt - Standard Steel

Diameter (in)	Plate Washer Size (in)	l ₁ (in)	Root Model No.	Lengths (in)
1/2	1/4 x 11/4 x 11/4	1	PAB4-XX	
5/8	% x 1½ x 1½	11/4	PAB5-XX	
3/4	3% x 2 x 2	1%	PAB6-XX	12" to 36"
7/8	3/8 x 21/4 x 21/4	1½	PAB7-XX	(in 6"
1	% x 2½ x 2½	1%	PAB8-XX	increments)
11/8	3/8 x 23/4 x 23/4	13/4	PAB9-XX	
11/4	½ x 3 x3	21/2	PAB10-XX	

PAB Anchor Bolt - High-Strength Steel

Diameter (in)	Plate Washer Size (in)	l ₁ (in)	Root Model No.	Lengths (in)
1/2	1/4 x 11/4 x 11/4	1	PAB4H-XX	
5/8	3% x 1½ x 1½	11/4	PAB5H-XX	
3/4	3% x 2 x 2	1%	PAB6H-XX	12" to 36"
7/8	3/8 x 21/4 x 21/4	1½	PAB7H-XX	(in 6"
1	% x 2½ x 2½	1%	PAB8H-XX	increments)
11/8	3/8 x 23/4 x 23/4	13/4	PAB9H-XX	
11/4	½ x 3 x 3	21/2	PAB10H-XX	

^{1.} Plate washers are designed to develop the capacity of the bolt.

How to specify and order:

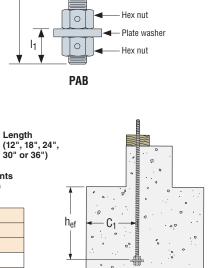
When calling out PAB anchor bolts, substitute the desired length for the "XX" in the Root Model Number.

For a %"x18" anchor bolt, the model number would be PAR5-18 (or PAB5H-18 for high strength).

Naming Scheme:

PAB

Anchor



The diameter

code on the

head is the

same as that

used for rebar:

%" Diameter anchor rod

 $4 = \frac{1}{2}$ ", $5 = \frac{5}{8}$ "

 $6 = \frac{3}{4}$ ", etc.

and Grade * Units in 1/8" Increments (Ex: 9 = %" or 11/8")

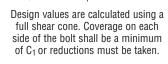
Diameter'

Length

PAB5H-12

PAB Anchor Bolt – Anchorage Solutions

			I	Factored Tensil	e Resistance N	r	
		Wind/Se	ismic I _E FaSa(0.	2) < 0.35	Seisn	nic I _E FaSa(0.2)	≥ 0.35
Model No.	Diameter (in)	h _{ef}	C ₁	Nr	h _{ef}	C ₁	Nr
140.	(,	in	in	lbs	in	in	lbs
		mm	mm	kN	mm	mm	kN
PAB4	1/2	4	7	5600	4	7	5600
FAD4	/2	102	178	24.91	102	178	24.91
PAB5	5/8	5	8.5	8915	6	10	8915
LADO	78	127	216	39.66	152	254	39.66
PAB6	3/4	6	10	13175	7	11.5	13175
PADO	74	152	254	58.61	178	292	58.61
PAB7	7/8	8	13.5	18225	9	15	18225
PAD/	78	203	343	81.07	229	381	81.07
PAB7H	7/8	12	19	37725	15	24	37725
PAD/II	78	305	483	167.82	381	610	167.82
PAB8	1	9	15	23905	11	18	23905
PADO	'	229	381	106.34	279	457	106.34
PAB8H	1	15	24	49485	18	28.5	49485
ГАДОП	'	381	610	220.13	457	724	220.13
PAB9	11/	11	18	30100	13	21	30100
FADS	11/8	279	457	133.90	330	533	133.90
PAB10	41/	12	19.5	38225	15	24	38225
PABIU	11/4	305	495	170.04	381	610	170.04



2C₁ Min.

- 1. Factored resistances shown are in accordance with CSA A23.3-14 Annex D using 20 MPa concrete assuming cracked concrete and no supplementary reinforcement (Category B).
- 2. PAB8H values shown in italics for seismic applications require minimum 25 MPa concrete.
- 3. Foundation dimensions are for anchorage only. Foundation design (size and reinforcement) is the responsibility of the design professional.
- Factored resistances for seismic $I_EF_aS_a(0.2) \ge 0.35$ applications assumes ductile yielding in the D.4.3.5.3 CSA A23.3-14 for



MASA/MASAP Mudsill Anchors

This product is preferable to a) easier installation, b) high or a combination of these for

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Mudsill anchors have always been a time-saving alternative to anchor bolts, and the new MASA anchors provide even greater load-carrying capacity than our original MAS. As a result, the MASA provides an alternative for 5%" and ½" mudsill anchor bolts on 2x, double-2x and 3x mudsills. Two versions of the MASA are available – the standard MASA for installation on standard forms and the MASAP for panelized forms.

The MASA and MASAP have been tested to meet the requirements of ICC-ES acceptance criteria AC-398 for cracked and uncracked concrete. New test data is reflected in the table below.

MATERIAL: 16 gauge

FINISH: Galvanized, all available in ZMAX® coating. See Corrosion Information, pages 14-17.

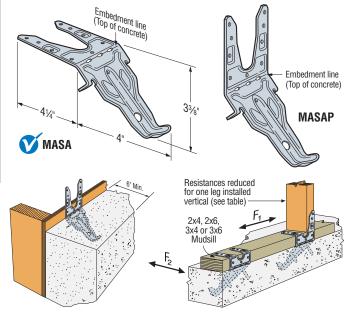
INSTALLATION: • Use all specified fasteners. See General Notes.

- Concrete shall have a minimum $f'_C = 2500 \text{ psi } (17.25 \text{ MPa}).$
- Spalling—Full resistances apply for spalls up to a maximum height of 11/4" and a maximum depth of 1/6". Any exposed portion of the mudsill anchor must be protected against possible corrosion.





Typical MASA Installation in Concrete



Typical MASAP Installation in Concrete

Typical MASA/MASAP Installation on Sill Plate

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

Factored Resistance for Non-Cracked Concrete

		Facto	eners				Factored R	lesistance l	lon-Cracke	d Concrete			
	Sill Plate Size	1 4310	511613			mic l _e FaSa(0.2) < 0.35			Seismic	leFaSa(0.2) ≥ 0.35	
Model				Uplift	F		F	_	Uplift	F		F	_
No.		Sides	Тор	(K _D =1.15)	$(K_D=1.15)$	$(K_D=0.65)$	(K _D =1.15)	$(K_D=0.65)$	$(K_D=1.15)$	$(K_D=1.15)$	$(K_D=0.65)$	$(K_D=1.15)$	$(K_D=0.65)$
		Oluca	тор	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs
				kN	kN	kN	kN	kN	kN	kN	kN	kN	kN
				STANDA			ttached to	D.Fir-L Sill	Plate				
	2x4, 2x6	3-10dx1½	6-10dx1½	1235	2000	1130	1800	1035	1155	1810	1130	1490	1035
MASA or	LX I, LXO	0 100/172	0 100X172	5.49	8.90	5.03	8.01	4.60	5.14	8.05	5.03	6.63	4.60
MASAP	3x4, 3x6	5-10dx1½	4-10dx1½	935	1910	1130	1260	710	730	1430	1130	1245	710
	0,4,0,0	0 100X172	4 100X172	4.16	8.50	5.03	5.60	3.16	3.25	6.36	5.03	5.54	3.16
							Attached to	D.Fir-L Sill					
MASA or	2x4, 2x6	6-10dx1½	3-10dx1½	1115	1330	755	_	_	875	1025	755	_	_
MASAP	ZAH, ZAO			4.96	5.92	3.36	_	_	3.89	4.56	3.36	_	_
		BOTH LE	GS OVER MA				LATION – At	tached to D					
MASA or		9-10dx1½	_	1310	1560	1130	_		980	1170	1130		
MASAP 2X4, 2X6	0 100/172		5.83	6.94	5.03	_	_	4.36	5.20	5.03	_	—	
			DC				ON – Attach						
MASA or	2x4, 2x6	5-10dx1½	2-10dx1½	1300	1555	880	1315	745	975	1290	880	1315	745
MASAP	LX I, LXO	0 100/172	L 100X172	5.78	6.92	3.91	5.85	3.31	4.34	5.74	3.91	5.85	3.31
						-	Attached to						
	2x4. 2x6	3-10dx1½	6-10dx1½	875	1505	1040	1275	735	875	1505	1040	1275	735
MASA or	27.1, 27.0		0 100/172	3.89	6.69	4.63	5.67	3.27	3.89	6.69	4.63	5.67	3.27
MASAP	3x4, 3x6	5-10dx1½	4-10dx1½	665	1615	1040	895	505	665	1430	1040	895	505
	0X1, 0X0	0 100/172	1 100/172	2.96	7.18	4.63	3.98	2.25	2.96	6.36	4.63	3.98	2.25
							Attached to	S-P-F Sill I					
MASA or	2x4, 2x6	6-10dx1½	3-10dx1½	795	950	650	_		795	950	650		_
MASAP	27.1, 27.0			3.54	4.23	2.89	_	_	3.54	4.23	2.89	_	_
		BOTH L	EGS OVER M				LATION – A	ttached to S					
MASA or	2x4, 2x6 9-10dx1½	_	960	1290	840	_	_	960	1170	840	_	_	
MASAP	27.1, 27.0	0 100/172		4.27	5.74	3.74	_	_	4.27	5.20	3.74	_	_
			D				ION – Attacl						
MASA or	2x4, 2x6	5-10dx1½	2-10dx1½	1000	1170	760	935	525	975	1170	760	935	525
MASAP	,		_ 100/11/2	4.45	5.20	3.38	4.16	2.34	4.34	5.20	3.38	4.16	2.34

Factored resistances shown are based on testing per ICC AC398 using the corresponding adjustment factors from CSA A23.3-14 Annex D
 The minimum 28-day concrete compressive streng h (f'c) shall be

2500 psi (17.25 MPa).

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^{3.} Factored resistances are base on a minimum wall width of 6".

^{4.} For simultaneous loads in more than one direction, the connector must be evaluated using the unity equation (see Instructions for the Designer Insteed note b. on page 20).

5. IN ALL 3: 00 11/2" = 0.148" dia x 1/2" long. See pages 22-23 for other nail sizes and information.

MASA/MASAP Mudsill Anchors



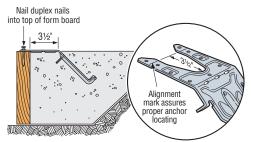
Factored Resistance for Cracked Concrete

	Enate	noro				Factore	d Resistanc	e Cracked C	Concrete			
	гази	:11612		Wind/Seis	mic l _e F _a S _a (0.2) < 0.35			Seismic	l _e F _a S _a (0.2) ≥ 0.35	
A or 2x4, 2x6 A or AP 2x4, 2x6			Uplift	F	1	F	2	Uplift	F	1	F	2
	Cidoo	Ton	$(K_D=1.15)$	$(K_D=1.15)$	$(K_D=0.65)$	$(K_D=1.15)$	$(K_D=0.65)$	$(K_D=1.15)$	$(K_D=1.15)$	$(K_D=0.65)$	(K _D =1.15)	$(K_D=0.65)$
0.20	Sines	ioh	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs
			kN	kN	kN	kN	kN	kN	kN	kN	kN	kN
			STANDA	ARD INSTAL	_LATION — A	ttached to	D.Fir-L Sill	Plate				
244 246	2 10dv11/	6 10dv11/	1165	2000	1130	1455	1035	875	1810	1130	1090	1035
284, 280	3-10ux 1 /2	0-100X172	5.18	8.90	5.03	6.47	4.60	3.89	8.05	5.03	4.85	4.60
244 246	E 104v11/	4.40dv41/	735	1910	1130	1215	710	550	1430	1130	910	710
3X4, 3X0	J-100X172	4-100X172	3.27	8.50	5.03	5.40	3.16	2.45	6.36	5.03	4.05	3.16
			ONE LEG	UP INSTA	LLATION -	Attached to	D.Fir-L Sill	Plate				
0.4 0.6	C 104v11/	0.40dv41/	880	1330	755	_	_	660	1025	755	_	_
ZX4, ZX0	0-1UUX 1 ½	3-100X1½	3.91	5.92	3.36	_	_	2.94	4.56	3.36	_	_
	BOTH LE	GS OVER MA	X. ½" PLYV	VOOD OR O	SB INSTALI	ATION – A	tached to D	.Fir-L Sill P	late and Ri	mboard		
0.4.0.0	0.40.1417		1125	1560	1130	_	_	840	1170	1130	_	_
2X4, 2X6	9-100X1½	_	5.00	6.94	5.03	_	_	3.74	5.20	5.03	_	_
			UBLE 2x SI	LL PLATE I	NSTALLATI	ON – Attach	ed to D.Fir-	L Sill Plate				
0.4.0.0	E 40 L 41/	0.401.41/	985	1555	880	1315	745	735	1290	880	1150	745
2x4, 2x6	5-10dx1½	2-100x1½	4.38	6.92	3.91	5.85	3.31	3.27	5.74	3.91	5.12	3.31
			STAND	ARD INSTA	LLATION -	Attached to	S-P-F Sill F	Plate				
0.4.0.0	0.40.141/	0.40-1-41/	875	1505	1040	1275	735	875	1505	1040	1090	735
2X4, 2X6	3-100X1½	6-100X1½	3.89	6.69	4.63	5.67	3.27	3.89	6.69	4.63	4.85	3.27
0.4.0.0	E 40 L 44/	4.40.1.447	665	1615	1040	895	505	550	1430	1040	895	505
3x4, 3x6	5-10dx1½	4-100x1½	2.96	7.18	4.63	3.98	2.25	2.45	6.36	4.63	3.98	2.25
			ONE LE	G UP INSTA	LLATION -	Attached to	S-P-F Sill	Plate				
0.400	0.401.444	0.401.411	795	950	650	_		660	950	650	_	_
2x4, 2x6	6-10dx1½	3-10dx1½	3.54	4.23	2.89	_	_	2.94	4.23	2.89	_	_
	BOTH L	EGS OVER M	AX. ½" PLY	WOOD OR C	OSB INSTAL	LATION – A	ttached to S	S-P-F Sill Pl	ate and Rir	nboard		
0.400			960	1290	840	_	_	840	1170	840	_	_
2x4, 2x6	9-10dx1½	_	4.27	5.74	3.74	_	_	3.74	5.20	3.74	_	_
		D		-		ON – Attac	hed to S-P-	-				
			<u> </u>						1170	760	935	525
2x4, 2x6	5-10dx1½	2-10dx1½	4.38	5.20	3.38	4.16	2.34	3.27	5.20	3.38	4.16	2.34
	2x4, 2x6 2x4, 2x6	Sill Plate Size Sides 2x4, 2x6 3-10dx1½ 3x4, 3x6 5-10dx1½ 2x4, 2x6 6-10dx1½ 2x4, 2x6 9-10dx1½ 2x4, 2x6 3-10dx1½ 2x4, 2x6 3-10dx1½ 2x4, 2x6 6-10dx1½ 2x4, 2x6 6-10dx1½ 2x4, 2x6 9-10dx1½	Plate Size Sides Top 2x4, 2x6 3-10dx1½ 6-10dx1½ 3x4, 3x6 5-10dx1½ 4-10dx1½ 2x4, 2x6 6-10dx1½ 3-10dx1½ BOTH LEGS OVER MA 2x4, 2x6 9-10dx1½ 2-10dx1½ 2x4, 2x6 3-10dx1½ 6-10dx1½ 3x4, 3x6 5-10dx1½ 4-10dx1½ 2x4, 2x6 6-10dx1½ 3-10dx1½ BOTH LEGS OVER MA 2x4, 2x6 9-10dx1½ 3-10dx1½ BOTH LEGS OVER MA 2x4, 2x6 9-10dx1½ D	Sill Plate Size Sides Top Uplift (K _D =1.15) lbs kN STANDA 1165 5.18 3.27 ONE LEG	Sill Plate Size Sides Top Uplift F (Kp=1.15) (Kp=1.15) Ibs Ibs	Sill Plate Sides Top Uplift F1 (KD=1.15) (KD=0.65) Ibs Ibs	Sill Plate Size Sides Top	Sill Plate Size Sides Top Uplift F ₁ F ₂ (K _D =1.15) (K _D =0.65) (K _D =0.15) (K _D =0.65) (K _D =0.15) (Sille	Note		Pasteners

See foot notes on page 44.

ALTERNATIVE MUDSILL ANCHOR INSTALLATIONS

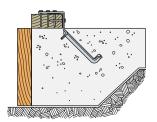
ALTERNATE INSTALLATION FOR INSIDE OF WALL CONTINUITY



1

STEP 1:

Attach MASA 3½" from inside of form. After concrete cures, remove nails and bend straps up 90°

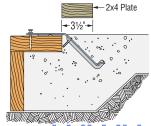


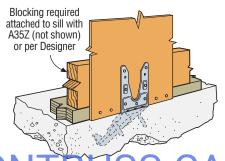


STEP 2:

Place mudsill on concrete and nail MASA over mudsill

ALTERNATE INSTALLATION FOR BRICK LEDGES





MASA/MASAP Rim Joist or Blocking Installation in Concrete Over Max. ½" Sheathing

Alternate MASA Installation to Blick Ledges RICONTRUSS CA

LMAZ/MAB/MASB Mudsill Anchors

The LMAZ offers a higher lateral load capacity in a lighter gauge.

The MASB is designed for installation on concrete masonry units.

The MAB anchors the mudsill to concrete block, poured walls or slab foundation.

MATERIAL: MASB, —16 gauge; LMAZ, MAB—18 gauge FINISH: Galvanized. Some products available in ZMAX®; LMAZ—ZMAX only. See Corrosion Information, pages 14-17.

INSTALLATION: • Use all specified fasteners. See General Notes.

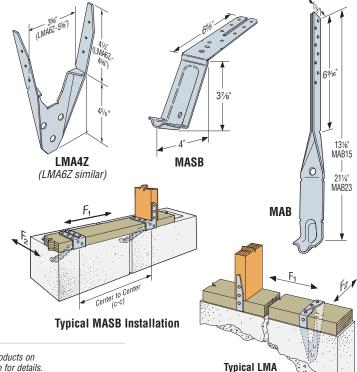
 Not for use where (a) a horizontal cold joint exists between the slab and foundation wall or footing beneath, unless provisions are made to transfer the load, or (b) anchors are installed in slabs poured over foundation walls formed of concrete block.

(13.8 MPa).

MASB—First fill CMU cell with concrete grout. Place MASB into the grouted cell, and adjust into position. Attach mudsill to anchor after the concrete cures.

All grout and concrete must have a minimum f'c of 2000 psi

• MAB—When used in monolithic slab or stemwall construction, prior to installation, spread the MAB legs to accommodate mudsill. Immediately after pouring and screeding, insert into the concrete or grout. Attach the mudsill to the anchor with 10dx1½" nails after the concrete cures. When installed in grouted concrete block or solid pour for a centre hole installation, drill a ¾" hole through the mudsill and install straps through the hole. Wrap MAB straps around the mudsill and install 10dx1½" nails.



These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

		Faste	nore		Factor	ed Resist	ance (K _l	o = 1.15)	
		rasic	511619		D.Fir-L			S-P-F	
Model No.	Sill Size	Sides	_	Uplift	Parallel to Plate F ₁	Perp. to Plate F ₂	Uplift	Parallel to Plate F ₁	Perp. to Plate F ₂
		(total)	Тор	lbs	lbs	lbs	lbs	lbs	lbs
				kN	kN	kN	kN	kN	kN
MASB	2x4, 2x6	2-10dx1½	6-10dx1½	200	1315	900	140	935	640
IVIASD	284, 280	Z-10UX 1 72	0-10ux 1 /2	0.89	5.85	4.00	0.62	4.16	2.85
MAB15	2x4, 2x6	2-10dx1½	4-10dx1½	800	725	705	570	515	500
CI DAIN	284, 280	Z-100X172		3.56	3.23	3.14	2.54	2.29	2.22
MAB23	2x4, 2x6	2-10dx1½		800	725	705	570	515	500
IVIADZS	284, 280	Z-100X172		3.56	3.23	3.14	2.54	2.29	2.22
	2x4	2-10dx1½	4-10dx1½	1410	955	930	1000	675	660
LMA4Z	2,44	Z-100X172	4-10ux 172	6.27	4.25	4.14	4.45	3.00	2.94
LIVIA42	3x4	4-10dx1½	2-10dx1½	1410	955	930	1000	675	660
	384	4-10ux 1 /2	2-10ux 1 72	6.27	4.25	4.14	4.45	3.00	2.94
	2x6	0.10dv11/	4-10dx1½	1410	1165	1125	1000	825	800
LMA6Z	2X0	2-10dx1½	4-10ux 1 /2	6.27	5.18	5.00	4.45	3.67	3.56
LIVIAUZ	3x6	1-10dv114	4-10dx1½	1570	1165	1125	1115	825	800
	3,0	4-10dx1½		6.98	5.18	5.00	4.96	3.67	3.56

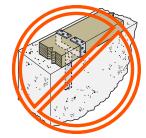
1. Factored resistances have been increased 15% for short-term load duration. Reduce where other durations govern.

For factored uplift resistances, provide attachment from the mudsill to the building structural components to prevent cross grain bending.

3. LMA attached to the studs has a factored uplift resistance of 1125 lbs (5.00 kN) for D.Fir-L and 800 lbs (3.55kN) for S-P-F; a factored F₁ resistance of 1025 lbs (4.56 kN) for D.Fir-L and 725 lbs (3.22 kN) for S-P-F; a factored F₂ resistance of 1075 lbs (4.78 kN) for D.Fir-L and 760 lbs (3.38 kN) for S-P-F.

4. MASB with one leg attached to the studs has a factored uplift resistance of 200 lbs (0.89 kN) for D.Fir-L and 140 lbs (0.62 kN) for S-P-F; a factored F₁ resistance of 1110 lbs (4.93 kN) for D.Fir-L and 1020 lbs (4.54 kN) for S-P-F; a factored F₂ resistance of 895 lbs (3.98 kN) for D.Fir-L and 635 lbs (2.82 kN) for S-P-F.

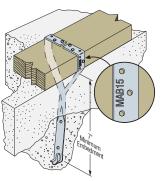
5. NAILS: $10dx1\frac{1}{2} = 0.148$ " dia. $x 1\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.

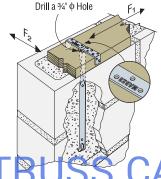


Installation (in concrete with framing)

MAB
Misinstallation
(MAB straps
must be
separated
before the
concrete is
poured)

Typical MAB15
Installation
in Concrete
(MAB23 similar,
with 15" minimum
embedment)
Not applicable
for concrete
block installation





Typical MAB23
Installation in
Concrete Block
(MAB15 similar)
MAB23 provides
a two block
embedment,
if required by
the local code
jurisdiction.
Concrete
installation is
Similar.

VWW.TRICONT

SIMPSON

GLB/HGLB/GLBT Beam Seats

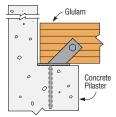
The GLB Series provides a connection between beam and concrete or CMU pilaster. $\label{eq:concrete} % \begin{center} \begin$

FINISH: Simpson Strong-Tie® gray paint. Hot-dip galvanized available.

INSTALLATION: • Use all specified fasteners. See General Notes.

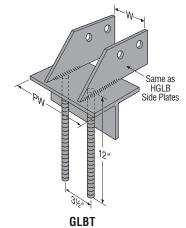
- Bolt holes in wood shall be a minimum of 1/2 to a maximum of 1/6 larger than the bolt diameter (per 12.4.1.2 CSA 086-14).
- Check the rebar spacing requirements on all installations.

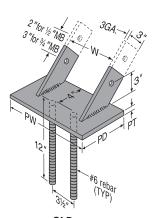
OPTIONS: • Beam seats for sawn timber and other sizes may be ordered by specifying special dimensions; use the letter designations shown on the illustrations.

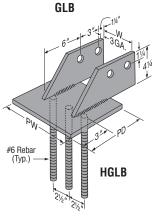


Typical GLB Installation

• Spe	cify if	two-k	oolt G	LB mo	odel is	desi	red; see illu	ıstration.		1)	pical GLB ins	statiativii
	Di	imens	ions (in)	Во	lts	Factored	l Bearing R	Resistance	(K _D = 1.00)	Factored Ho Resistance (
Model							Mas	te Block onry¹		ncrete ²	D.Fir-L	Spruce- Pine
No.	W ⁶	PD	PW	PT	Qty	Dia.	Type N Mortar	Type S Mortar	D.Fir-L Glulam	Spruce- Pine Glulam	Glulam	Glulam
							lbs	lbs	lbs	lbs	lbs	lbs
							kN 7765	kN 9700	kN 23930	kN 19805	kN	kN
GLB5A	51/4	5	7	3 ga	1	1/2"	34.54	43.15	106.45	88.10		_
GLB5B	51/4	6	7	3/8"	1	1/2"	9320	11640	28715	23765	_	_
GLDJD	J /4	0		78	<u>'</u>	/2	41.46	51.78	127.74	105.72	_	_
GLB5C	51/4	7	7	3/8"	1	1/2"	10870 48.35	13580 60.41	33500 149.02	27725 123.33		_
		-	_				12425	15520	38285	31685		
GLB5D	51/4	8	7	3/8"	1	1/2"	55.27	69.04	170.31	140.95	_	_
GLB7A	67/8	5	9	3 ga	1	3/4"	9985	12475	31040	26565		_
GLBITT	078	ļ .		o gu	<u>'</u>	/-	44.42	55.49	138.08	118.17		_
GLB7B	6%	6	9	3/8"	1	3/4"	11980 53.29	14970 66.59	38520 171.35	31880 141.81		_
01.070	07/			2711	_	2/11	13975	17465	44940	37190		_
GLB7C	6%	7	9	3/8"	1	3/4"	62.17	77.69	199.91	165.44	_	_
GLB7D	67/8	8	9	3/8"	1	3/4"	15975	19955	51360	42505		_
	***	_	-		·		71.06 7625	88.77 9530	228.47 14590	189.08 12075	4690	4620
	31/4	5	10	3/8"	2	3/4"	33.92	42.39	64.90	53.71	4680 20.82	20.55
	F1/	_	10	2/11		2/11	9845	12300	23930	19805	4680	4680
HGLBA	51/4	5	10	3/8"	2	3/4"	43.79	54.72	106.45	88.10	20.82	20.82
HULDA	7	5	10	3/8"	2	3/4"	11095	13860	32100	26565	4680	4680
	Ŀ.	-			_		49.35 11095	61.65 13860	142.79 39685	118.17 32845	20.82 4680	20.82 4680
	85/8	5	10	3/8"	2	3/4"	49.35	61.65	176.53	146.11	20.82	20.82
	01/	_	10	3/11	0	3/11	9150	11435	17510	14490	5280	4620
	31/4	6	10	3/8"	2	3/4"	40.70	50.87	77.89	64.46	23.49	20.55
	51/4	6	10	3/8"	2	3/4"	11815	14760	28715	23765	8580	7510
HGLBB							52.56 13310	65.66 16630	127.74 38520	105.72 31880	38.17 11555	33.41 10110
	7	6	10	3/8"	2	3/4"	59.21	73.98	171.35	141.81	51.40	44.97
	85/8	6	10	3/8"	2	3/4"	13310	16630	47625	39415	14060	12420
	078	U	10	/0		/4	59.21	73.98	211.85	175.33	62.54	55.25
	31/4	7	10	3/8"	2	3/4"	10675 47.49	13340 59.34	20425 90.86	16905 75.20	5280 23.49	4620 20.55
	F4.	_	40	0.411	_	0./11	13780	17220	33500	27725	8580	7510
HGLBC	51/4	7	10	3/8"	2	3/4"	61.30	76.60	149.02	123.33	38.17	33.41
HULDU	7	7	10	3/8"	2	3/4"	15530	19405	44940	37190	11555	10110
	<u> </u>						69.08 15530	86.32 19405	199.91 55560	165.44 45980	51.40 14060	44.97 12420
	8%	7	10	3/8"	2	3/4"	69.08	86.32	247.15	204.54	62.54	55.25
	31/4	8	10	3/8"	2	3/4"	12200	15245	23345	19320	5280	4620
	374	0	10	78		94	54.27	67.82	103.85	85.94	23.49	20.55
	51/4	8	10	3/8"	2	3/4"	15750	19680	38285	31685	8580	7510
HGLBD							70.06 17750	87.54 22175	170.31 51360	140.95 42505	38.17 11555	33.41 10110
	7	8	10	3/8"	2	3/4"	78.96	98.64	228.47	189.08	51.40	44.97
	85%	8	10	3/8"	2	3/4"	17750	22175	63500	52550	14060	12420
	078	0	10	/8		/4	78.96	98.64	282.47	233.76	62.54	55.25
GLBT5124	—	51/4	12	5/16"	2	3/4"	13975 62.17	17465 77.69			8580 ⁸ 38.17	7510 ⁸ 33.14
OI DTC:::		011	1.	2	_	2	17305	21620			8580 ⁸	7510 ⁸
GLBT612 ⁴		6½	12	3/8"	2	3/4"	76.98	96.17			38.17	33.14
GLBT516	_	51/4	16	5/16"	2	3/4"	18635	23285	See	See	8580 ⁸	7510 ⁸
3251010		074		710	_	/"	82.90	103.58	foonote	foonote	38.17	33.14
GLBT616	—	6½	16	3/8"	2	3/4"	23070	28825 128.23	3	3	8580 ⁸ 38.17	7510 ⁸ 33.14
OL DEFOC		F1/	00	E/ "	_	2/11	23295	29105			8580 ⁸	7510 ⁸
GLBT520	_	51/4	20	5/16"	2	3/4"	103.63	129.47		A 1-	38.17	33.14
GLBT620		6½	20	3/8"	2	3/4"	36035	28840			8580 ⁸	7510 ⁸
		1 - / -		VV	V	V ·	160.30	128.29			38.17	33.14







- 1. Factored bearing resistances for concrete block masonry assume a compressive unit strength of 15.0 MPa (net area) using solid or grouted units as per Table 4 of CSA S304-14.
- 2. Factored bearing resistances for concrete assume a 28-day compressive strength (f'c) of 20 MPa as per CSA A23.3-14.
- 3. Bearing resistance of wood member will govern for these applications. Calculate factored resistance in accordance with CSA 086-14.

 4. Bearing resistances shown assume a glulam width of 10½". For smaller widths, ensure that
- the factored bearing resistance of the wood member does not govern.
- 5. The GLBT5 has a WT4x9 structural tee; the GLTB6 has a WT4x12 structural tee.
- 6. Specify "W" dimension when ordering HGLB and GLTB beam seats.
- 7. Factored horizontal resistances include a 15% increase for short term loading; reduce if masonry or concrete is limiting.

PAI/MPAI Purlin Anchors

Wood-to-concrete and -concrete block connectors. The PA's dual embedment line allows installation in concrete or concrete block.

MATERIAL: MPAI—14 gauge; PAI—12 gauge

FINISH: Galvanized. Some products available HDG or ZMAX® coating.

INSTALLATION: • Minimum concrete strength is 15 MPa.

- Use all specified fasteners; some models have extra fastener holes. See General Notes.
- Wood splitting may occur when anchor is nailed to wood less than 3½" wide. To reduce splitting for widths less than 31/2", fill every other nail hole with 10dx1½" nails. Reduce the factored resistance based on the size and quantity of fasteners used. (See nail table on page 22.)

EDGE DISTANCE—Minimum concrete edge distance is 5". Minimum concrete block left-to-right edge distance is 20".

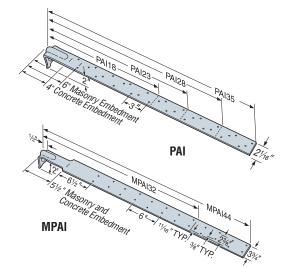
CONCRETE BLOCK WALLS—The masonry embedment line on PAI, MPAI allows for 4" of grout embedment in a standard 8" concrete masonry unit.

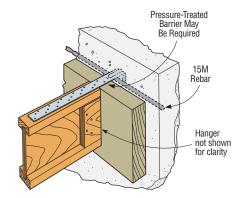
The minimum wall specifications are:

- A One 15M vertical rebar, 32" long, 16" each side of anchor;
- **B** Two courses of grout filled block above and below the anchor (no cold joints allowed);
- C A horizontal bond beam with two 15M rebars, 40" long, a maximum of two courses above or below the anchor.

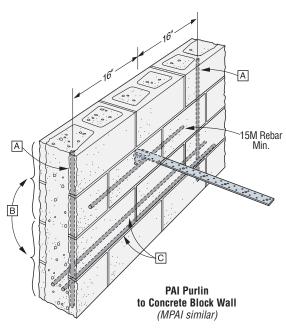
All cells grouted with 15MPa 3/8" aggregate grout. Grout shall be vibrated per the Code. Rebar quantities, sizes and lengths are minimum requirements and may be increased per any additional wall design requirements.

		Faste	eners	Factore	d Tensile Re	sistance (Kı) = 1.15)
				D.F	ir-L	S-	P-F
Model	del L o. (in) Masonry	0	Masonry	Concrete	Masonry	Concrete	
NU.	(111)	wasonry	Concrete	lbs	lbs	lbs	lbs
				kN	kN	kN	kN
			No	Ledger			
PAI18	10	40.40441/	12-10dx1½	2910	3490	2580	3095
PAH8	18	10-10dx1½	12-100X1 ½	12.94	15.52	11.48	13.77
PAI23	23	15-10dx1½	17-10dx1½	3980	4945	3870	4385
PAIZS	23	15-10ux172	17-10ux 1 72	17.70	22.00	17.22	19.51
PAI28	29	21-10dx11/2	23-10dx1½	3980	5215	3980	5215
1 7120	23	Z1-10ux172	25-100X172	17.70	23.20	17.70	23.20
PAI35	35	26-10dx11/2	29-10dx1½	3980	5215	3980	5215
I Aloo	00	20-10ux1/2	23-10ux1/2	17.70	23.20	17.70	23.20
MPAI32	32	16-10dx11/2		3920	_	3615	_
IVIFAIOZ	JZ	10-10ux172		17.44	_	16.08	_
MPAI44	44	24-10dx1½		4055	_	4055	_
IVIT A144	44	24-10ux172	_	18.04	_	18.04	
			1¾" LVL a	and 2x Ledge	er		
PAI18	18	9-10dx1½	11-10dx1½	2620	3200	2320	2840
1 7110	10	9-10ux172	11-10UX172	11.65	14.23	10.32	12.63
PAI23	23	14-10dx1½	16-10dx1½	3980	4655	3610	4130
I AILU	20	14 10ux172	10 100X172	17.70	20.71	16.06	18.37
PAI28	29	20-10dx1½	22-10dx1½	3980	5215	3980	5215
I AIZU	23	20 10ux172	22 100X172	17.70	23.20	17.70	23.20
PAI35	35	26-10dx11/2	28-10dx11/2	3980	5215	3980	5215
171100	00	20 TOUX172	20 TOUX 172	17.70	23.20	17.70	23.20
MPAI32	32	16-10dx11//		3920	_	3615	_
1111 71102	02	10 100/172		17.44	_	16.08	_
MPAI44	44	24-10dx1½	_	4055	_	4055	_
				18.04	_	18.04	_
			2-2x an	d 4x Ledger			
PAI18	18	7-10dx1½	9-10dx1½	2035	2620	1805	2320
				9.05	11.65	8.03	10.32
PAI23	23	12-10dx1½	14-10dx1½	3490	3980	3095	3610
				15.52	17.70	13.77	16.06
PAI28	29	18-10dx1½	20-10dx1½	3980	5215	3980	5160
				17.70	23.20	17.70	22.95
PAI35	35	24-10dx1½	26-10dx1½	3980	5215	3980	5215
				17.70	23.20	17.70	23.20
MPAI32	32	16-10dx1½	_	3920	_	3615	_
		-	A /13 C	17.44		16.08	
MPAI44	44	24-10dx1½	$\Lambda A \Lambda$	4055	ID	4055	1/1
			<u> </u>	13.04		18.04	





PAI Purlin to Concrete Wall (MPAI similar)



1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed; reduce where other loads govern.

2. Factored resistances are for horizontal installation

into the side of a concrete or masonry wall. **IAIL S:** 10d; $116 = 0.148^\circ$ dia. \times $1\frac{1}{2}^\circ$ long. See pages 22-23 for other nail sizes and information.

LTT/HTT Tension Ties

SIMPSON
Strong-Tie

The HTT4 and HTT5 are the latest generation of tension ties. They feature an optimized nailing pattern which results in better performance with less deflection. Designed to meet new code standards, the HTT4 and HTT5 offer higher capacities than their predecessors.

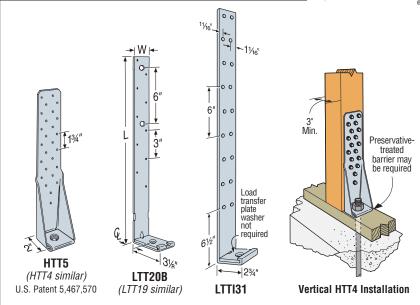
The LTT19 Light Tension Tie is designed for 2x joists or purlins and the LTT20B is for nail- or bolt-on applications. The 3" nail spacing makes the LTT20B suitable for wood I-joists with 10dx1½. The LTTI31 is designed for wood chord open web truss attachments to concrete or masonry walls and may also be installed vertically on a minimum 2x6 stud.

MATERIAL: See table FINISH: Galvanized

INSTALLATION: • Use all specified fasteners.

See General Notes.

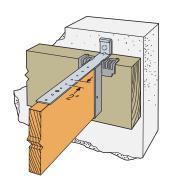
- Use the specified number and type of nails to attach the strap portion to the top or side of purlin or beam (minimum 4x width (2-2x4 or 4x4), except LTT19). Bolt the base to the wall or foundation with a suitable anchor; see table for the required bolt diameter.
- Do not install LTT tension ties raised off the mudsill.
- See AT-XP® anchoring adhesive, page 29.
- To tie multiple 2x members together, the Designer must determine the fasteners required to join members to act as one unit without splitting the wood.



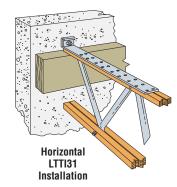
For tension ties, per ASTM test standards, anchor bolt nut should be finger-tight plus ½ to ½ turn with a hand wrench, with consideration given to possible future wood shrinkage. Care should be taken to not over-torque the nut. Impact wrenches should not be used.

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

Madal		Di	mensio (in)	ns	Fast	eners	Minimum		ile Resistance 1.15)	Deflection ⁶ at Factored
Model No.	Ga				Anchor		Wood Thickness	D.Fir-L	S-P-F	Resistance
NO.		W	L	© 10	Bolt Dia.	Fasteners	(in)	lbs	lbs	in
				_	(in)		, ,	kN	kN	mm
						8-10dx1½	3	1795	1645	0.243
LTT19	16	13/4	191/8	1 ½16	3/4	0-10UX172	S	7.98	7.32	6.17
LITIS	10	194	1978	I 716	94	8-10d	3	1930	1785	0.241
						0-10u	S	8.59	7.94	6.12
						10-10dx1½	3	1900	1680	0.250
						10-10ux 1 72	S	8.45	7.47	6.35
LTT20B	12	2	19¾	11/2	3/4	10-10d	3	2100	1840	0.250
LIIZUD	12	2	1374	1 /2	74	10-100	J	9.34	8.19	6.35
						2-1/2" Bolts	3	2270	2115	0.250
						Z-72 DUILS	3	10.10	9.41	6.35
						18-10dx1½	3	1890	1560	0.250
LTTI31	18	3¾	31	17/16	5/8	10 10ux172	0	8.41	6.94	6.35
LITIOI	10	074	01	1710	/ 0	18-10dx1½	3	3120 ⁸	2845 ⁸	0.250
						10 10ux172	U	13.888	12.668	6.35
						18-10dx1½	3	4580	4020	0.175
HTT4	11	2½	12%	15/16	5/8	10 10ux172	J J	20.37	17.88	4.45
111114	''	L /2	12/0	1710	76	18-16d	3	6000	5265	0.225
						10 100	Ů	26.69	23.42	5.72
						26-10dx1½	3	6565	5760	0.250
					-	20 TOUX 172		29.20	25.62	6.35
HTT5	11	21/2	16	15/16		26-10d	3	6720	5895	0.250
	''	- /′	10	1710		26-10d	3	29.89	26.22	6.35
						26-16d	3	7125	6255	0.250
						20 100	J	31.69	27.82	6.35



Horizontal LTT19 Installation (LTT20B similar)



- Factored resistances have been increased 15% for short term load duration. Reduce where other load durations govern.
- The Designer must specify anchor bolt type, length and embedment to ensure adequate anchorage to concrete.
- Post design shall be by the Designer. Tabulated values are based on a minimum wood member thickness in the direction of the fastener penetration. Post may consist of multiple 2x members provided they are connected independently of the holdown fasteners.
- Holdowns shall be installed centred along the width of the attached post.
 Deflection at Factored Resistance includes fastener slip, holdown elongation
- 5. Deflection at Factored Resistance includes fastener slip, holdown elongation and anchor bolt elongation (L = 6°). Additional elongation of anchor bolts shall be accounted for by the designer when the length of the anchor bolt above the top of the concrete to the attachment at the hollown is longer than 6. Similar consideration for floor to floor connections must be addressed by the Designer.
- Deflection values may be reduced linearly for lesser loads including specified wind loads at h/500.
- 7. A ½" or ¾" diameter anchor bolt may be used for the LTT19 or the LTT20B. A standard cut washer is required between the anchor bolt nut and the bearing seat of the tension tie for this application.
- 8. When the LTTI31 is installed with the base flush with the concrete or masonry wall this increased resistance applies.
- When using structural composite lumber columns, screws must be applied to the wide face of the column (see technical bulletin T-SCLCOLUMCAN for details).
- Centre line dimension is taken from the face of the post/framing member to
 the centre of anchor
- the centre of anchor.

LSTHD/STHD Strap-Tie Holdown WEINEERED



This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

The STHD is an embedded strap-tie holdown offering high load capacity and a staggered nail pattern to help minimize splitting. The STHD incorporates many features that aid correct installation and improve performance. When installed on the forms with the StrapMate® strap holder the unique design of the STHD delivers enhanced stability before and during the pour to help prevent both parallel and perpendicular movement (relative to the form). This results in accurate positioning of the strap and reduced possibility of spalling.

FEATURES

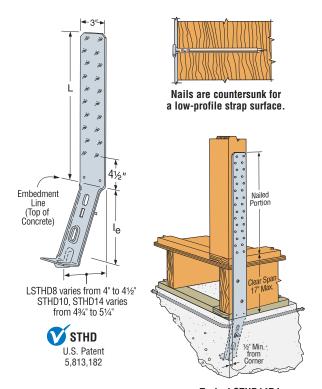
- The nailing pattern allows for nailing to the edges of double 2x's
- Strap nail slots are countersunk to provide a lower nail head profile
- The slots below the embedment line enable increased front-to-back concrete bond and help to reduce spalling
- Rim joist models accommodate up to a 17" clear span without any loss of strap nailing

MATERIAL: LSTHD-14 gauge, STHD-12 gauge

FINISH: Galvanized

INSTALLATION: • Use all specified fasteners. See General Notes.

- Install before concrete pour with a StrapMate, or other holding device.
- Nail strap from the bottom up.
- Strap may be bent one full cycle (bent horizontal 90° then bent vertical) to aid wall placement, but may cause spalling behind the strap. If the spall is 1" or less, measured from the embedment line to the bottom of the spall, full values apply. Any portion of the strap left exposed should be protected against corrosion.
- Unless otherwise noted, do NOT install where: (a) a horizontal cold joint exists within the embedment depth between the slab and foundation wall or footing beneath, unless provisions are made to transfer the load, or the slab is designed to resist the load imposed by the anchor; or (b) slabs are poured over concrete block foundation walls.
- · Additional studs attached to the shearwall studs or post may be required by the Designer for wall sheathing nailing.
- · Wood shrinkage after strap installation across horizontal members may cause strap to buckle outward.



Typical STHD14RJ **Rim Joist Application**

Factored Resistances for Wind and Seismic $I_EF_aS_a(0.2) < 0.35$

Min.	Mor	lel No.	Strap Length				Factored Tensile Resistance (K _D = 1.15)					
Stem	IVIOL	IGI NU.	(L) (i	n)	١		N	on Cracke	d		Cracked	
Wall				Dim	le (in)	Fasteners	Midwall	Corner	Endwall	Midwall	Corner	Endwall
Width	Standard	Rim Joist	Standard	Rim Joist	(111)		lbs	lbs	lbs	lbs	lbs	lbs
(in)				JUIST			kN	kN	kN	kN	kN	kN
	LSTHD8	LSTHD8RJ	185/8	321/8	8	20-10d	4625	4130	2515	3975	3550	2160
	LSTHUO	LOINDON	1078	3278	0	20-10u	20.57	18.37	11.19	17.68	15.79	9.61
6	STHD10	STHD10RJ	24%	381/8	1/8 10	24-10d	5485	5485	3045	4470	4470	2480
0	3111010	SIUDIONJ	2478	3078	10	27-10u	24.40	24.40	13.55	19.88	19.88	11.03
	STHD14	STHD14RJ	261//8	395/8	14	14 30-10d	7655	7655	4755	7655	7655	4755
	3111114	SINDIANS	2078	3978	14	30-10u	34.05	34.05	21.15	34.05	34.05	21.15
	LSTHD8	LSTHD8RJ	185/8	321/8	8	20-10d	4625	4015	3310	3975	3450	2845
	LOTTIDO	LOTTIDONS	1078	3278	0	20-10u	20.57	17.86	14.72	17.68	15.35	12.66
8	STHD10	CTUD10D I	2.45%	381/8	10	24-10d	7400 ³	6320	4670	6070	5150	3810
0	3111010	STHD10RJ	24%	3078	10	2 4 -100	32.92 ³	28.11	20.77	27.00	22.91	16.95
	STHD14	STHD14RJ	261/6	39% 14	14 30-10d	8800 ⁴	8195 ⁴	6185	7960 ⁴	7350	5550	
	3111014		261/8			30-10u	39.15 ⁴	36.45 ⁴	27.51	35.414	32.70	24.69

Factored Resistances for Seismic $I_EF_aS_a(0.2) \ge 0.35$

Min.	Mod	lel No.	Strap Le	ength				Factored '	Tensile Re	sistance ($K_D = 1.15$)
Stem	IVIOU	IEI NU.	(Ľ) (i	n)	١.		N	on Cracke	ed		Cracked	
Wall				D	le (in)	Fasteners	Midwall	Corner	Endwall	Midwall	Corner	Endwall
Width	Standard	Rim Joist	Standard	Rim Joist	(111)		lbs	lbs	lbs	lbs	lbs	lbs
(in)				JUIST			kN	kN	kN	kN	kN	kN
	LSTHD8	LSTHD8RJ	185%	321/8	8	20-10d	3470	3100	1885	2980	2660	1620
	L91UD0	гэтипока	1078	3278	0	20-10a	15.44	13.79	8.39	13.26	11.83	7.21
6	STHD10	STHD10RJ	245%	381/8	31/8 10	24-10d	4110	4110	2280	3350	3350	1860
U	3111010	SIUDIOUS	2478	3078		10 2	24-10u	18.28	18.28	10.14	14.90	14.90
	STHD14	STHD14RJ	261/8	39%	14	30-10d	5740	5740	3565	5740	5740	3565
	3111014	3111D14N3	2078	3378	14	30-10u	25.53	25.53	15.86	25.53	25.53	15.86
	LSTHD8	LSTHD8RJ	185/8	321/8	8	20-10d	3470	3010	2485	2980	2585	2135
	LOTTIDO	LOTTIDONS	1078	3278	0	20-10u	15.44	13.39	11.05	13.26	11.50	9.50
8	STHD10	STHD10RJ	24%	381/8	10	24-10d	5550	4740	3505	4525	3865	2855
o	3111010	0 STHDTORJ 24% 38%	10	24-10u	24.69	21.09	15.59	20.13	17.19	12.70		
	STHD14	STHD14RJ	BJ 26½ 39%	11	30-10d	6655	6145	4640	5970	5510	4160	
	3111014	311101400	2078	V	V * .	30-100	29.60	27.34	20.64	26.56	24.51	18.51

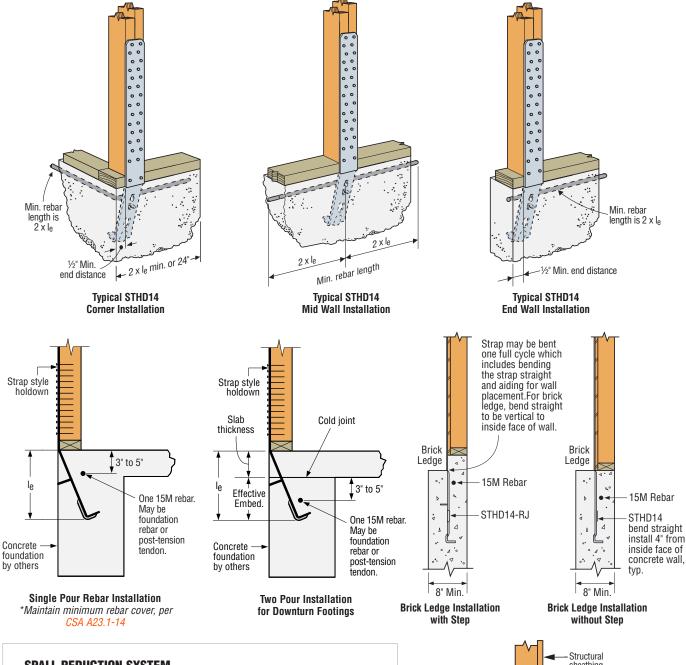
- 1. Factored resistances have been developed based on testing per ICC-ES AC 398 using the corresponding adjustment factors from CSA A23.3-14 Annex D. 2. Unless otherwise noted, tabulated
- values are applicable to D.Fir-L and S-P-F framing members. 3. S-P-F factored resistance is 7210 lbs (32.07 kN).
- 4. S-P-F factored resistance is 7725 lbs (34.36 kN).
- 5. The minimum 28 day concrete compressive strength (f'c) shall be 2500 psi (17.25 MPa)
- 6. The minimum centre-to-centre spacing is 3 times the required embedment depth ($S_{min} = 3xI_e$).
- 7. See T-SCLCOLUMNCAN for installation on structural composite lumber posts or studs.
- Deflection at the highest factored resistance for installation over double studs are as follows:
- Installed on framing: LSTHD8 = 0.094" STHD10 = 0.157"
- STHD14 = 0.135"
- Installed over structural sheathing: LSTHD8 = 0.159" STHD10 = 0.201" STHD14 = 0.290
- Deflection values shown are applicable for D.Fir-L studs. For attachment to S-P-F studs multiply the deflection values by 1.13.
- 9. Deflection values may be reduced linearly for lesser loads including specified wind loads at h/500.
- 10. Use the specified number of nails listed. In some cases, not all nail holes will be filled. Nail strap from bottom up. **NAILS:** 10d = 0.148" dia. x 3" long.

and information.

ee pages 22-23 for other nail sizes

LSTHD/STHD Strap-Tie Holdown





SM1

U.S. Patent 6.796.099

SPALL REDUCTION SYSTEM FOR STHD STRAP TIE HOLDOWN

FEATURES

- Built-in tab.
- StrapMate® locator line.
- Additional diamond hole in RJ versions.

BENEFITS

Built-in Tab:

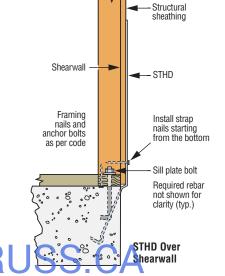
- · Reduces spalling and costly retrofits.
- · No additional labor to install.
- Holds STHD away from form board.

StrapMate Locator Line:

- Easy inspection to ensure proper location.
- Allows adjustment without removing STHD.

Additional Diamond Hole:

 One more fastener to help prevent the STHD RJ models from bowing out at the rim joist section.



HDU/DTT2Z Holdown

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

The HDU series of holdowns combine the advantages of low deflection and high capacity from the pre-deflected geometry with the ease of installation of Simpson Strong-Tie® patented Strong-Drive® SDS Heavy-Duty Connector screws.

The DTT2Z tension tie is suitable for lighter-duty holdown applications on single or double 2x posts, and installs easily with Strong-Drive SDS Heavy-Duty Connector screws (included).

HDU SPECIAL FEATURES:

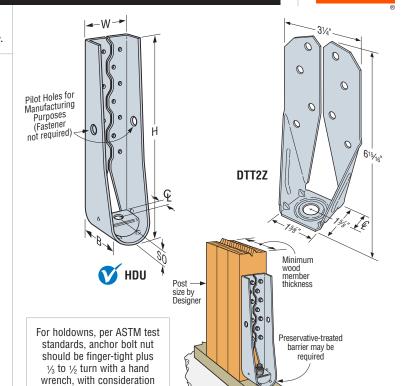
- Pre-deflected body virtually eliminates deflection due to material stretch.
- Uses Strong-Drive SDS Heavy-Duty Connector screws which install easily, reduce fastener slip, and provide a greater net section area of the post compared to bolts.
- ¼"x2½" Strong-Drive SDS Heavy-Duty Connector screws are supplied with the holdowns. (Lag screws will not achieve the same capacity.) This ensures the proper fasteners are used and is convenient for the installer.
- No stud bolts to countersink at openings.

MATERIAL: See table

FINISH: HDG—Galvanized, DTT2Z—ZMAX® coating, and DTT2SS—Stainless steel

INSTALLATION: • Use all specified fasteners. See General Notes.

- Place the holdown over the anchor bolt.
- No additional washer required for HDU, the DTT2Z requires standard cut washer (included) be installed between the nut and seat.
- To tie multiple 2x members together, the Designer must determine the fasteners required to join the members to act as one unit without splitting the wood. See page 26 for Strong-Drive SDW values.
- See SB and SSTB Anchor Bolts on pages 38-42 for anchorage options.
- Strong-Drive SDS Heavy-Duty Connector screws install best with a low speed high torque drill with a \%" hex head driver.



These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

given to possible future

wood shrinkage. Care

should be taken to not

over-torque the nut. Impact

wrenches should not be used.

	80 - 4 - 1			Dim	ensio	ns		Fa	asteners	Minimum	Factored Resistance		Deflection ^{7,8} at Factored
	Model No.	Ga						Anchor	SDS	Wood Thickness	D.Fir-L	S-P-F	Resistance
	NO.		W	Н	В	© 11	SO	Bolt Dia.	Screws	(in)	lbs	lbs	in
								(in)	Outows	()	kN	kN	mm
									8-1/4"x11//" SDS	11/2	2805	2520	0.25
	DTT2Z								0-74 X172 3D3	1 /2	12.48	11.21	6.35
	DITZL	14	3½	615/16	1%	15/16	3/16	1/2	8-¼"x1½" SDS	3	3060	2565	0.25
		14	J /2	0 /16	178	/16	/16	/2	0-74 X172 3D3	J	13.61	11.41	6.35
	DTT2Z-SDS2.5								8-1/4"x21/2" SDS	3	3060	2950	0.25
	D1122-0002.0								0-74 XZ72 3D3	3	13.61	13.12	6.35
	HDU2-SDS2.5	14	27/8	811/16	31/4	13%	13/8	5/8	6-1/4"x21/2" SDS	3	3210	2900	0.092
	11002 0002.0	17	2/0	0 716	074	170	170	70	0 74 XZ72 0D0	0	14.28	12.90	2.34
	HDU4-SDS2.5	14	27/8	1015/16	31/4	13%	13/8	5/8	10-¼"x2½" SDS	3	5350	4515	0.13
	11004 0002.0	١٦_	L/0	10 716	074	170	170	70	10 74 XZ72 0D0		23.80	20.08	3.30
	HDU5-SDS2.5	14	27/8	133/16	31/4	13%	13/8	5/8	14-1/4"x21/2" SDS	3	7485	6130	0.153
	11000 0002.0	17	270	10710	074	170	170	76	14 74 KE72 ODO	0	33.30	27.27	3.89
										3	9130	7330	0.124
	HDU8-SDS2.5	10	3	165%	31/2	13/8	11/2	7/8	20-¼"x2½" SDS		40.61	32.61	3.15
	11000 0002.0	10	O	1078	072	170	172	76	20 74 8272 000	41/2	12890	9280	0.190
										772	57.34	41.28	4.83
										51/2	14090	10145	0.196
	HDU11-SDS2.5	10	3	221/4	31/2	13/8	11/2	1	30-1/4"x21/2" SDS	072	62.68	45.13	4.98
	11011 0002.0		Ü		0,2	170	172	i i	00 74 8272 000	71/4	16985	12230	0.197
										174	75.56	54.40	5.00
										71/4	20930	15070	0.250
	HDU14-SDS2.5	7	31/8	2511/16	31/2	1%6	1%6	1	36-1/4"x21/2" SDS		93.10	67.04	6.35
_	5 5252.0	, i	2.3		0,2					51/29	20850	15010	0.250
										٠,٠	92.75	66.77	6.35

Factored resistances have been increased 15% for short term load duration. Reduce where other load durations govern.

Vertical HDU

Installation

SIMPSON

- The Designer must specify anchor bolt type, length and embedment to ensure dequate anchorage to concrete.
- When using structural composite lumber columns, screws must be applied to the wide face of the column (see technical bulletin T-SCLCOLUMNCAN for details).
- 4. Post design shall be by the Designer. Tabulated values are based on a minimum wood member thickness in the direction of the fastener penetration. Post may consist of multiple 2x members provided they are connected independently of the holdown fasteners.
- 5. Holdowns shall be installed centred along the width of the attached post.
- 6. Tension values are valid for holdowns flush or raised off of the sill plate.
- 7. Deflection at Factored Resistance includes fastener slip, holdown elongation and anchor bolt elongation (L = 6"). Additional elongation of anchor bolts shall be accounted for by the Designer when the length of the anchor bolt above the top of the concrete to the attachment at the holdown is longer than 6". Similar consideration for floor to floor connections must be addressed by the Designer.





- 8. Deflection values may be reduced linearly for lesser loads including specified wind loads at h/500.
- 9. Noted HDU14 factored resistances are based on a 51/2" wide (6x6) post. All other resistances assume

10 HDU14 requires heavy hex nut for anchor bolt (supplied with hold bwn).
11. Centre line dimension is taken toon, the face of the post/framing member to the centre of anchor.

HDQ8/HHDQ Holdowns



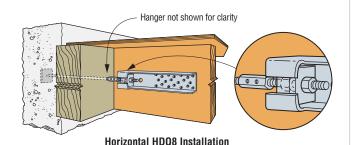
The HHDQ series of holdowns combines low deflection and high loads with ease of installation. The unique seat design of the HDQ8 greatly minimizes deflection under load. Both styles of holdown employ the Simpson Strong-Tie® Strong-Drive® SDS Heavy-Duty Connector screws which install easily, reduce fastener slip and provide a greater net section area of the post when compared to bolts. They may be installed either flush or raised off the mudsill without a reduction in capacity.

SPECIAL FEATURES:

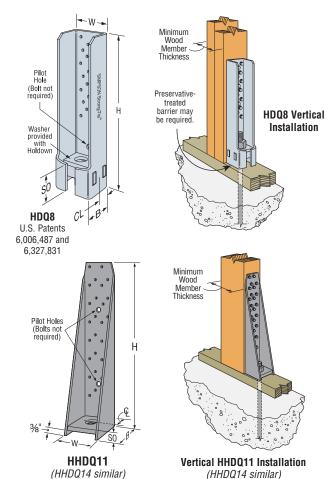
- · Uses Strong-Drive SDS Heavy-Duty Connector screws which install easily, reduce fastener slip, and provide a greater net section area of the post compared to bolts.
- · Strong-Drive SDS Heavy-Duty Connector screws are supplied with the holdowns to ensure proper fasteners are used.
- · No stud bolts to countersink at openings.

MATERIAL: HDQ8—7 gauge; HHDQ—Body: 7 gauge, washer: 1/2" plate FINISH: HDQ8—Galvanized; HHDQ— Simpson Strong-Tie® gray paint INSTALLATION: • Use all specified fasteners. See General Notes.

- For use in vertical and horizontal applications.
- · No additional washer is required.
- To tie multiple 2x members together, the Designer must determine the fasteners required to join members without splitting the wood.
- See SB and SSTB Anchor Bolts on pages 38-42 for anchorage options.
- · Strong-Drive SDS Heavy-Duty Connector screws install best with a low speed high torque drill with a 3/8" hex head driver.
- HDQ8 has 5/4" of adjustability perpendicular to the wall.
- HHDQ14 requires a heavy hex anchor nut (supplied with holdown).



These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.



For holdowns, per ASTM test standards, anchor bolt nut should be finger-tight plus 1/3 to 1/2 turn with a hand wrench, with consideration given to possible future wood shrinkage. Care should be taken to not over-torque the nut. Impact wrenches should not be used

Model			Di	mensioi (in)	18			Fasteners	Minimum Wood	Factored Tens (K _D =		Deflection ^{7,8} at Factored
No.	Ga						Anchor		Thickness	D.Fir-L	S-P-F	Resistance
		W	Н	В	Ģ ¹¹	80	Bolt Dia.	SDS Screws	(in)	lbs	lbs	in
							(in)			kN	kN	mm
									3	9825	7075	0.112
HDQ8-SDS3	7	2 ½	14	2½	11/4	23/8	7/8 20-1/4"x3" SDS		J	43.71	31.47	2.84
11000-3033	'	2/8	14	Z /2	1 /4	278	/8	20-¼"x3" SDS	4½	13885	9995	0.139
									4 72	61.77	44.46	3.53
HHDQ11-SDS2.5	7	3	151/8	3½	1%16	7/8	1	24-1/4"x21/2" SDS	5½	16285	12420	0.218
ппиш11-3032.3	1	3	1378	372	I 7/16	78	'	24-74 XZ72 3D3	372	72.44	55.25	5.54
									71/4	17510	12610	0.168
HHDQ14-SDS2.5	7	3	18¾	3½	1%6	7/8	,	30-1/4"x21/2" SDS	1 74	77.89	56.09	4.27
ППИЦ14-3И32.3	'	3	1074	3 72	I 716	78	'		5½9	20355 ^{9,10}	16280 ^{9,10}	0.140
									J 72°	90.55	72.42	3.56

- 1. Factored resistances have been increased 15% for short term load duration. Reduce where other load durations govern.
- 2. The Designer must specify anchor bolt type, length and embedment to ensure adequate anchorage to concrete.
- 3. When using structural composite lumber columns, screws must be applied to the wide face of the column (see technical bulletin T-SCLCOLUMNCAN for details).
- 4. Post design shall be by the Designer. Tabulated values are based on a minimum wood member thickness in the direction of the fastener penetration. Post may consist of multiple 2x members provided they
- 7. Deflection at Factored Resistance includes fastener slip, holdown elongation and anchor bolt elongation (L = 6"). Additional elongation of anchor bolts shall be accounted for by the Designer when the length of the anchor bolt above the top of the concrete to the attachment at the holdown is longer than 6". Similar consideration for floor to floor connections must be addressed by the Designer.
- Deflection values may be reduced linearly for lesser loads including specified wind loads at h/500.
- Noted HHDQ14 factored resistances are based on a 51/2" wide (6x6) post. All other resistances assume 31/2" wide posts (minimum).
- 10. Requires heavy hex nut for anchor bolt (supplied with holdown).
- Centre line dimension is taken from the face of the post/framing member centre of ancho

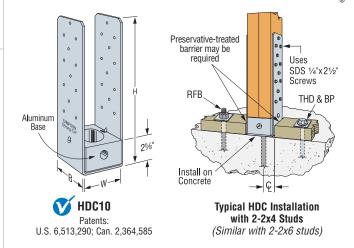
DC <u>Concentric</u> Holdown

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

The unique design of the HDC holdowns eliminate eccentricity. They install with Simpson Strong-Tie® Strong-Drive® SDS Heavy-Duty Connector screws (included) to reduce slip and provide a greater net section area of the post compared to bolts.

MATERIAL: 10 gauge strap FINISH: Galvanized strap, aluminum base INSTALLATION: • Use all specified fasteners. See General Notes.

- · Install on concrete.
- For use in vertical and horizontal applications.
- Sized for 2-2x and 4x. Centre posts on holdown.
- Uses Strong-Drive SDS Heavy-Duty Connector screws supplied with the holdowns to ensure proper fasteners are used.
- Slot in the seat allows for 3/8" of adjustment perpendicular to plate.
- Narrow cut washer with outside diameter of 13/4" required between base and anchor nut.
- · Witness slot in the base to inspect the nut .
- Maximum anchor bolt height above concrete is 21%".
- To tie multiple 2x members together, the Designer must determine the fasteners required to join members without splitting the wood.
- Aluminum standoff cannot be in contact with preservative-treated wood.
- · Strong-Drive SDS Heavy-Duty Connector screws install best with a low speed high torque drill with a 3/8" hex head driver.



SIMPSON

For holdowns, per ASTM test standards, anchor bolt nut should be finger-tight plus 1/3 to 1/2 turn with a hand wrench, with consideration given to possible future wood shrinkage. Care should be taken to not over-torque the nut. Impact wrenches should not be used.

Model	Doct		Dimer (i			Fa	asteners		ile Resistance 1.15)	Deflection ^{5,6} at Factored	Concrete Bearing
Model No.	Post Size					Anchor		D.Fir-L	S-P-F	Resistance	$(f'_c = 20 MPa)$
	0.20	W	Н	В	မ ို	Bolt Dia.	SDS Screws	lbs	lbs	in	lbs
						(in)		kN	kN	mm	kN
HDC10/22-SDS2.5	2-2x4	31/8	14½	3	111/16	7/8 24-1/4"x21/2" SDS		11785	8485	0.086	12940
Предо/22-2022.3	2-284	378	1472	٥	I 716	78	24-74 X272 3D3	52.42	37.74	2.18	57.56
HDC10/4-SDS2.5	4x4	3%6	141/4	3	17/8	7/8	24-1/4"x21/2" SDS	11785	8485	0.086	15755
110010/4-3032.3	484	J /16	1474	٥	1 78	/8	Z4-74 XZ72 3D3	52.42	37.74	2.18	70.08

- 1. Factored resistances have been increased 15% for short term load duration. Reduce where other load durations govern.
- 2. The Designer must specify anchor bolt type, length and embedment to ensure adequate anchorage to concrete.
- When using structural composite lumber columns, screws must be applied to the wide face of the column (see technical bulletin T-SCLCOLUMNCAN for details). Concrete bearing resistance has been calculated in accordance with 10.8 CSA
- A23.3-14 and may be increased when HDC is not placed near an edge or with $f'_{\rm C} > 20$ MPa to a maximum value of 21310 lbs (94.80 kN).
- 5. Deflection at Factored Resistance includes fastener slip, holdown elongation and anchor bolt elongation ($L=6^{\circ}$). Additional elongation of anchor bolts shall be accounted for by the Designer when the length of the anchor bolt above the top of the concrete to the attachment at the holdown is longer than 6". Similar consideration for floor to floor connections must be addressed by the Designer.
- 6. Deflection values may be reduced linearly for lesser loads including specified wind loads at h/500
- The post capacity must be verified by the Designer.
- 8. Centre line dimension is taken from the outside face of the HDC to the centre of anchor.

HDB/HD Holdowns

Simpson Strong-Tie offers a wide range of bolted holdowns offering lowdeflection performance for a range of load requirements. All of these holdowns have been tested in accordance with ICC-ES's AC 155 acceptance criteria.

The HD3B is light-duty holdown designed for use in shearwalls and braced-wall panels, as well as other lateral applications.

The HD5B, HD7B and HD9B bolted holdowns incorporate the proven design of our HDQ8 SDS-style holdown and feature a unique seat design which greatly minimizes deflection under load. HDB holdowns are self jigging, ensuring that a minimum of seven bolt diameters from the end of the post is met. They can be installed directly on the sill plate or raised above it and are suitable for back-to-back applications where eccentricity is a concern. HDBs are designed to provide resistances for intermediate-load-range shearwalls, braced-wall panels and lateral applications.

HD holdowns offer the highest bolted capacities for both vertical and horizontal applications. The HD12 and HD19 are self jigging, ensuring that a minimum of seven bolt diameters from the end of the post is met. They can be installed back-to-back when eccentricity is an issue.

MATERIAL: See table

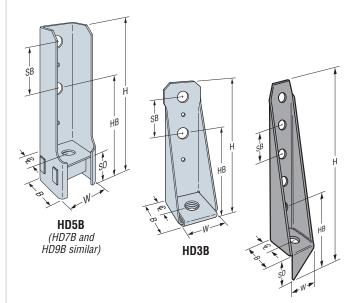
FINISH: HD3B/HD5B/HD7B/HD9B - Galvanized;

HD - Simpson Strong-Tie® gray paint

INSTALLATION: • Use all specified fasteners. See General Notes.

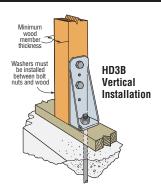
- Bolt holes shall be a minimum of 1/32" to a maximum of 1/16" larger than the bolt diameter (per 12.4.1.2 CSA 086-14).
- Stud bolts should be snugly tightened with standard cut washers between the wood and nut.
- The Designer must specify anchor bolt type, length, and embedment.
- See SB and SSTB Anchor Bolts. (pages 38-42).

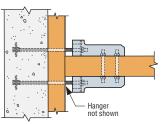
 To tie multiple 2x members togethen the Designer must determine CONTRUSS.CA
 the fasteners required to join members without splitting the wood.



HD19 (HD12 similar)

HDB/HD Holdowns





Horizontal HDB Installation (Plan View)

Minimum wood member thickness Washers must be installed between bolt nuts and wood Stand off provides minimum end distance to end of post from post bolt Vertical HD19

Installation

For holdowns, per ASTM test standards, anchor bolt nut should be finger-tight plus 1/3 to 1/2 turn with a hand wrench, with consideration given to possible future wood shrinkage. Care should be taken to not over-torque the nut. Impact wrenches should not be used.

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

HD/HDB Holdown Resistances

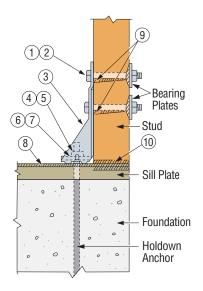
Mada	Dark	Dimensions (in)							Fa	steners		Wood		ile Resistance 1.15)	Deflection ^{8,9} at Factored
Model No.	Body Ga								Anchor	Stud	Bolts	Member Thickness	D.Fir-L	S-P-F	Resistance
NU.	ua	HB	SB	W	Н	SO	В	Ç ¹¹	Bolt Dia.	04	Dia.	(in)	lbs	lbs	in
								-	(in)	Qty.	(in)	(,	kN	kN	mm
												1½	1305	1030	0.118
												1 72	5.81	4.58	3.00
HD3B	12	43/4	2½	2½	85%	3/8	21/4	15/16	5/8	2	5/8	3	2610	2060	0.088
ספעח	12	474	Z 72	272	078	78	274	I 716	78		78	ر ا	11.61	9.16	2.24
												3½	3055	2412	0.114
												372	13.59	10.73	2.90
												3	3100	2445	0.106
												٥	13.79	10.88	2.69
HD5B	10	51/4	3	2½	93%	2	2½	11/4	5/8	2	3/4	3½	3630	2865	0.116
מפתח	10	374	3	272	978		Z/2	174	78		74	372	16.15	12.74	2.95
												41/2	4645	3670	0.142
												472	20.66	16.33	3.61
												3	4645	3670	0.107
												ر ا	20.66	16.33	2.72
												3½	5440	4295	0.121
HD7B	10	51/4	3	2½	12%	2	2½	11/4	7/8	3	3/4	372	24.20	19.11	3.07
מיטח	10	374	3	272	1278		Z/2	1 74	78	٥		4½	5965	4710	0.130
												472	26.53	20.95	3.30
												5½	5965	4775	0.130
												372	26.53	21.24	3.30
												31/2	5415	4275	0.112
												372	24.09	19.02	2.84
												41/2	8165	6445	0.155
HD9B	7	61/8	3½	27/8	14	23/8	21/2	11/4	7/8	3	7/8	472	36.32	28.67	3.94
טפטוו	1	0 /8	3/2	2/8	14	278	Z/2	1 /4	/8	٥	/8	5½	7915	6330	0.152
												J /2	35.21	28.16	3.86
												6	8320	6570	0.157
												0	37.01	29.23	3.99
												3½	9700	7660	0.150
												372	43.15	34.07	3.81
												41/2	12425	9810	0.166
HD12	3	7	4	3½	205/16	35⁄8	41/4	21/8	1	4	1	472	55.27	43.64	4.22
חטוב	3	/	4	372	20716	378	474	2 78		4	'	5½ ⁷	12045 ⁷	9635 ⁷	0.134
												3 72'	53.58	42.86	3.40
												67	16565 ⁷	13080 ⁷	0.171
												0.	73.69	58.19	4.34
											5½ ⁷	15060 ⁷	12045 ⁷	0.187	
HD19	3	7	4	3½	2/1/	35/8	41/4	21/	11/8	5	1	372	66.99	53.58	4.75
прія	3	1	4	3 //2	24½	3%	474	21/8	1 78	Э	1	67	20710 ⁷	16350 ⁷	0.229
												0,	92.12	72.73	5.82

- 1. Factored resistances have been increased 15% for short term load duration. Reduce where other load durations govern.
- 2. The Designer must specify anchor bolt type, length and embedment to ensure adequate anchorage to concrete.
- 3. When using structural composite lumber columns, bolts must be applied to the wide face of the column (see technical bulletin T-SCLCOLUMNCAN for details).
- 4. Post design shall be by the Designer. Tabulated values are based on a minimum wood member thickness in the direction of the fastener penetration. Post may consist of multiple 2x members provided they are connected independently of the holdown fasteners.
- 5. Holdowns shall be installed centred along the width of the attached post
- 6. Tension values are valid for holdowns flush or raised off of the sill plate.
 7. Noted HD12 and HD19 factored resistances are based on a 5½ wide post (6x6 or 4-2x6). All other resis ances assume 3½ wide posts minimum.
- 8. Deflection at Factored Resistance includes fastener slip, holdown elongation and anchor bolt elongation (L = 6"). Additional elongation of anchor bolts shall be accounted for by the Designer when the length of the anchor bolt above the top of the concrete to the attachment at the holdown is longer than 6". Similar consideration for floor to floor connections must be addressed by the Designer.
- 9. Deflection values may be reduced linearly for lesser loads including specified wind loads at h/500.
- The factored resistances shown are based on the lower of the assembly testing and the bolt calculations in accordance with 12.4 CSA 086-14. For applications where the HD or HDB holdowns are used on opposite sides of the post the capacity of the connection may be calculated using the lower of two times the tabulated value or the bolt calculations in accordance with 12.4 6SA 096-14 assuming double shear. Centre line dimension is taken from the face of the post/framing member to the cent e of anchor.

SOURCES OF DEFLECTION AT SHEARWALL HOLDOWN CONNECTIONS



- 1. Bolt slip can occur at holdown stud bolts.
- Increased bolt slip can occur if oversized holes are drilled through the stud for holdown stud bolts (oversized holes are when the hole diameter is greater than the bolt diameter plus 2mm per 12.4.1.2 CSA 086-14).
- When a holdown is installed on only one side of the stud, an eccentricity exists during loading which can cause more movement in the shearwall system.
- Unrestrained anchor bolt nuts can spin loose during cyclic loading; using steel nylon locking nuts or thread adhesive may prevent nut spin.
- Movement can occur when nuts are not tightened enough. Retightening bolts before covering wall may prevent this.
- Deflection can occur in the holdown under load caused by stresses due to earthquake or high wind.
- Lateral displacement at the top of the wall rotates the stud around its base causing the holdown base plate to displace vertically.
- 8. Wood shrinkage can occur due to drying of the sill plate, rim joist, and/or top plate; nuts may require retightening.
- 9. Uplift forces on the bolts can cause localized wood crushing at bolt bearing locations. Using larger bearing plates may prevent this.
- 10. Wood at the end of the studs (sill plates, rim joists, etc.) may crush under normal dead and live loading; additional compressive forces due to overturning during earthquake and high wind loads add to the deflection.



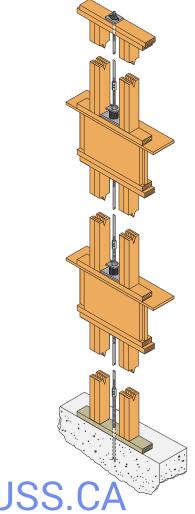
12 and 6 do not apply to the HDU.

NEED A HIGHER CAPACITY HOLDOWN?



When one of our conventional holdowns doesn't offer enough overturning capacity for a multistorey project, consider specifying the Simpson Strong-Tie® Anchor Tiedown System (ATS). ATS is a high-capacity overturning-restraint system commonly used in 2-6 storey wood structures to anchor stacked shearwalls. This continuous rod system features our patented shrinkage take-up devices, extends from the foundation to the top of the structure and is restrained (tied off) at each level. Designed properly, it can provide over 50,000 lbs. of overturning restraint; important when designing for the cumulative overturning forces in multi-storey buildings.

For more information see page 76 of this catalogue and for specification options see our *Anchor Tiedown Systems Canadian Limit States Design* catalogue (C-ATSCAN11) or visit *www.strongtie.com/ats*.



ABA/ABU/ABW Adjustable and Standoff Post Bases

Post Bases provide tested capacity. They feature 1" standoff height above concrete floors. They reduce the potential for decay at post and column ends.

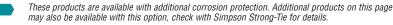
MATERIAL: See table

FINISH: Galvanized. Most available in ZMAX® coating;

see Corrosion Information, pages 14-17.

INSTALLATION: • Use all specified fasteners. See General Notes.

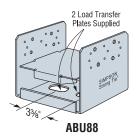
- Post bases do not provide adequate resistance to prevent members from rotating about the base and therefore are not recommended for non top-supported installations (such as fences or unbraced carports).
- Can be used with cast-in-place anchors. For epoxy or wedge anchors, select and install according to anchor manufacturer's recommendations; anchor diameter shown in table. Install required washer, which is not included for ABAs.
- ABU88 centre to centre dimension of anchors is 3%".
- See pages 31-32 for post-installed anchorage solutions.



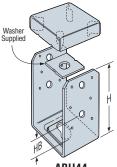
These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

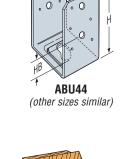


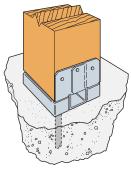












Typical ABA44 Installation

			Mat	erial	Di	mensio	ıns	East	onoro		Factored F	Resistance	
		Nominal	(g	a)		(in)		rasi	eners	D.F	ir-L	S-I	P-F
	Model No.	Post Size	Base	Strap	W	L	Н	Anchor Dia.	Nails	Uplift (K _D = 1.15)	Normal (K _D = 1.00)	Uplift (K _D = 1.15)	Normal (K _D = 1.00)
				o up		_		(in)		lbs	lbs	lbs	lbs
								` ,		kN	kN	kN	kN
	ABA44Z	4x4	16	16	3%16	31/8	31/16	1/2	6-10d	1030	10375	730	8610
	7107112	17.1	10	10	0710	070	0710	,,,	0 100	4.58	46.15	3.25	38.30
ÆW.	ABW44Z	4x4	16	16	39/16	39/16	21/4	1/2	8-10d	1635	9965	1160	8260
	7,01112	IXI	10	10	0710	0710	-/-	/-	0 100	7.27	44.33	5.16	36.74
	ABU44	4x4	16	12	3%16	3	5½	5/8	12-16d	2955	10940	2095	9070
	7.5011	.,,,			07.10		0,2	,,,	.2 .00	13.15	48.67	9.32	40.35
	ABA44R	RGH 4x4	16	16	41/16	31/8	213/16	1/2	6-10d	925	10630	655	8810
						• • •				4.11	47.29	2.91	39.19
NEW D	ABW44RZ	RGH 4x4	16	16	4	41/16	2	1/2	8-10d	1280	9965	905	8260
										5.69	44.33	4.03	36.74
	ABA46Z	4x6	14	14	39/16	53/16	31/8	5/8	8-16d	990	17200	705	14290
_										4.40	76.51	3.14	63.57
NEW	ABW46Z	4x6	12	16	3%16	5%16	3	1/2	10-10d	1415 6.29	6645 29.56	1005 4.47	5500
~										3490	29.56	2480	24.47 16765
	ABU46	4x6	12	12	3%16	5	7	5/8	12-16d	15.52	89.81	11.03	74.58
										990	15090	705	12515
	ABA46R	RGH 4x6	14	14	41/16	53/16	21/8	5/8	8-16d	4.40	67.13	3.14	55.67
										1305	6645	925	5500
愈	ABW46RZ	RGH 4x6	14	16	4	6	213/16	1/2	10-10d	5.81	29.56	4.11	24.47
_										1020	17635	725	13055
	ABA66Z	6x6	14	14	5½	51/4	31/8	5/8	8-16d	4.54	78.45	3.23	58.07
~-										1985	18860	1410	13810
NEW	ABW66Z	6x6	12	14	5½	5%16	3	1/2	12-10d	8.83	83.01	6.27	61.43
	4.01100	0.0	40	40	F4./	_	04 (5.4	10.10.1	3590	24880	2550	18445
	ABU66	6x6	12	10	5½	5	61/16	5/8	12-16d	15.97	110.68	11.34	82.05
	ADAGGD	DOLLO: 0	4.4	4.4	0	F2/	07/	<i>E /</i>	0.40.4	1020	17635	725	13055
	ABA66R	RGH 6x6	14	14	6	5¾6	21/8	5/8	8-16d	4.54	78.45	3.23	58.07
	A DIMICCO Z	DOLL CC	10	4.4		_	213/16	1/	10 101	1780	18660	1265	13810
NEW	ABW66RZ	RGH 6x6	12	14	6	6	219/16	1/2	12-10d	7.92	83.01	5.63	61.43
	A D1 10 04	0,40	10	1.1	71/	7	7	0.5/	10 164	3555	28275	2525	20805
	ABU88 ⁴	8x8	12	14	7½	7	7	2-%	18-16d	15.81	125.78	11.23	92.55
	ABU88R	RGH 8x8	12	14	8	7	7	2-5/8	18-16d	3555	28275	2525	20805
	ADUOON	TIGIT 0X0	12	14	U	_ ′	1	Z-78	10-100	15.81	125.78	11.23	92.55
NEW	ABU1010Z	10x10	12	12	9 1/2	9	7 1/4	2-5/8	22-16d	3055	44950	2170	37025
NEW	70010107	10.10	12	14	3 72	3	1 74	2.78	22 TOU	13.59	199.96	9.65	164.70
NEW	ABU1010RZ	RGH	12	12	10	9	7	2-5/8	22-16d	3055	44950	2170	37025
	ADDIVIONA	10x10	12	12	10	9	,	2.78	22 TOU	13.59	199.96	9.65	164.70

- 1. Uplift resistances have been increased 15% for short term loading, no further increase is allowed
- 2. Factored Normal resistances may not be increased for short term loading.
- Specifier to design concrete for applied loads
- ABU88 and ABU88R may be installed with 8-SDS 1/4"x3" wood screws for same tabulated values.
- 5. Factored resistances shown assume and No.2 for 6x6, 8x8 a
- 6. Factored resistances shown assume a minimum concrete compressive strength of 15 MPa with a concrete surface area of four times the bearing area of the connector. See 10.8.1 CSA A23.3-14.
- 7. Factored resistances shown assume dry service condition (KSF = 1.00). Multiply table values by 0.67 for wet service conditions.
- 8. Structural composite lumber columns have sides that show either the wide face or the edges of the lumber strain syeneers. Values in the tables reflect installation into the wide face. **NA LS:** 16d = 0.162 dia. x $3\frac{1}{2}$ long, 10d = 0.148 dia. x 3 long. See pages 22-23 for other nail sizes and information.

PB/PBS Regular and Standoff Post Bases

The PBS features a 1" standoff height. It reduces the potential for decay at post and column ends.

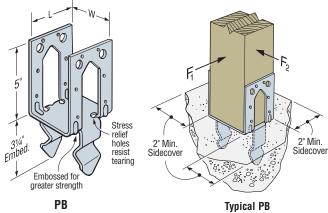
MATERIAL: PB—12 gauge; PBS44A, PBS46—14 gauge strap, 12 gauge standoff; PBS66—12 gauge

FINISH: Galvanized. Some products available in ZMAX® or HDG coating; see Corrosion Information, pages 14-17.

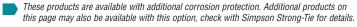
INSTALLATION: • Use all specified fasteners. See General Notes.

- Install either nails or bolts (see page 19, note d).
- Post bases do not provide adequate resistance to prevent members from rotating about the base and therefore are not recommended for non top-supported installations (such as fences or unbraced carports).
- Holes are provided for installation with either 16d commons or 1/2" bolts. A 2" minimum sidecover is required to obtain the full capacity.
- PBS: Embed into wet concrete up to the bottom of the 1" standoff base plate. A 2" minimum side cover is required to obtain the full capacity. Holes in the bottom of the straps allow for free concrete flow.

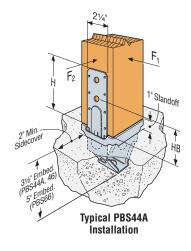
OPTIONS: PBS available in rough sizes, contact Simpson Strong-Tie.



Installation



	Din	nensio	ons	Eac	tener	c		Fact	ored Resist	ance (K _D =	1.15)	
80 - 4 - 1		(in)		газ	rener	٥		D.Fir-L			S-P-F	
Model No.					Во	Its	Uplift	F ₁	F ₂	Uplift	F ₁	F ₂
140.	W	L	Н	Nails	Qty.	Dia.	lbs	lbs	lbs	lbs	lbs	lbs
					uty.	(in)	kN	kN	kN	kN	kN	kN
PB44	3%16	31/4	5	12-16d	2	1/2	1935	1445	1655	1375	1025	1655
FD44	3716	374	5	12-10u		72	8.61	6.43	7.36	6.12	4.56	7.36
PB44R	4	31/4	5	12-16d	2	1/2	1935	1445	1655	1375	1025	1655
F D44N	4	374	J	12-10u		72	8.61	6.43	7.36	6.12	4.56	7.36
PB46	51/2	31/4	5	12-16d	2	1/2	1935	1445	1655	1375	1025	1655
FD40	372	374	5	12-10u		72	8.61	6.43	7.36	6.12	4.56	7.36
PB66	51/2	51/4	5	12-16d	2	1/2	1935	1445	1655	1375	1025	1655
L D 0 0	J 72	J 74	J	12-10U	2	72	8.61	6.43	7.36	6.12	4.56	7.36
PB66R	6	51/4	5	12-16d	2	1/2	1935	1445	1655	1375	1025	1655
FBOOR	0	J 74	J	12-10u	~	72	8.61	6.43	7.36	6.12	4.56	7.36



- 1. Uplift and lateral resistances have been increased 15% for short term load duration. No further increase is allowed.
- 2. Download capacity is the lower of the concrete or post capacity per CSA A23.3-14 or CSA 086-14.
- 3. Structural composite lumer columns have sides that show either the wide face or the edges of the lumber strands/veneers. For SCL columns, the fasteners should be installed through the wide face.
- 4. Factored resistances shown assume dry service condition (KSF = 1.00). Multiply table values by 0.67 for wet service conditions.
- 5. Factored resistances shown assume a minimum of f_c = 15MPa.
- 6. NAILS: 16d = 0.162" dia. x 3½" long. See pages 22-23 for other nail sizes and information.
- These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

	Dii	mensio	ns	Enr	steners					Factored F	lesistance			
		(in)		Газ	steller 2			D.F	ir-L			S-F	P-F	
Model					Во	lte	Uplift	F ₁	F ₂	Normal	Uplift	F ₁	F ₂	Normal
No.	w		н	Nails	DU	112	$(K_D = 1.15)$	$(K_D = 1.15)$	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.15)$	$(K_D = 1.15)$	$(K_D = 1.00)$
	vv		"	Ivalis	Qty.	Dia.	lbs							
					uty.	(in)	kN							
PBS44A	3%16	3½	61/4	14-16d	2	1/2	2745	1650	1345	10920	1950	1170	955	9125
FD344A	3 716	372	074	14-10u	۷	72	12.21	7.34	5.98	48.58	8.67	5.20	4.25	40.59
PBS46	3%16	57/16	6%16	14-16d	2	1/2	2745	1650	1345	15835	1950	1170	955	13155
FD340	3 716	3 716	0716	14-10u	2	72	12.21	7.34	5.98	70.44	8.67	5.20	4.25	58.52
PBS66	5½	5%	6½	14-16d	2	1/2	2745	1650	1345	15835	1950	1170	955	13155
F D 300	J 72	J78	0 72	14-10u		72	12.21	7.34	5.98	70.44	8.67	5.20	4.25	58.52

- 1. Uplift and lateral resistances have been increased 15% for short term load duration. No further increase is allowed.
- 2. Structural composite lumer columns have sides that show either the wide face or the edges of the lumber strands/veneers. For SCL columns, the fasteners should be installed through the wide face.
- 3. Specifier shall design concrete for shear capacity.
- 4. Normal loads (gravity) may not be increased for short term load duration.
- 5. PBS66 factored uplift resistance is 4650 lbs (20.68 kN) D.Fir-L and 3720 lbs (16.55 kN) S-P-F when installed with two $\frac{1}{2}$ " diameter bolts.
- 6. Factored resistances shown assume dry service condition (Ksr = 100). Multiply table values by 0.67 for wet service conditions
- 8. **NAILS:** 16d = 0.162" dia. x 31/2" 22-23 for other nail sizes and information

CPTZ Concealed Post Tie

The new CPTZ post base incorporates a knife plate with a standoff base. It achieves a clean, concealed look while providing a 1" standoff height above concrete. The CPTZ is installed with 1/2" diameter galvanized dowels (supplied). The 1" standoff height is code-required when supporting permanent structures that are exposed to weather or water splash, or in basements. The standoff reduces the potential for decay at post or column ends.

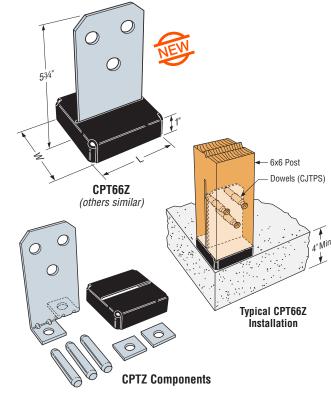
The anchorage for the CPTZ can be either cast-in-place or retrofit with adhesive or mechanical anchors. See our Anchoring and Fastening Systems for Concrete and Masonry catalog for additional information concerning retrofit solutions. The graphic and table below detail a possible cast-in-place

MATERIAL: 10 gauge FINISH: ZMAX® coating

INSTALLATION: • Use all specified fasteners. See General Notes.

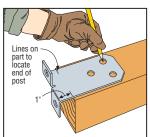
- Use knife blade portion of CPTZ as a template to mark dowel/bolt locations on post
- Drill ½" diameter holes perpendicular to post at marked locations
- Cut a 3/16" wide slot in the end of the post. Cut slot on face adjacent to the one with the holes. If using a circular saw cut the slot roughly 6½" up the post. Test that the knife blade slides freely in the slot.
- Install the knife blade portion of the connector on the anchor bolts and then place the supplied washers over top of the connector's tabs and on the anchor bolt. Use nuts to attach the knife blade and washers to the anchors.
- . The Designer must specify anchor bolt type, length and embedment.
- · Slide the stand off base on to the knife blade assembly
- Stand the post on the knife blade and drive in the dowels supplied with the connector.
- See flier F-CPTZ13 for additional installation information and details.
- Post bases do not provide adequate resistance to prevent members from rotating about the base and therefore are not recommended for non top-supported installations (such as fences or unbraced carports).



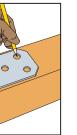


		Dime	noiono	(in)			Fasten	0.40				Factored R	esistanc	е		
		ווווע	ensions	(111.)			rasieii	612		D.	Fir-L			S-	-P-F	
Model	Post				Anc	hor		Post		(K _D =1.15)		Normal ⁷		(K _D =1.15)		Normal ⁷
No.	Size	w		н	Allu	1101		FUSI	Uplift	F ₁	F ₂	$(K_D = 1.00)$	Uplift	F ₁	F ₂	$(K_D = 1.00)$
		VV	_	п	0+1/	Dia.	Otu	Tyno	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs
					Qty.	Dia.	Qty.	Туре	kN	kN	kN	kN	kN	kN	kN	kN
CPT66Z	6x6	53/8	53⁄8	53/4	2	1/2"	4:	4300	1095	1120	30465	3680	995	1120	22695	
CF 100Z	6x6 RGH	3%8	378	3%4		72	3	72 X494 UUWEI	19.13	4.87	4.98	135.52	16.37	4.43	4.98	100.96
CDT007	8x8	71/4	71/4	53/4	2	1/"	3 ½"x4¾" dowel	4600	1235	1800	30465	3680	1120	1800	30465	
CPT88Z	8x8 RGH	1 1/4	1 1/4	5%4	2	1/2"	3	72 X474 UOWEI	20.46	5.49	8.01	135.52	16.37	4.98	8.01	135.52

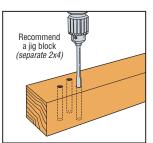
INSTALLATION SEQUENCES



1. Using parts as template

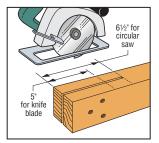


2. Drilling holes

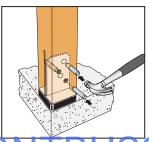




Installing post on CP7Z



3. Cutting slot



- 1. Uplift resistances have been increased 15% for short term loading, no further increase is allowed.
- 2. Factored normal resistances may not be increased for short term loading.
- 3. Factored resistances shown assume No.2 grade minimum.
- 4. Factored resistances shown assume a minimum concrete compressive strength of
- 5. Factored resistances shown assume seasoned lumber under dry service condition (K_{SE}=1.00). Multiply table values by 0.67 for wet service conditions
- 6. ½" diameter ASTM A307 Grade A bolts may be substituted for the 1/2"x43/4" dowels with no reduction in capacity. Standard cut washers are required between the head/nut and the wood.
- 7. Factored normal resistances for installation flush with the corner edge of concrete is 14975 lbs (66.61 kN) for all applications.
- 8. Factored resistances assume 1/2" diameter cast-in-place hex head anchor bolts with 4" embedment, Contact Simpson for post-install solutions.
- 9. When anchoring to a round concrete pier. a minimum 12" diameter is required to achieve tabulated uplift and lateral capacities.



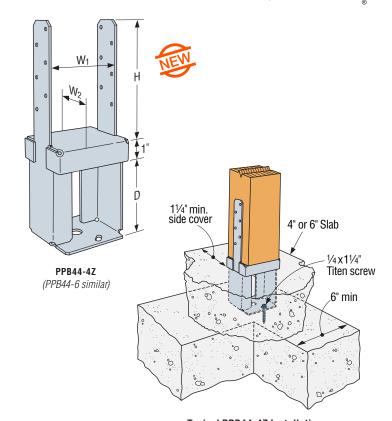


The PPBZ porch post base is designed to be installed once and will support permanent porch framing throughout all stages of construction. This design eliminates the need for temporary support of the porch roof structure and provides full access to installers/inspectors. Install the PPBZ to the cured grade beam with two Simpson Strong-Tie® Titen® screws just prior to rough framing. Designed to withstand vertical construction loads prior to embedment in concrete, the PPBZ will support most framed porches and overhangs. Finally, the concrete contractor is able to complete their last phase of the porch slab without the interference of temporary support. Depending upon the slab thickness, either a 4" or 6" slab of concrete is poured up to the bottom of the 1" standoff.

- Stiffened embedded side stirrups provide temporary vertical download support without being embedded into concrete
- 1" stand-off reduces the potential for decay at post or column ends
- Two available sizes provide both 4" and 6" slab thicknesses
- · Pre-pour installation eliminates temporary support
- · No disruption in scheduling
- · Eliminates additional move-ins by trades and certain inspection call backs

MATERIAL: 12 gauge FINISH: ZMAX® coating INSTALLATION:

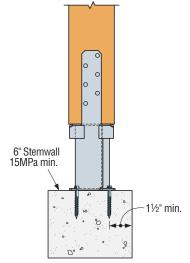
- Use all specified fasteners. See General Notes.
- Titen® screws (Model No: TTN25114H) are not provided with the base
- See page 32 for Titen® screw information and installation instructions
- Minimum 11/4" side cover on concrete is required
- Post bases do not provide adequate resistance to prevent members from rotating about the base, and are not recommended for non top-supported installations (i.e. fences or carports).



Typical PPB44-4Z Installation

)imanais	(i		Factor			Factored F	Resistance	
	Nominal		Dimensio	ons (in.)	Faster	iers	Prior t	o Pour	Embedded i	nto Concrete
Model No.	Model Post			_		Faundation	Beet	Uplift (K _D = 1.15)	Normal (K _D = 1.00)	Uplift (K _D = 1.15)	Normal (K _D = 1.00)
		W ₁	W ₂	D	Н	Foundation	Post	lbs	lbs	lbs	lbs
								kN	kN	kN	kN
PPB44-4Z	4x4	35/8	35/16	4	53/4	2-1/4x11/4 Titen	12-10dx1½	265	6610	1540	10970
PPD44-4Z	4 4 4	378	3716	4	374	2-74X 174 TILEII	12-100X172	1.18	29.40	6.85	48.80
DDD44 C7	454	A.A. 05/ 05/ 0 59/ 0 1/v41/ Tiber	2-1/4x11/4 Titen	12-10dx1½	265	6020	1765	14715			
PPB44-6Z	4x4	35/8	35/16	6	53⁄4	2-74X 174 TILETI	12-100X1½	1.18	26.78	7.85	65.46

- 1. Factored uplift resistances for Embeded into Concrete installation have been increased 15% for earthquake or wind loading with no further increase allowed; reduce where other loads govern. 2. Tablulated factored resistances apply to both D.Fir-L and S-P-F posts.
- 3. The minimum 28-day concrete compressive strength (f'_{C}) shall be 2200 psi (15 MPa) for Prior to Pour, and 4650 psi (32 MPa) for Embeded into Concrete.
- . Structural composite lumber columns have sides that show either the wide face or the edges of the lumber strands/veneers. Uplift values in the table reflect installation fastener installation into the wide face.
- 5. Designer is responsible for concrete and column design.
- 6. Factored resistances shown assume dry service conditions ($K_{SF} = 1.00$). Multiply uplift values by 0.67 for wet service conditions.
- 7. For slab thickness not shown between 4"-6", use PPB44X and specify slab thickness (D). Factored resistances may be interpolated.
- 8. NAILS: 10dx1½ = 0.148" x 1½" long. Use hot-dip galvanized nails. See pages 22-23 for other nail sizes and information.



EPB Elevated Post Bases

The EPB44A is a single-piece, non-welded elevated post base. The EPB44PHDG can be used both for pier block and cast-in-place installations for 4x4 posts.

MATERIAL: EPB44A—14 gauge; EPB44, EPB46, EPB66—12 gauge base plate, 11/16" OD x 8" pipe; EPB44PHDG-12 gauge base plate, 3/4"x6" threaded rod support (nut and washer are shipped assembled)

FINISH: EPB44A—Galvanized; EPB44, EPB46, EPB66— Simpson Strong-Tie® gray paint (may be ordered HDG); EPB44PHDG—Hot-dip galvanized, see Corrosion Information, pages 14-17.

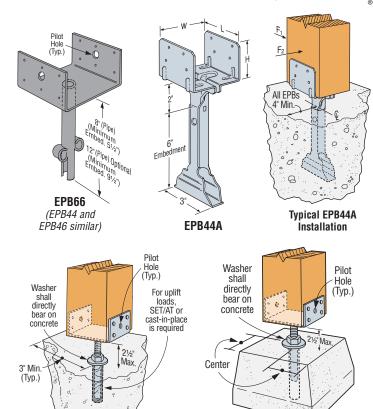
INSTALLATION: • Use all specified fasteners. See General Notes.

- Allows 1" to 21/2" clearance above concrete, 2" for EPB44A. Insert EPB into concrete after screeding.
- · Post bases do not provide adequate resistance to prevent members from rotating about the base and therefore are not recommended for non top-supported installations (such as fences or unbraced carports).
- Provide a minimum side cover of 3" for EPB44PHDG and 4" for all others.

EPB44PHDG

- Secured with Epoxy—Drill a 7/8" diameter hole 4" deep minimum and fill the hole halfway with SET epoxy or drill a 13/16" diameter hole 4" deep minimum and fill the hole halfway with AT adhesive. Insert the EPB44PHDG and adjust to the desired height. The threaded rod shall be embedded a minimum of 31/2".
- Supported by a Nut—Drill a 1" diameter hole 3½" deep minimum. Insert the EPB44PHDG and adjust to the desired height.
- Embedded in Wet Concrete—Embed the ¾" diameter rod a minimum of 3½". Ensure nut and washer are flush with the top of the concrete.
- · Fully engage at least three threads in the base.

OPTIONS: 12" long pipe available for EPB44, EPB46, EPB66; specify "-12" after model number.



Typical EPB44PHDG Installed with SET Epoxy or AT Adhesive



These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details. These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

	Di	imensio	ns					Factored F	Resistance			
		(in)				D.F	ir-L			S-I	P-F	
Model				Nails	Uplift	F ₁	F ₂	Down	Uplift	F ₁	F ₂	Down
No.	w		Н	Nans	$(K_D = 1.15)$	$(K_D = 1.15)$	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.15)$	$(K_D = 1.15)$	$(K_D = 1.00)$
	VV	-	"		lbs							
					kN							
EPB44PHDG	29/	21/.	21/4	8-16d	1045³		_	5660	1045³	_	_	5660
LFD44FIIDG	31/4	Z /4	0-10u	4.65	_	_	25.18	4.65	_	_	25.18	
EPB44A	3%16	3	2%	8-16d	1965	1340	1530	4370	1395	950	1085	3640
LF D44A	3716	כ	2 78	0-10u	8.74	5.96	6.81	19.44	6.21	4.23	4.83	16.19
EPB44	3%16	31/4	2 5⁄16	8-16d	1270	1945	1700	8465	900	1380	1205	6995
LI D44	3 716	J /4	2716	0-10u	5.65	8.65	7.56	37.66	4.00	6.14	5.36	31.12
EDD/16	5½	35/16	3	12-16d	1270	1390	1635	8465	900	990	1160	6980
EPB46	J /2	J / Ib	0	12-100	5.65	6.18	7.27	37.66	4.00	4.40	5.16	31.05
EDB66	5½	5½	3	12-16d	1570	1390	1635	8465	1115	990	1160	6225
EPB66	J /2	J /2		12 100	6.98	6.18	7.27	37.66	4.96	4.40	5.16	27.69

- 1. Uplift and lateral resistances have been increased 15% for short term loading. No further increase is allowed.
- 2. EPB44 and EPB46 have extra nail holes; only eight must be filled to achieve the resistances shown. 3. Uplift resistances for EPB44PHDG require the threaded rod to be set in wet concrete or attached to cured
- concrete with SET epoxy or AT adhesive. Uplift values do not apply to connection with pier block.
- 4. Specifier shall design concrete for applied loads
- 5. Factored resistances shown assume dry service condition (KSF = 1.00). Multiply table values by 0.67 under wet service conditions.
- 6. Structural composite lumber columns have sides that show either the wide face or the edges of the lumber strands/veneers. Values in the tables reflect installation into the wide face.
- 7. Minimum f'c shall be 15MPa
- 8. **NAILS:** 16d = 0.162" dia x

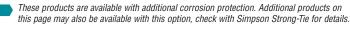
EPS4Z Column Bases

The EPS4Z provides a light-duty connector for attachment of posts to concrete.

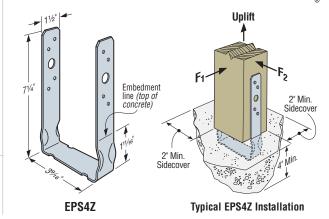
MATERIAL: 14 gauge FINISH: ZMAX® coating; see Corrosion Information, pages 14-17.

INSTALLATION: • Use all specified fasteners. See General Notes.

- Post bases do not provide adequate resistance to prevent members from rotating about the base and therefore are not recommended for non top-supported installations (such as fences or unbraced carports).
- Embed into wet concrete up to the embedment line. A 2" minimum side cover is required to obtain the full load.
- Posts shall be preservative-treated wood to meet building code requirements.



			Factor	ed Resist	ance (K _D	= 1.15)	
			D.Fir-L			S-P-F	
Model No.	Fasteners	Uplift	F ₁	F ₂	Uplift	F ₁	F ₂
		lbs	lbs	lbs	lbs	lbs	lbs
		kN	kN	kN	kN	kN	kN
EDC 47	8-10dx1½	1765	965	815	1255	685	580
EPS4Z	0-10UX172	7.85	4.29	3.63	5.58	3.05	2.58



Strong-Tie

- Uplift and lateral resistances have been increased 15% for short term loading. No further increase is allowed.
- Download capacity is the lower of the concrete or post capacity per CSA A23.3-14 or CSA 086-14.
- Structural composite lumer columns have sides that show either the wide face or the edges of the lumber strands/veneers. For SCL columns, the fasteners should be installed through the wide face.
- Factored resistances shown assume dry service condition (K_{SF} = 1.00). Multiply table values by 0.67 under wet service conditions.
- 5. NAILS: 10dx1½ = 0.148" dia. x 1½" long. See pages 22-23 for other nail sizes and information.

RCPS Rebar Carport Saddles

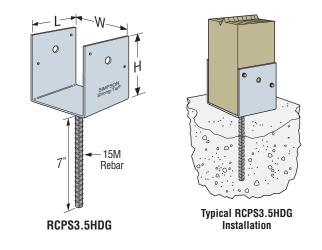
Rebar saddle bracket for connecting post to concrete.

MATERIAL: 13 gauge FINISH: Hot-dip galvanized, use HDG fasteners INSTALLATION: • Use all specified fasteners. See General Notes.

 Post bases do not provide adequate resistance to prevent members from rotating about the base and therefore are not recommended for non top-supported installations (such as fences or unbraced carports).

Model)imensions (in	1)	Fasteners
No.	W	L	Н	rastellers
RCPS3.5HDG	35%	31/4	3½	4-16d
RCPS4HDG	41/8	4	3½	4-16d
RCPS46HDG	41/8	6	3½	4-16d
RCPS4.5HDG	45/8	3½	3½	4-16d
RCPS5.5HDG	5%	5½	3½	4-16d
RCPS6HDG	61//8	6	3½	4-16d
RCPS7.5HDG	75%	73/8	3½	4-16d
RCPS8HDG	81//8	8	3½	4-16d

1. NAILS: 16d = 0.162" dia. x 3½" long. See pages 22-23 for other nail sizes and information.



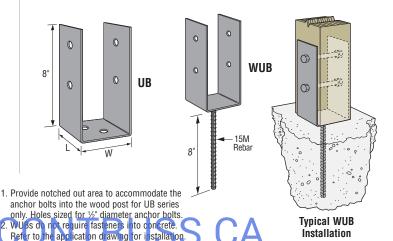
UB/WUB Post Brackets

Saddle bracket for connecting post to concrete.

MATERIAL: 3 gauge FINISH: Hot-dip galvanized, use HDG fasteners INSTALLATION: • Use all specified fasteners. See General Notes.

 Post bases do not provide adequate resistance to prevent members from rotating about the base and therefore are not recommended for non top-supported installations (such as fences or unbraced carports).

Model	Post	Dimens	ions (in)	Fasteners
No.	Size	W	L	Post
UB44HDG	4x4	35/8	3	2-1/2" MB
UB44RHDG	4x4R	41//8	3	2-1/2" MB
UB66HDG	6x6	5%	4	2-1/2" MB
UB66RHDG	6x6R	61//8	4	2-1/2" MB
WUB44HDG	4x4	35/8	3	2-1/2" MB
WUB44RHDG	4x4R	41/8	3	2-1/2" MB
WUB66HDG	6x6	5%	3	2-1/2" MB
WUB66RHDG	6x6R	61/4	V \3	2-½" MB



Catalogue C-C-CAN2015 © 2015 SIMPSON STRONG-TIE COMPANY INC

LCB/CB Column Bases



LCB—Low-cost column base for patios, carports, breezeways and porches. CB—For columns that require high structural values and rugged performance. MATERIAL: See table

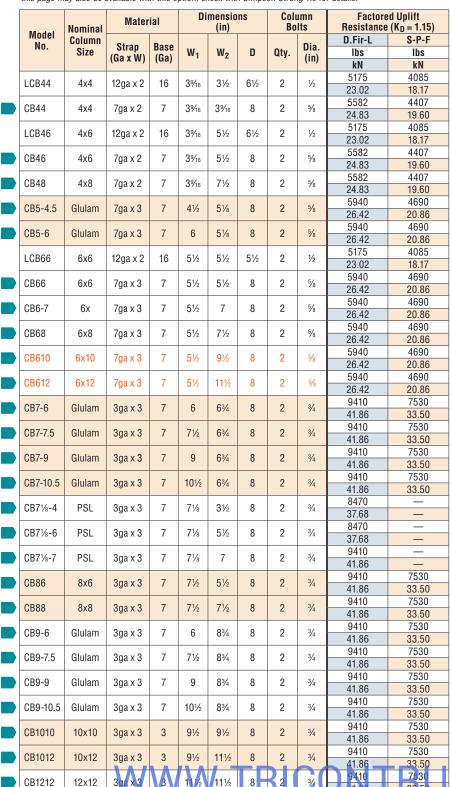
FINISH: LCB, CB44, CB46, CB48, CB66, CB68, CB86—galvanized; all other CB (including all CBGT)—Simpson Strong-Tie® gray paint or HDG. Some models available in stainless steel.

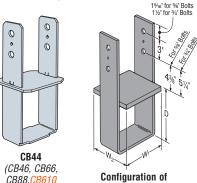
INSTALLATION: • Use all specified fasteners. See General Notes

- For full loads, minimum side cover required is 3" for CB, 2" for LCB.
- Install all models with bottom of base plate flush with concrete.
- These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.
- · Post bases do not provide adequate resistance to prevent members from rotating about the base and therefore are not recommended for non top-supported installations (such as fences or unbraced carports).
- Contact engineered wood manufacturers for connections that are not through the wide face.

OPTIONS: • The LCB may be shipped unassembled; specify "Disassembled".

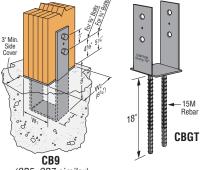
- LCB and CB are available in rough size. Other sizes available for CB specify W₁ and W₂ dimensions. Consult Simpson Strong-Tie for bolt sizes and factored resistances. See PBS.
- For rebar option add "GT" to the model name, i.e., CBGT44. (Base plate comes 3 ga for all CBGTs)



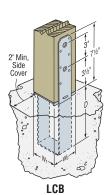


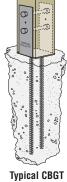
CB88.CB610 similar)

all other CB sizes



(CB5, CB7 similar) for Glulam Column





Installed

- 1. Factored uplift resistances have been increased 15% for earthquake or wind loading, with no further increase allowed: reduce where other loads govern.
- 2 PSI is parallel strand lumber

33.50

- Factored resistances shown assume dry service condition (K_{SF} = 1.00). Multiply table values by 0.67 for uplift under wet service conditions
- 4. Factored uplift resistance for CBGT option is 4350 lbs (19 35 kN)
- 5. LCB products must be installed with bolts to achieve table values.
- Designer is responsible for concrete design.
- Structural composite lumber columns have sides that show either the wide face or the edges of the lumber strands/veneers. Values in the tables reflect installation into the wide face.

8. 1_c snal be 29 MPa. Vinimum. 9. NAILS: 16d = 0.162 dia. x 3½ long. See pages 22-23 for other nail sizes and information.

CBSQ Column Bases



THE ERED

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

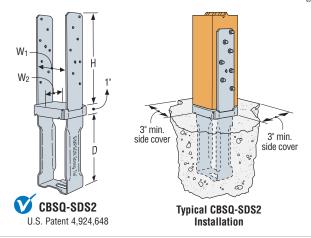
The CBSQ uses Simpson Strong-Tie® Strong-Drive® SDS Heavy-Duty Connector screws, which allows for fast installation, reduced reveal, high capacity and provides a greater net section area of the column compared to bolts.

MATERIAL: See table

FINISH: Galvanized, available in HDG

INSTALLATION: • Use all specified fasteners. See General Notes.

- Install ¼"x2" Strong-Drive SDS Heavy-Duty Connector screws, which are provided with the column base. (Lag screws will not achieve the same load.)
- Minimum 3" side cover on concrete is required.
- Post bases do not provide adequate resistance to prevent members from rotating about the base and therefore are not recommended for non top-supported installations (such as fences or unbraced carports).



These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

		I.	(laterial		Dime	sions				Factored F	Resistance	
	Naminal	IV	iaiciiai		(i	n)		Number of	D.F	ir-L	S-F	P-F
Model No.	Nominal Column Size	Base	Strap	W ₁	W ₂	D	н	Simpson Strong-Tie SDS 1/4"x2"	Uplift (K _D = 1.15)	Normal (K _D = 1.00)	Uplift (K _D = 1.15)	Normal (K _D = 1.00)
		(Ga)	(Ga x W)	VVŢ	W Z	יי	"	Screws	lbs	lbs	lbs	lbs
									kN	kN	kN	kN
CBSQ44-SDS2	4x4	12	10 Ga x 21/4	3%16	3½	71/16	8%	14	7310	16195	5265	11660
003044-3032	484	12	10 Ga X 274	74 3716 372 1716 078 I	14	32.56	72.14	23.45	51.94			
CBSQ46-SDS2	4x6	12	10 Ga x 3	3%16	55/16	73/4	811/16	14	7310	21280	5265	15320
003040-3032	480	12	10 da x 3	J /16	J 716	1 74	0 /16	14	32.56	94.79	23.45	68.24
CBSQ66-SDS2	6x6	12	10 Ga x 3	5½	5½	67/8	83/4	14	7310	21280	5265	15320
0B3Q00-3D32	0.00	12	10 da x 3	J /2	J /2	0 /8	0 74	14	32.56	94.79	23.45	68.24
CBSQ86-SDS2	6x8	12	7 Ga x 3	71/2	5%	61/8	811/16	12	6220	25140	4475	18100
0D3Q00-3D32	0.00	12	7 Ga X S	1 /2	J 78	0 /8	0 /16	12	27.71	111.98	19.93	80.62
CBSQ88-SDS2	8x8	12	7 Ga x 3	7½	73/8	61/8	811/16	12	6730	26545	4845	19115
CBSQ88-SDS2	0.00	12	1 Ud X S	1 72	1 78	078	0 716	12	29.98	118.24	21.58	85.14

- For higher factored normal resistances, solidly pack grout under 1" standoff plate before installing CBSQ into concrete.
 Base factored normal resistances on column or concrete, according to the code.
- Structural composite lumber columns have sides that show either the wide face or the edges of the lumber strands/veneers. Values in the tables reflect installation into the wide face.
- 3. Designer is responsible for
- concrete and column design.

 4. Factored resistances shown assume dry service condition (K_{SF} = 1.00). Multiply table values by 0.67 under wet service conditions.
- 5. Minimum f'c shall be 20 MPa.

CBQGT Column Bases

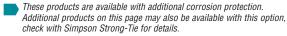
The CBQGT uses Simpson Strong-Tie® Strong-Drive® SDS Heavy-Duty Connector screws, which allows for fast installation, reduced reveal, high capacity and provides a greater net section area of the column compared to bolts.

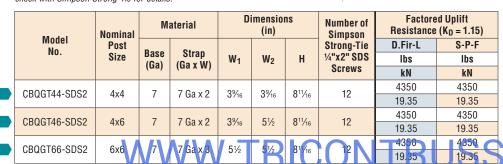
MATERIAL: See table

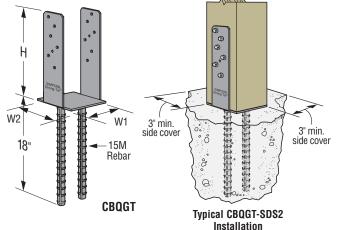
FINISH: Simpson Strong-Tie® gray paint, available in HDG

INSTALLATION: • Use all specified fasteners. See General Notes.

- Install ¼"x2" Strong-Drive SDS Heavy-Duty Connector screws, which are provided with the column base. (Lag screws will not achieve the same load.)
- Minimum 3" side cover on concrete is required.
- Post bases do not provide adequate resistance to prevent members from rotating about the base and therefore are not recommended for non top-supported installations (such as fences or unbraced carports).
- · Other sizes available. Check with Simpson Strong-Tie for details.







- Factored uplift resistances have been increased 15% for earthquake or wind loading, with no further increase allowed; reduce where other loads govern.
- Structural composite lumber columns have sides that show either the wide face or the edges of the lumber strands/ veneers. Values in the tables reflect installation into the wide face.
- Designer is responsible for concrete design.
 Factored resistances shown assume dry service condition (K_{SF} = 1.00). Multiply table values by 0.67 under wet service conditions.

shall be 20 MPa.

BC/HBC/BCS Post Caps and Bases



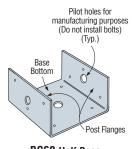
The BCS allows for the connection of 2-2x's to a 4x post or 3-2x's to a 6x post. Double shear nailing between beam and post gives added strength! The BC/HBC series offers dual purpose post cap/base for light cap or base connections.

MATERIAL: HBC-12 gauge; all others-18 gauge

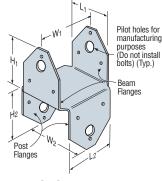
FINISH: HBC—HDG; all others—galvanized. Some products available in ZMAX® coating; see Corrosion Information, pages 14-17.

INSTALLATION:

- Use all specified fasteners. See General Notes.
- Do not install bolts into pilot holes.
- BCS—install dome nails on beam; drive nails at an angle through the beam into the post below to achieve the table loads
- BC-install with 16d commons or 16dx21/2" nails.
- Post bases do not provide adequate resistance to prevent members from rotating about the base and therefore are not recommended for non top-supported installations (such as fences or unbraced carports).



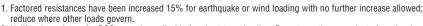
BC60 Half Base (others similar)



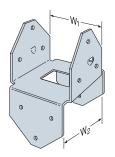
BC4 Cap/Base (BC46, BC6 similar)

- These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.
- These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

			Dime		3			Fastener	9	Factored Resistance (K _D = 1.15)						
Model		,	(i	n)							ir-L	•	P-F			
No.									Base	Uplift	Lateral	Uplift	Lateral			
	W ₁	W ₂	L ₁	L ₂	H ₁	H ₂	Beam	Post	Bottom	lbs	lbs	lbs	lbs			
									Dottom	kN	kN	kN	kN			
								CAPS								
BC4	3%16	3%16	27/8	27/8	3	3	6-16d	6-16d		875	1495	620	1060			
D04	3716	3 716	2/8	2/8	3	3	0-100	0-100		3.90	6.66	2.76	4.72			
BC46	3%16	5½	47/8	27/8	3½	2½	12-16d	6-16d	_	1415	1495	1005	1060			
5040	0710	072	770	270	072	L /2	12 100	0 100		6.30	6.66	4.48	4.72			
BC4R	4	4	4	4	3	3	12-16d	12-16d	_	875	1495	620	1060			
		·		·	Ŭ	Ŭ	12 100	12 100		3.90	6.66	2.76	4.72			
BC6	5½	5½	43/8	43/8	3%	3%	12-16d	12-16d	_	1450	3145	1030	2765			
200	072	072	170	170	0,0	0,0	12 100	12 100		6.46	14.01	4.59	12.32			
BC6R	6	6	6	6	3	3	12-16d	12-16d		1560	3145	1110	2765			
Boon	Ů	Ů	Ů	Ů	Ŭ	Ŭ	12 100	12 100		6.94	14.01	4.94	12.32			
BC8	7½	7½	7½	7½	4	4	12-16d	12-16d		2545	3145	1810	2765			
200	. / -	. / 2	. / 2	. / 2			.2 .00	.2 .00		11.34	14.01	8.06	12.32			
BCS2-2/4	31/8	3%16	27/8	27/8	215/16	215/16	8-10d	6-10d		1190	1560	845	1370			
										5.30	6.95	3.76	6.09			
BCS2-3/6	45/8	5%16	4 3/8	27/8	35/16	215/16	12-16d	6-16d	_	1370	2445	970	1735			
										6.10	10.89	4.32	7.73			
								BASES								
BC40	3%16	_	31/4	_	21/4	_	6-16d	_	4-16d	510	1050	360	960			
										2.27	4.68	1.60	4.28			
HBC40HDG	3%16	_	31/4	_	21/4	_	6-16d	_	4-16d	510	1050	360	960			
	•									2.27	4.68	1.60	4.28			
BC40R	4	_	4	_	3	_	6-16d	_	4-16d	510	1050	360	960			
							0 .00			2.27	4.68	1.60	4.28			
BC460	5½	_	3%	_	3	_	6-16d	_	4-16d	510	1050	360	960			
										2.27	4.68	1.60	4.28			
BC60	5½	_	5½	_	3	_	6-16d	_	4-16d	510	1050	360	960			
										2.27	4.68	1.60	4.28			
HBC60HDG	5½	_	5½	_	3	_	6-16d	_	4-16d	510	1050	360	960			
										2.27	4.68	1.60	4.28			
BC60R	6	_	6	_	3	_	6-16d	_	4-16d	510	1050	360	960			
										2.27	4.68	1.60	4.28			
BC80	7½	_	7½	_	4	_	6-16d	_	4-16d	510	1050	360	960			
		. / 2		1/2	<u> </u>								2.27	4.68	1.60	4.28
BC80R	8	_	8	_	4	_	6-16d	_	4-16d	510	1050	360	960			
			-		1					2.27	4.68	1.60	4.28			

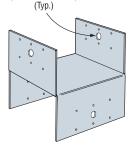


- 2. Uplift values shown for BCO are only applicable for short-term loading. Do not use these products for other load durations.
- 3. Uplift resistances do not apply where Bases are nailed into the end grain of post as per 12.9.3.4 CSA 086-14.
- 4. Structural composite lumber columns have sides that show either the wide face or the edges of the lumber strands/veneers. Values in the tables reflect installation into the wide face (see technical bulletin T-SCLCOLUMNCAN for details).
- 5. Factored resistances shown assume dry service condition (K_{SF} = 1,00). Multiply table values by 0.67 under wet service conditions.
- 6. NAILS: 16d = 0.162" dia x 3 ½ long 107 = 0.748 dia x 3 long. See lages 22-23 for other nail's zes and information

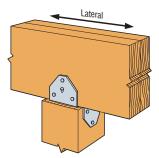


BCS2-2/4 U.S. Patent 5,603,580

Pilot holes for manufacturing purposes (Do not install bolts)



BC8 Cap/Base (BC4R, BC6R similar)



Typical BCS Installation

AC/ACE/LPCZ/LCE Post Caps

The LCE4's universal design provides high capacity while eliminating the need for

rights and lefts. For use with 4x or 6x lumber. The AC MAX design allows for higher load capacity to match comparable post bases. LPCZ—Adjustable design allows greater connection versatility.

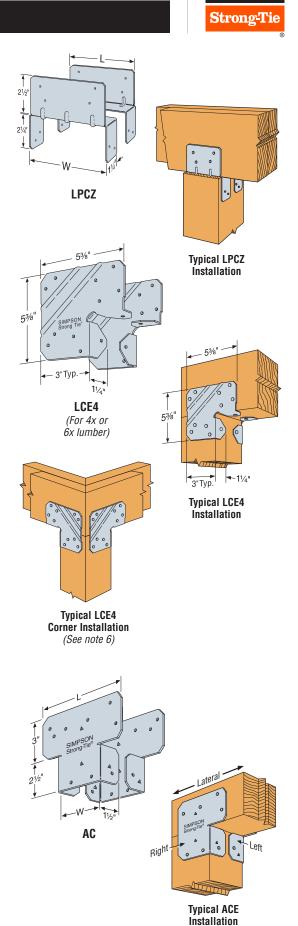
MATERIAL: LCE4—20 gauge; AC, ACE, LPC4Z—18 gauge; LPC6Z—16 gauge FINISH: Galvanized. Some products available in ZMAX® coating and stainless steel; see Corrosion Information, pages 14-17.

INSTALLATION: • Use all specified fasteners. See General Notes.

- Install all models in pairs.
- These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.
- These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

	Dimer	nsions	East	eners	Factored Resistance (K _D = 1.15)					
84 - 4 - 1	(i	n)	Газіс	SIICI S	D.F	ir-L	S-I	P-F		
Model No.		L			Uplift	Lateral	Uplift	Lateral		
140.	W		Beam	Post	lbs	lbs	lbs	lbs		
					kN	kN	kN	kN		
L DC 47	3%16	3½	8-10d	8-10d	1225	460	870	325		
LPC4Z	3716	372	0-10u	0-100	5.46	2.05	3.88	1.45		
LCE4		5¾	14-16d	10-16d	2560	2300	2315	1910		
LUL4		J /8	14-10u	10-100	11.40	10.24	10.31	8.51		
AC4 (Min)	3%16	6½	8-16d	8-16d	2095	2290	1920	1625		
A04 (WIIII)	3 716	072	0-10u	0-10u	9.33	10.20	8.55	7.24		
AC4 (Max)	3%16	6½	14-16d	14-16d	3670	2850	3360	2025		
AU4 (IVIAX)	3 716	072	14-100	14-100	16.35	12.69	14.97	9.02		
ACE4 (Min)		41/2	6-16d	6-16d	1570	695	1220	495		
AUL4 (WIIII)		4/2	0-10u	0-10u	6.99	3.10	5.43	2.20		
ACE4 (Max)	_	41/2	10-16d	10-16d	2225	900	1580	640		
AUL4 (IVIAX)		4/2	10-100	10-100	9.91	4.01	7.04	2.85		
AC4RZ (Min)	4	7	8-16d	8-16d	2095	2290	1920	1625		
AO4112 (WIII)	4			3 100	9.33	10.20	8.55	7.24		
AC4RZ (Max)	4	7	14-16d	14-16d	3670	2850	3360	2025		
704112 (Wax)	7	,	14 100	14 100	16.35	12.69	14.97	9.02		
LPC6Z	5%16	5½	8-10d	8-10d	1040	695	735	495		
LI 002	3716	372	0 100	0 100	4.63	3.10	3.27	2.20		
AC6 (Min)	5½	81/2	8-16d	8-16d	2095	1925	1855	1365		
AGG (WIIII)	J 72	072	0 100	0 100	9.33	8.57	8.26	6.08		
AC6 (Max)	5½	8½	14-16d	14-16d	3670	3670	3030	2845		
AGG (WIAX)	J 72	072	14 100	14 100	16.35	16.35	13.50	12.67		
ACE6 (Min)	_	6½	6-16d	6-16d	1570	1300	1440	1070		
AOLO (WIIII)		072	0 100	0 100	6.99	5.79	6.41	4.77		
ACE6 (Max)	_	6½	10-16d	10-16d	2620	2075	2400	1800		
/ TOLO (IVIAX)		072	10 100	10 100	11.67	9.24	10.69	8.02		
AC6RZ (Min)	6	9	8-16d	8-16d	2095	1925	1920	1365		
AUDITZ (WIIII)	U	9	0 100	0 100	9.33	8.57	8.55	6.08		
AC6RZ (Max)	6	9	14-16d	14-16d	3670	3670	3360	2845		
ACORZ (IVIAX)	J	9	17 100	17 100	16.35	16.35	14.97	12.67		

- 1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed; reduce where other loads govern.
- 2. Resistances apply only when used in pairs.
- 3. LPCZ lateral resistance is in the direction parallel to the beam.
- 4. MIN nailing quantity and resistances fill all round holes; MAX nailing quantities and resistances - fill round and triangle holes.
- 5. Uplift values do not apply to splice conditions.
- 6. LCE4 uplift capacity for mitered corner conditions is 1615 lbs (7.18 kN) D.Fir-L and 1145 lbs (5.09 kN) S-P-F. Lateral resistances do not apply
- 7. Structural composite lumber columns have sides that show either the wide face or the edges of the lumber strands/veneers. Values in the tables reflect installation into the wide
- face. See technical bulletin T-SCLCOLUMNCAN for values on the narrow face (edge). 8. **NAILS:** 16d = 0.162" dia. x $3\frac{1}{2}$ " long 10d



SIMPSON

PCZ/EPCZ Post Caps

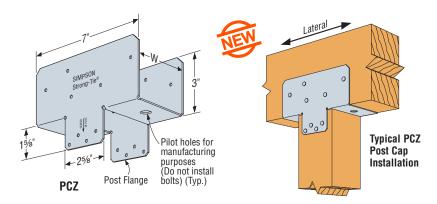
The next-generation PCZ/EPCZ post caps are designed with their post and beam flanges in-line so that one PCZ/EPCZ model can accommodate several post sizes. The PCZ/EPCZ now uses easierto-install 10d common nails. An alternate choice of fasteners is #9x11/2" Strong-Drive® SD Connector screws. ZMAX® finish is standard to meet exposure conditions in many environments. See additional corrosion information at www.strongtie.com/info.

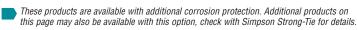
MATERIAL: 16 gauge FINISH: ZMAX® coating INSTALLATION:

- · Use all specified fasteners; see General Notes.
- · Do not install bolts into pilot holes.

OPTIONS:

- For end conditions, specify EPCZ post caps.
- · For heavy-duty applications, see CCQ and CC Series.

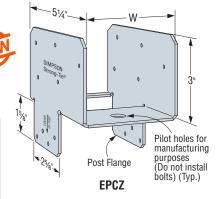




These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

		F1		Factored Resistance (K _D = 1.15)							
	w	Faste	eners	D.F	ir-L	S-P-F					
Model	(in)			Uplift	Lateral	Uplift	Lateral				
		Beam	Post	lbs	lbs	lbs	lbs				
				kN	kN	kN	kN				
PC4Z	3%16	10-10d	8-10d	1920	1920	1785	1515				
PU4Z	3716	10-100	0-10u	8.54	8.54	7.94	6.74				
EPC4Z	29/	3% ₆ 10-10d	8-10d	1920	1795	1450	1315				
LF U4Z	3716		0-10u	8.54	7.98	6.45	5.85				
PC4RZ	4	10-10d	8-10d	1920	1920	1785	1515				
PU4RZ	4	10-100	0-100	8.54	8.54	7.94	6.74				
EPC4RZ	4	10-10d	8-10d	1920	1795	1450	1315				
EPU4NZ	4	10-100	0-10u	8.54	7.98	6.45	5.85				
PC6Z	E1/	½ 10-10d	8-10d	1920	1920	1785	1515				
PU02	372		0-10u	8.54	8.54	7.94	6.74				
EPC6Z	51/2		0.404	1920	1795	1485	1315				
EPU0Z	372	10-100	8-10d	8.54	7.98	6.61	5.85				
PC6RZ	6	10-10d	8-10d	1920	1920	1785	1515				
PUUNZ	0	10-100	0-10u	8.54	8.54	7.94	6.74				
EPC6RZ	6	10-10d	8-10d	1920	1795	1485	1315				
EFUUNZ	0	10-100	0-10u	8.54	7.98	6.61	5.85				
PC8Z	71/2	10-10d	8-10d	1920	1920	1785	1515				
PUOZ	/ /2	10-100	0-10u	8.54	8.54	7.94	6.74				
EPC8Z	71/2	10-10d	8-10d	1920	1795	1485	1315				
EPUOL	/ 1/2	10-100	0-100	8.54	7.98	6.61	5.85				
PC8RZ	8	10-10d	8-10d	1920	1920	1785	1515				
PUORZ	0	10-100	0-100	8.54	8.54	7.94	6.74				
EPC8RZ	8	10-10d	8-10d	1920	1795	1485	1315				
EFUORZ	0	10-100	0-100	8.54	7.98	6.61	5.85				

- 1. Factored resistances have been increased 15% for earthquake or wind loading. No further increase is permitted. Reduce where other load durations govern.
- 2. Factored uplift resistances do not apply to beams spliced over the column.
- 3. Spliced conditions must be detailed by the Designer to transfer tension loads between spliced members by means other than the post cap.
- 4. Structrual composite lumber columns have sides that show either the wide face or the edges of the lumber strands/veneers. Values in the table are only appicable to installation into the wide face of the post. 5. 10d nails may be substituted with Strong-Drive® SD#9x1½* Connector screws for full capacities shown.
- 6. Multiply the tablulated values x 0.67 for wet service conditions.
- 7. NAILS: 10d = 0.148" dia. x 3" long. See pages 22-23 for other nail sizes and information. **SCREWS:** SD#9x1½" = 0.131" dia. x 1½" long (SD9112)



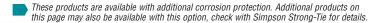


CCQ Column Caps

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

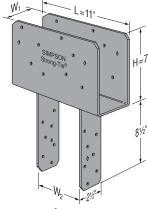
This design uses Simpson Strong-Tie® Strong-Drive® SDS Heavy-Duty Connector screws to provide faster installation. The Strong-Drive SDS Heavy-Duty Connector screws provide for a lower profile compared to standard through bolts. MATERIAL: CCQ3, CCQ4, CCQ4.62, CCQ6—7 gauge; all others—3 gauge. FINISH: Simpson Strong-Tie® gray paint, available in HDG; CCOQ—uncoated INSTALLATION: Fasteners provided. See General Notes.

- Install 1/4"x21/2" Strong-Drive SDS Heavy-Duty Connector screws, which are provided with the column cap. (Lag screws will not achieve the same load.) **OPTIONS:** Straps may be rotated 90° where $W_1 \ge W_2$.
 - CCOQ—may be ordered for field welding to pipe or other columns (no loads apply).
 - Custom sizes are available. Contact Simpson Strong-Tie for more information.
 - See page 73 for CCCQ and CCTQ options.

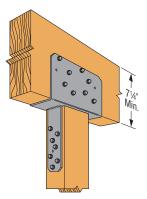


	Dime		No.	. of	Factored Resistance							
Model	Dimensions (in)		SDS 1/4	"x2½"	Uplift ($K_D = 1.15$) Normal ($K_D = 1.00$)							
Model No.			Scr	ews	D.Fir-L	S-P-F	D.Fir-L	S-P-F	SCL			
NU.			_		lbs	lbs	lbs	lbs	lbs			
	W ₁	W ₂	Beam	Post	kN	kN	kN	kN	kN			
0000 40000 5	01/	05/	10	4.4	9500	7705	23075	18955	_			
CCQ3-4SDS2.5	31/4	3%	16	14	42.26	34.27	102.65	84.32	_			
CCQ3-6SDS2.5	31/4	5½	16	14	9500	8855	27915	22545	_			
00Q3-03D32.3	374	J 72	10	14	42.26	39.39	124.18	100.29	_			
CCQ44SDS2.5	35/8	35/8	16	14	9500	7705	25845	21230	39100			
	-	0,0			42.26	34.27	114.97	94.44	173.93			
CCQ46SDS2.5	35/8	5½	16	14	9500 42.26	8855 39.39	31260	23660 105.25	42040			
					9500	8855	139.06 31260	23660	187.01 42040			
CCQ48SDS2.5	35/8	7½	16	14	42.26	39.39	139.06	105.25	187.01			
	457	251			9500	7705	33230	27295				
CCQ4.62-3.62SDS2.5	45/8	3%	16	14	42.26	34.27	147.82	121.42	_			
CCQ4.62-4.62SDS2.5	45%	45/8	16	14	9500	7705	40195	30420	_			
UUU4.02-4.023D32.3	478	478	10	14	42.26	34.27	178.80	135.32	_			
CCQ4.62-5.5SDS2.5	45/8	5½	16	14	9500	8855	40195	30420	_			
	170	072	10		42.26	39.39	178.80	135.32	_			
CCQ5-4SDS2.5	51/4	35/8	16	14	10700 47.60	7705	37845	31090				
	-				13285	34.27 9500	168.35 45775	138.30 36975				
CCQ5-6SDS2.5	51/4	5½	16	14	59.10	42.26	203.63	164.48				
					13285	9500	45775	37885				
CCQ5-8SDS2.5	51/4	7½	16	14	59.10	42.26	203.63	168.53	_			
000040000	F1/	05/	10	4.4	9500	7705	40615	33360	58660			
CCQ64SDS2.5	5½	3%	16	14	42.26	34.27	180.67	148.40	260.9			
CCQ66SDS2.5	5½	5½	16	14	9500	8855	49125	37175	63065			
000000000000000000000000000000000000000	072	072	10		42.26	39.39	218.53	165.37	280.5			
CCQ6-7.1SDS2.5	5½	71/8	16	14	9500	8855	49125	37175	63065			
					42.26 9500	39.39 8855	218.53 49125	165.37 37175	280.5 63065			
CCQ68SDS2.5	5½	71/2	16	14	42.26	39.39	218.53	165.37	280.5			
000740000 5	07/	05/	40	4.4	10700	7705	49845	40940				
CCQ74SDS2.5	6%	3%	16	14	47.60	34.27	221.73	182.12	_			
CCQ76SDS2.5	6%	5½	16	14	13285	9500	60290	48695	_			
000700002.0	078	372	10	14	59.10	42.26	268.19	216.61	_			
CCQ77SDS2.5	67/8	67/8	16	14	13285	9500	60290	49895	_			
					59.10	42.26	268.19	221.95	_			
CCQ78SDS2.5	67/8	71/2	16	14	13285 59.10	9500 42.26	60290 268.19	49895 221.95				
					10700	7705	200.13	221.33	78205			
CCQ7.1-4SDS2.5	71/8	3%	16	14	47.60	34.27		_	347.89			
00074 00000 5	71/	F1/	10	4.4	13285	9500	_	_	84085			
CCQ7.1-6SDS2.5	71/8	5½	16	14	59.10	42.26	_	_	374.04			
CCQ7.1-7.1SDS2.5	71/8	71/8	16	14	13285	9500		_	84085			
0007.1 7.10002.0	178	170	10		59.10	42.26		_	374.04			
CCQ7.1-8SDS2.5	71/8	71/2	16	14	13285	9500		_	84085			
					59.10 13285	42.26 9500	66990	50695	374.04			
CCQ86-SDS2.5	71/2	5½	16	14	59.10	42.26	298.00	225.51				
00000 0000 5	7. (747	40	4.4	13285	9500	66990	50695				
CCQ88-SDS2.5	7½	7½	16	14	59.10	42.26	298.00	225.51	_			
CCQ96-SDS2.5	83/4	5½	16	14	13285	9500	75920	61320				
00Q30-0D32.J	074	J 72	10	14	59.10	42.26	337.72	272.78	_			
CCQ98-SDS2.5	83/4	7½	16	14	13285	9500	75920	62830				
					59.10	42.26	337.72	279.49	<u>-</u>			
CCQ106-SDS2.5	9 1/2	51/2	16	14	13285 59.10	9500 42.26	84 855 37 7.47	64215 285.65				









Typical CCQ46SDS2.5 Installation

NOTE: Structural composite lumber columns have sides that show either the wide face or the edges of the lumber strands/veneers Values in the tables reflect installation into the wide face.

- 1. Factored down resistances are determined using ϕF_{CD} equal to: 812 psi (5.6 MPa) for D.Fir-L and 672 psi (4.64 MPa) for Spruce-Pine glulam sizes; 1092 psi (7.53 MPa) for SCL sizes; reduce where end grain bearing or buckling capacity of post governs.
- 2. Spliced conditions must be detailed by the Designer to transfer tension loads between spliced members by means other than the column cap.
- 3. Factored uplift resistances do not apply to splice conditions.
- 4. Post sides are assumed to lie in the same vertical plane as the beam sides.
- 5. Loads may not be increased for short-term loading.
- 6. Uplift loads have been increased 15% for earthquake or wind loading; reduce for other loading conditions in accordance with the code.
- 7. Designer to design beam for factored uplift resistance based on effective shear depth as per 12.2.1.4 CSA 086-14.
- 8. SCL assumes SG = 0.50
- 9. Beam depth must be greater than 71/4".
- 10. For uplift values when using SCL, use either D.Fir-L or S-P-F factored resistances based on SCL manufacturers recommendations.

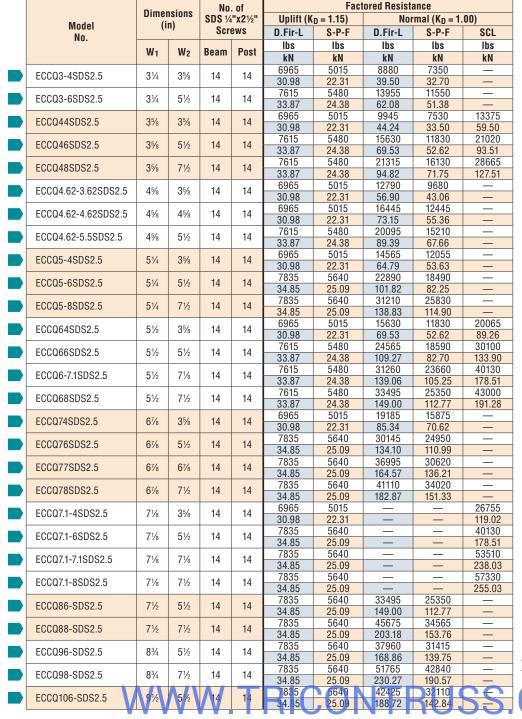


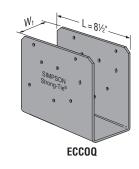
This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

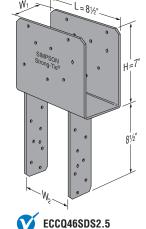
This design uses Simpson Strong-Tie® Strong-Drive® SDS Heavy-Duty Connector screws to provide faster installation. The Strong-Drive SDS Heavy-Duty Connector screws provide for a lower profile compared to standard through bolts. MATERIAL: ECCQ3, ECCQ4, ECCQ4.62, ECCQ6—7 gauge; all others—3 gauge. FINISH: Simpson Strong-Tie® gray paint, available in HDG; ECCOQ—uncoated. INSTALLATION: Fasteners provided. See General Notes.

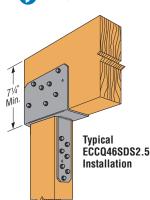
- Install 1/4"x21/2" Strong-Drive SDS Heavy-Duty Connector screws, which are provided with the column cap. (Lag screws will not achieve the same load.) **OPTIONS:** Straps may be rotated 90° where $W_1 \ge W_2$.
 - ECCOQ—may be ordered for field welding to pipe or other columns (no loads apply).
 - Custom sizes are available. Contact Simpson Strong-Tie for more information.
 - See page 73 for ECCLQ options.

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.









NOTE: Structural composite lumber columns have sides that show either the wide face or the edges of the lumber strands/veneers. Values in the tables reflect installation into the wide face.

- 1. Factored down resistances are determined using ϕ Fcp equal to: 812 psi (5.6 MPa) for D.Fir-L and 672 psi (4.64 MPa) for Spruce-Pine glulam sizes; 1092 psi (7.53 MPa) for SCL sizes; reduce where end grain bearing or buckling capacity of post governs.
- Spliced conditions must be detailed by the Designer to transfer tension loads between spliced members by means other than the column cap.
- 3. Factored uplift resistances do not apply to splice conditions.
- 4. Post sides are assumed to lie in the same vertical plane as the beam sides.
- 5. Loads may not be increased for short-term loading. 6. Uplift loads have been increased
- 15% for earthquake or wind loading; reduce for other loading conditions in accordance with the code.
- 7. ECCQ downloads assume a post of $W_1 \times W_2$.
- Designer to design beam for factored uplift resistance based on effective shear depth as per 12.2.1.4 CSA 086-14.
- 9. SCL assumes SG = 0.50
- 10. Beam depth must be greater than 71/4".
- 11. For uplift values when using SCL, use either D.Fir-L or S-P-F factored resistances based on SCL manufacturers recommendations.

CC/ECC/ECCU Column Caps



₹1½"

0

0

23/4" for Bolts

3/4" Bolts

3" for

11/4"

CC

The industry standard column caps. Precision factory gang-punched holes speed installation on this product line.

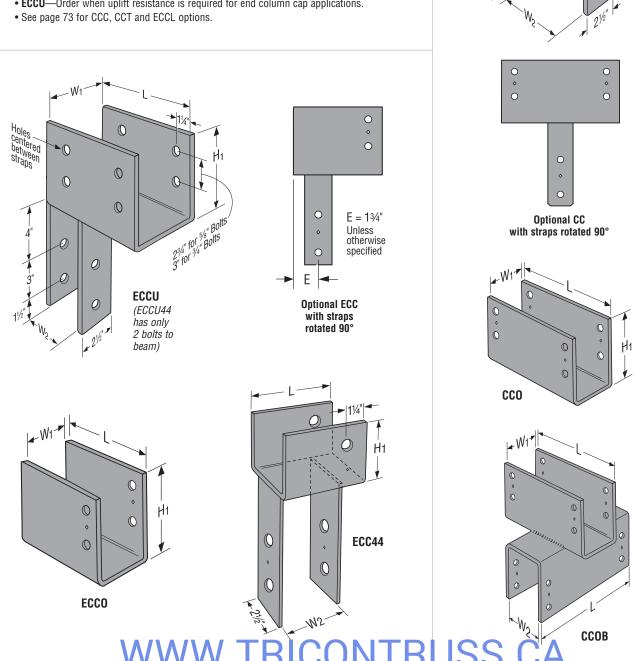
MATERIAL: CC31/4, CC44, CC4.62, CC6, ECC31/4, ECC4, ECC4.62, ECC6—7 gauge; all others—3 gauge

FINISH: Simpson Strong-Tie® gray paint; may be ordered HDG; CCO, ECCO—uncoated INSTALLATION: • Use all specified fasteners. See General Notes.

- \bullet Bolt holes shall be a minimum of 1/32" to a maximum of 1/16" larger than the bolt diameter (per 12.4.1.2 CSA 086-14).
- · Contact engineered wood manufacturer for connections that are not through the wide face.

OPTIONS: • Straps may be rotated 90° where $W_1 \ge W_2$ (see illustration).

- For special, custom, or rough cut lumber sizes, provide dimensions. An optional W_2 dimension may be specified with any column size given (note that the W_2 dimension on straps rotated 90° is limited by the W_1 dimension).
- CCO//≥ECCO—Column cap only (no straps) may be ordered for field-welding to pipe or other columns. No resistances apply. CCO/ECCO dimensions are the same as CC/ECC.
- CCOB—Any two CCO's may be specified for back-to-back welding to create a cross beam connector. Use the tabulated resistances; the resistance is no greater than that of the lesser element employed.
- **ECCU**—Order when uplift resistance is required for end column cap applications.

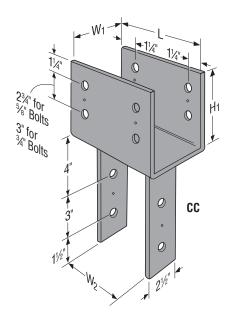


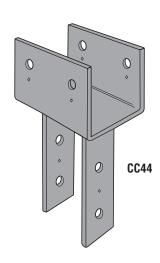
CC/ECC/ECCU Column Caps



These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

			nsions n)			Faste	eners		Factored Normal Resistance ($K_D = 1.00$)			
Model					Be	am	Po	st	D.Fir-L	S-P-F	SCL	
No.	W ₁	W ₂	L	H ₁		Dia.		Dia.	lbs	lbs	lbs	
		_			Qty.	(in)	Qty.	(in)	kN	kN	kN	
									23080	18955		
CC31/4-4	31/4	35/8	11	6½	4	5/8	2	5/8	102.67	84.32		
									27915	23100		
CC31/4-6	31/4	5½	11	6½	4	5/8	2	5/8	124.18	102.76	_	
					_		_		19895	15055	26755	
CC44	35%	3%	7	4	2	5/8	2	5/8	88.50	66.97	119.02	
CC46	35/8	5½	11	6½	4	5/8	2	5/8	31260	23660	42040	
0040	378	J 72	11	072	4	78		78	139.06	105.25	187.01	
CC48	35%	7½	11	6½	4	5/8	2	5/8	31260	23660	42040	
0040	078	1 /2	''	072		76		76	139.06	105.25	187.01	
CC4.62-3.62	45%	35/8	11	6½	4	5/8	2	5/8	33235	27295		
									147.84	121.42		
CC4.62-4.62	45/8	45/8	11	6½	4	5/8	2	5/8	40195	30420		
									178.80	135.32		
CC4.62-5.5	45/8	5½	11	6½	4	5/8	2	5/8	40195 178.80	30420 135.32		
									37850	31085		
CC51/4-4	51/4	35/8	13	8	4	3/4	2	3/4	168.37	138.28		
									51810	36980	_	
CC5¼-6	51/4	5½	13	8	4	3/4	2	3/4	230.47	164.50	_	
				_			_		54100	44770		
CC5½-8	51/4	7½	13	8	4	3/4	2	3/4	240.66	199.15	_	
0004	F1/	05/	44	C1/	4	E/	_	E/	40620	33360	58655	
CC64	5½	3%	11	6½	4	5/8	2	5/8	180.69	148.40	260.92	
CC66	5½	5½	11	6½	4	5/8	2	5/8	49125	37175	63065	
0000	372	072	'''	072		70		70	218.53	165.37	280.54	
CC6-71/8	5½	71/8	11	6½	4	5/8	2	5/8	49125	37175	63065	
	0,,,					,,,	_		218.53	165.37	280.54	
CC68	5½	7½	11	6½	4	5/8	2	5/8	49125	37175	63065	
									218.53	165.37	280.54	
CC74	6%	35/8	13	8	4	3/4	2	3/4	49850 221.75	40940	_	
									68235	182.12 48710		
CC76	6%	5½	13	8	4	3/4	2	3/4	303.54	216.68		
									71255	58970		
CC77	6%	6%	13	8	4	3/4	2	3/4	316.97	262.32	_	
									71255	58970	_	
CC78	6%	7½	13	8	4	3/4	2	3/4	316.97	262.32	_	
0071/ 4	71/	05/	10	_	4	2/	_	2/	_	_	78205	
CC7½-4	71/8	3%	13	8	4	3/4	2	3/4	_	_	347.89	
CC7½-6	71/8	5½	13	8	4	3/4	2	3/4	_	_	99370	
007 78-0	1 /8	J /2	10	0	1	74		74	_	_	442.04	
CC71/8-71/8	71/8	71/8	13	8	4	3/4	2	3/4	_		99370	
00170170	170	170	10			/ -	_	/ -	_		442.04	
CC86	7½	5½	13	8	4	3/4	2	3/4	75820	54120		
									337.28	240.75		
CC88	7½	7½	13	8	4	3/4	2	3/4	79170	59915		
									352.18 85025	266.53		
CC96	8%	5½	13	8	4	3/4	2	3/4	85925 382.23	61335 272.84		
									89725	74255	_	
CC98	8%	7½	13	8	4	3/4	2	3/4	399.13	330.32		
									96035	68550	_	
CC106	9½	51/2	13	8	1 4	3/4	2	3/4	427.20	304.94		





- 1. Post sides are assumed to lie in the same vertical plane as the beam sides.
- 2. Factored resistances may not be increased for short-term load duration.
- 3. Factored resistances are determined using ϕF_{cp} equal to: 812 psi (5.6 MPa) for D.Fir-L and 672 psi (4.64 MPa) for Spruce-Pine glulam sizes; 1092 psi (7.53 MPa) for SCL sizes; reduce where end grain bearing or buckling capacity of post governs
- end grain bearing or buckling capacity of post governs.

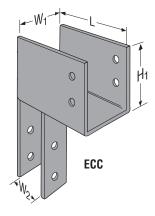
 4. Spliced conditions must be detailed by the Designer to transfer tension loads between spliced members by means other than the column cap.
- 5. SCL assumes SG = 0.50.
- 6. Beam depth must be greater than H₁.
 7. Contact Si npson Strong-Tie for uplift resistances.

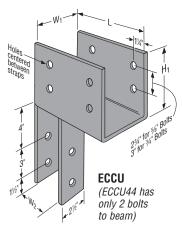
CC/ECC/ECCU Column Caps

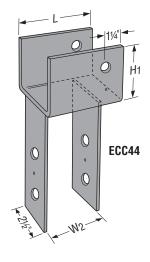
SIMPSON Strong-Tie

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

	Dimensions (in)						Fa	astenei	rs		Factored Normal Resistance (K _D = 1.00)			
Model			L			Beam Post					D.Fir-L	S-P-F	SCL	
No.	W ₁	W ₂			H ₁	Qt	ty.	Dia.		Dia.	lbs	lbs	lbs	
			ECC	ECCU		ECC	ECCU	(in)	Qty.	(in)	kN	kN	kN	
F0001/ 4	01/	05/	71/	01/	01/	_	4	<i>E</i> /	_	E/	8880	7350	_	
ECC31/4-4	31/4	3%	7½	9½	6½	2	4	5/8	2	5/8	39.50	32.70	_	
ECC31/4-6	31/4	5½	7½	9½	6½	2	4	5/8	2	5/8	13955	11550	_	
					-			-			62.08 9945	51.38 7530	13375	
ECC44	35/8	3%	5½	6½	4	1	2	5/8	2	5/8	44.24	33.50	59.50	
F0046	05/	5½	8½	9½	6½	2	4	5/8	2	5/8	15630	11830	21020	
ECC46	35/8	372	0 //2	97/2	0 1/2	2	4	78		9/8	69.53	52.62	93.51	
ECC48	35/8	7½	8½	9½	6½	2	4	5/8	2	5/8	21315	16130	28665	
											94.82 12780	71.75 9680	127.51	
ECC4.62-3.62	45/8	35/8	8½	9½	6½	2	4	5/8	2	5/8	56.85	43.06	_	
F00400 400	45/	45/	01/	01/	01/	_	4	E/		5/	16445	12445	_	
ECC4.62-4.62	4%	4%	8½	9½	6½	2	4	5/8	2	5/8	73.15	55.36	_	
ECC4.62-5.5	45/8	5½	8½	9½	6½	2	4	5/8	2	5/8	20095	15210	_	
200 1102 010	.,,	0,2	0,2	0,2	0,2	_	·	,,,	_	,,,	89.39	67.66	_	
ECC51/4-4	51/4	35/8	9½	10½	8	2	4	3/4	2	3/4	14565 64.79	12055 53.63	_	
											22890	18940	_	
ECC51/4-6	51/4	5½	9½	10½	8	2	4	3/4	2	3/4	101.82	84.25	_	
ECC51/4-8	51/4	7½	9½	10½	8	2	4	3/4	2	3/4	31210	25830	_	
EUU374-0	374	1 72	972	1072	0		4	74		74	138.83	114.90	_	
ECC64	5½	3%	7½	9½	6½	2	4	5/8	2	5/8	15630	11830	20065	
											69.53 24565	52.62 18590	89.26 30100	
ECC66	5½	5½	7½	9½	6½	2	4	5/8	2	5/8	109.27	82.70	133.90	
F000 71/	F1/	71/	01/	01/	C1/	0	4	5/	_	5/	31260	23660	40130	
ECC6-71/8	5½	71/8	9½	9½	6½	2	4	5/8	2	5/8	139.06	105.25	178.51	
ECC68	5½	7½	9½	9½	6½	2	4	5/8	2	5/8	33495	25350	43000	
											149.00 19185	112.77 15875	191.28	
ECC74	61/8	35/8	10½	10½	8	2	4	3/4	2	3/4	85.34	70.62	_	
F0070	07/	F1/	401/	401/		_	4	2/		2/	30145	24950	_	
ECC76	6%	5½	10½	10½	8	2	4	3/4	2	3/4	134.10	110.99	_	
ECC77	61/8	67/8	10½	10½	8	2	4	3/4	2	3/4	36995	30620	_	
											164.57	136.21	_	
ECC78	61/8	7½	10½	10½	8	2	4	3/4	2	3/4	41110 182.87	34020 151.33	_	
F0074 / 4	74.6	05/	101/	101/			_	2.4		0.4	—	—	26755	
ECC71/8-4	71/8	3%	10½	10½	8	2	4	3/4	2	3/4	_	_	119.02	
ECC71/8-6	71/8	5½	10½	10½	8	2	4	3/4	2	3/4		_	42040	
											_	_	187.01	
ECC71/8-71/8	71/8	71//8	10½	10½	8	2	4	3/4	2	3/4		_	53510 238.03	
					_	_			_		33495	25350		
ECC86	7½	5½	10½	10½	8	2	4	3/4	2	3/4	149.00	112.77	_	
ECC88	7½	7½	10½	10½	8	2	4	3/4	2	3/4	45675	34565	_	
						_			_		203.18	153.76	_	
ECC96	81/8	5½	10½	10½	8	2	4	3/4	2	3/4	37960 168.86	31415 139.75	_	
50000	67:		4-22	121.	_	_		0.	_	2.	51765	42840	_	
ECC98	8%	7½	10½	10½	8	2	4	3/4	2	3/4	230.27	190.57	_	
ECC106	9½	51/2	10½	101/2	8	2	4	3/4	2	3/4	42425	32110	_	
200100	3/2	7,4	171	X7	IA	_		3/4	1	1.1	188.72	142.84	66	







- 1. Post sides are assumed to lie in the same vertical plane as the beam sides.
- 2. Factored resistances may not be increased for short-term load duration.
- 3. Factored resistances are determined using ϕF_{CP} equal to: 812 psi (5.6 MPa) for D.Fir-L and 672 psi (4.64 MPa) for Spruce-Pine glulam sizes; 1092 psi (7.53 MPa) for SCL sizes; reduce where end bearing or buckling capacity of post governs.
- 4. ECC downloads assume a post of W₁ x W₂.
- 5. SCL assumes SG = 0.50.
- Beam depth must be greater than H₁.
 Contact Simpson Strong-Tie for uplift resistances.

ECCLQ/CCCQ/CCTQ Column Caps



The ECCLQ, CCCQ and CCTQ column caps provide high capacity, multiple beam to column connector options. The design uses Simpson Strong-Tie® Strong-Drive® SDS Heavy-Duty Connector screws to provide faster installation and a lower profile compared to standard through bolts. Screws are configured to provide high uplift design values.

MATERIAL: 7 gauge

FINISH: Simpson Strong-Tie® gray paint, also available in HDG

INSTALLATION:

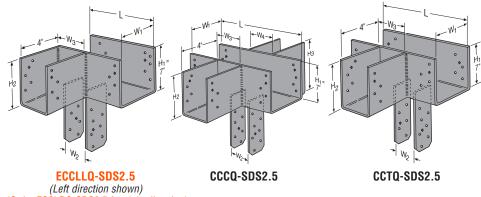
- Install ¼ "x2½" Strong-Drive SDS Heavy-Duty Connector screws, which are provided, in all round holes. (Lag screws will not achieve the same load.)
- No additional welding is allowed.

OPTIONS:

- Many combinations of beam and post sizes can be manufactured (refer to worksheet T-CCQLTC-WS).
- Available in widths up to 8" wide.
- ECCLQ is available in left or right side beam orientations. Specify ECCLLQ or ECCLRQ.

ORDERING:

- The L dimension varies depending on the width of the side stirrup (W₃ or W₄). Contact Simpson Strong-Tie for exact dimensions.
- Main beam stirrup height (H₁) is 7". Side beam stirrups (H₂ or H₃) can vary in height with the minimum height of 7". Specify the side stirrup height from the top of the cap.
- Example Order: End condition with a 4x main beam, 4x side beam and 4x post oriented to the left is an ECCLLQ44.



(Leπ airection snown)
(Order ECCLRQ-SDS2.5 for right direction)

		Facto	red Uplift Res	sistance (K _D =	1.15)		
	Main	Beam	Side	Beam	Total		
Model No.	D.Fir-L	S-P-F	D.Fir-L	S-P-F	D.Fir-L	S-P-F	
140.	lbs	lbs	lbs	lbs	lbs	lbs	
	kN	kN	kN	kN	kN	kN	
ECCLQ-SDS2.5	5345	3845	3075	2215	6335	4560	
EUULU-3D32.3	23.78	17.10	13.68	9.85	28.18	20.28	
CCCQ-SDS2.5	7200	5185	3920	2825	7200	5185	
000Q-3D32.3	32.03	23.06	17.44	12.57	32.03	23.06	
CCTQ-SDS2.5	8140	5900	3920	2825	8875	6390	
001Q-3D32.3	36.21	26.25	17.44	12.57	39.48	28.43	

- 1. Factored resistances are per seat. Side beams must be loaded symmetrically for the CCCQ.
- The combined uplift loads applied to all beams in the connector must not exceed the total factored resistance listed in the table
- 3. The combined factored download for all of the carried beams shall not exceed the factored normal resistance for the unmodified product on pages 68-69 (CCQ value for CCCQ and CCTQ, or ECCQ value for ECCLQ). The maximum factored download for each side beam shall not exceed 35% of the maximum factored normal resistance for the unmodified product or 11100 lbs (49.38 kN).

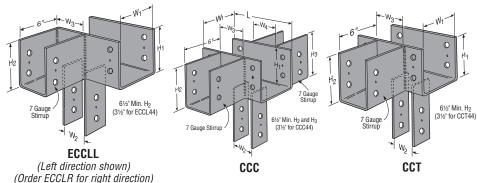
ECCL/CCC/CCT Column Caps

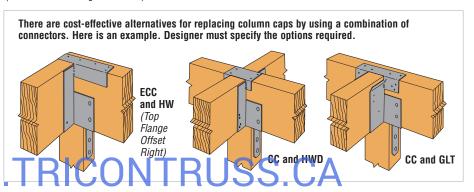
Column to beam connections often have multiple beams framing on top of a column. L, T, and Cross column caps provide design solutions for this application. Many combinations of beam and post sizes can be manufactured (refer to worksheet T-CCLTC-WS for details) with the following criteria applied:

- The factored resistance shall be determined from the capacity for the unmodified product (see pages 71-72). The side beam can take a maximum of 40% of the download and shall not exceed 13,640 lbs (60.68 kN). The sum of the loads for the side beam(s) and main beam can not exceed the tabulated values.
- The column width in the direction of the main beam width must be the same as the main beam width (W₁).
- Specify the stirrup height from the top of the cap. The minimum side stirrup heights (H₂ or H₃) is 6½" (3½" for 44's).
- The L dimension may vary depending on the width of the side stirrup (W₃ or W₄).
- Column caps may be ordered without the column straps for field welding to a column.
 No loads apply. Specify CCOC/CCOT/ECCOL.

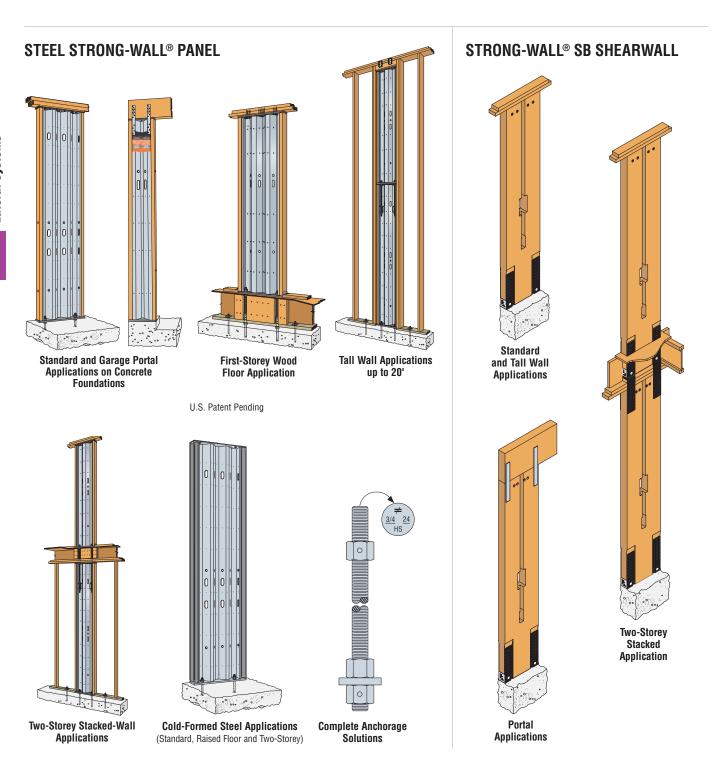
Ordering Examples:

- A CCC66C with W₃ = 5½", H₂ and H₃ = 6½" is a CC66C column cap with 5½" beams on each side with all beam seats flush.
- An ECCL66C with W₃ = 3⁵/₈", H₂ = 7¹/₂" is an ECC66C end column cap with a 4x beam on the right side (specify direction left (which is shown) or right for stirrup) and stirrup seat 1" below the cap seat.

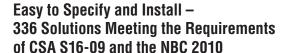




As the industry leader in Lateral Systems innovation and testing, Simpson Strong-Tie is in a unique position to gain insight from Designers and builders as to what they need in a pre-fabricated shearwall. This insight continues to drive innovation in our Steel and Wood Strong-Wall® product lines resulting in new products and expanded code-listed solutions.



For more information on Strong-Wall Shearwalls, contact Simpson Strong-Tie at (800) 999-5099 or visit our website at www.strongtie.com.



The Simpson Strong-Tie® Strong Frame® moment frame provides designers with the flexibility and performance they need while offering contractors the speed and efficiency of bolted connections. Now we have nearly doubled our offering by adding a new 16'-tall column and 14', 18' and 20'-wide beams, resulting in a total of 336 frame configurations to choose from.



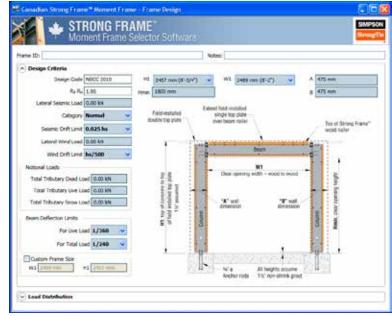


New Custom Sizes

If one of our standard sizes of Strong Frame doesn't suit your project, have no fear – we now offer custom sizes to fit almost any project. Using our standard Strong Frame column and beam profiles we can manufacture frames to your size specifications in widths ranging from 6' to 20'-4" and heights from 6' to 19'-101/2". Now you can get just the right size Strong Frame for your new or retrofit project with lead times that are typically less than six weeks. Call your local Simpson Strong-Tie representative for more details on the ordering process and lead times.

Canadian Strong Frame® **Selector Software**

The Simpson Strong-Tie® Strong Frame® Selector software is designed to help the Designer select an appropriate frame for their given geometry and loading. Only minimum inputs are required for the software to select an appropriate frame for the available space. Based on input geometry, the Strong Frame Selector software will narrow down from the 336 available stock frames to a handful of possible solutions. It can also help with custom frame designs. Download your free copy at www.strongtie.com/strongframe.



SIMPSON
Strong-Tie

The Simpson Strong-Tie® Anchor Tiedown System (ATS) is designed to anchor stacked shearwalls in multi-storey wood-frame buildings while compensating for shrinkage effects commonly seen in these types of structures. The system is comprised of threaded rods, bearing plates, couplers and nuts, used in combination with our proprietary shrinkage take-up devices to resist

ATS serves the same purpose as conventional holdowns, though it can be configured to provide significantly higher resistances (up to 60,000 lbs).

shearwall overturning forces.



THE ATS ADVANTAGE

Proven Product Performance:

The ATS products have been thoroughly tested in full-scale wall systems and structures to develop proven products and design philosophies.

High-Quality Products:

All ATS components are manufactured and assembled in the USA to the highest quality standards to ensure consistent performance.

No-Equal Support:

Simpson Strong-Tie has dedicated sales and engineering staff to support ATS as well as our unmatched field service from our national sales force. Any field issues or questions, about any of our products, are resolved quickly and professionally by our highly-trained staff.

Specifications Made Easy:

Specifying ATS is more complex than specifying other products in our line as one can't just turn to a page in a catalogue with a single load and select the product. To best serve our customers and to assist engineers in specifying the product we have developed multiple methods of specification and a support service.

For more information on specification, please see our *Anchor Tiedown Systems Canadian Limit States Design* catalogue (C-ATSCAN11) or visit *www.strongtie.com/ats*.



FACE MOUNT HANGERS LUCZ/LU/U/HU Standard Joist Hangers



LUCZ concealed flange hanger is available for 2x6, 2x8, 2x10 and 2x12 lumber. Ideal for end of ledger/header or post conditions, the LUCZ also provides cleaner lines for exposed conditions such as overhead decks.

See Hanger tables on pages 79-84. See Hanger Options on page 230 for hanger modifications, which may result in reduced resistances.

LU—Value engineered for strength and economy. Precision-formed—engineered for installation ease and design value.

U—The standard U hanger provides flexibility of joist to header installation. Versatile fastener selection with tested factored resistances.

HU—Most models have triangle and round holes. To achieve maximum resistances, fill both round and triangle holes with common nails. These heavy-duty connectors are designed for schools and other structures requiring additional strength, longevity and safety factors.

MATERIAL: See tables on pages 79-84.

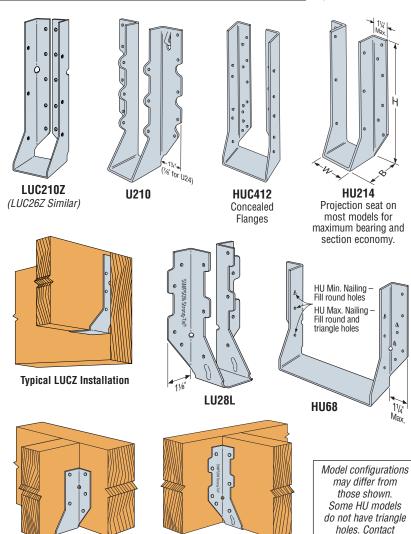
FINISH: Galvanized. Some products available in ZMAX® coating.

INSTALLATION:

- · Use all specified fasteners. See General Notes.
- HU—can be installed filling round holes only, or filling round and triangle holes for maximum values.
- Joists sloped up to 1/4:12 achieve tabulated values.
- For installations to masonry or concrete see page 201.
- HU hangers can be welded to a steel member refer to technical bulletin T-HUHUC-W.

OPTIONS: • HU hangers available with the header flanges turned in for 25/16" width and larger, with no reduction in resistances—order HUC hanger.

- See Hanger Options on pages 230-231 for sloped and/or skewed U/HU models, and HUC (concealed flange) models.
- HU only—rough beam sizes available by special order.
- See page 84 for stocked U hanger rough sizes tables. Rough sizes are not available in 8x.
- · Also see LUS and HUS series.



Typical LU28L Installation

JOIST FACTORED SHEAR RESISTANCES

The maximum capacity of a horizontal joist or rafter may be limited by its factored shear resistance (V_r) . This table gives the capacity for common sizes.

	I	Factored Shear	Resistance (Vr)
Joist	D.F	ir-L	S-I	P-F
or	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$
Rafter	lbs	lbs	lbs	lbs
	kN	kN	kN	kN
2x4	1470	1695	1160	1335
2.84	6.54	7.54	5.18	5.95
2x6	1900	2200	1505	1730
280	8.51	9.79	6.71	7.71
2x8	2150	2475	1695	1945
ZXO	9.59	11.02	7.54	8.67
2x10	2515	2895	1985	2280
2X I U	11.21	12.89	8.83	10.16
2x12	2785	3205	2195	2525
2.8.1.2	12.41	14.27	9.78	11.25

Typical HU Installation

 Factored shear resistances shown assume a single member system factor (K_H=1.00). Resistances may be increased as per 6.4.4 CSA 086-14 for Case 1 and Case 2 systems.

Simpson Strong-Tie.

2. Resistances shown are for No. 1/No. 2 grades.

FACE MOUNT HANGERS LUS/HUS Double Shear Joist Hangers

SIMPSON Strong-Tie



This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

See Hanger tables on pages 79-84. See Hanger Options on page 230 for hanger modifications, which may result in reduced resistances.

All hangers in this series have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation, and the use of standard nails for all connections. (Do not bend or remove tabs.)

MATERIAL: See tables, pages 79-84.

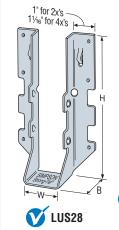
FINISH: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pages 14-17.

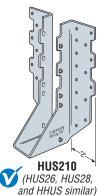
INSTALLATION: • Use all specified fasteners. See General Notes.

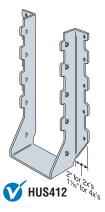
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances.
- · Not designed for welded or nailer applications.
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated resistance.
- With 3x carrying members, use 16dx21/2" nails into the header and 16d commons into the joist with no reduction in resistances. With a single 2x carrying member, use 10dx1½" nails into the header and 10d commons into the joist, reduce the resistance to 0.64 of the table value.

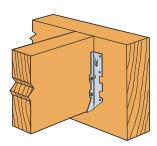
OPTIONS: • LUS hangers cannot be modified.

- HUS hangers available with the header flanges turned in for 2-2x (31/8") and 4x only, with no reduction in resistances. See the HUSC Concealed Flange illustration.
- See Hanger Options, page 230.

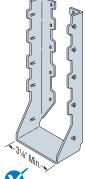








Typical LUS28 Installation Standard LUS28 Installation use .148x3" (10d common)



W HUSC Concealed Flanges (not available for HHUS, HGUS and HUS2x)



Double Shear **Nailing** Top View



Double Shear **Nailing** Side View Do not bend tab back



Dome Double Shear Nailing prevents tabs from breaking off (available on some models)

U.S. Patent 5.603.580

HUCQ Heavy Duty Joist Hangers

The HUCQ series are heavy duty joist hangers that incorporate Simpson Strong-Tie® Strong-Drive® SDS Heavy-Duty Conntector screws. Designed and tested for installation at the end of a beam or on a post, they provide a strong connection with fewer fasteners than nailed hangers.

MATERIAL: 14 gauge

FINISH: Galvanized. Most models available in stainless steel or ZMAX® coating.

INSTALLATION: • Use all specified fasteners. See General Notes.

- · Strong-Drive SDS Heavy-Duty Conntector screws supplied.
- For use on solid sawn or engineered wood products.

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

9

3

12-1/4"x21/2" SDS

12-1/4"x21/2" SDS

45/8

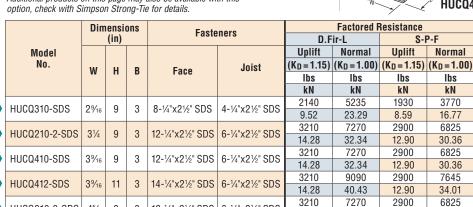
51/2 9 3

51/2

HUCQ210-3-SDS

HUCQ610-SDS

HUCQ612-SDS



6-1/4"x21/2" SDS

6-1/4"x21/2" SDS

14.28

3210

14.28

3210

14.28

32.34

7270

32.34

9090

12.90

2900

12.90

2900

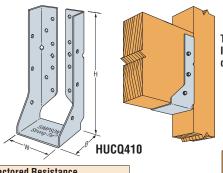
30.36

6825

30.36

645

34.01



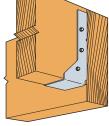


Typical HUCQ

Installation

on a Beam

Typical HUCO Installation on a Post







HEAVY-DUTY CONNECTOR Screw

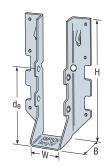
- 15% for earthquake or wind loading with no further increase allowed. Reduce by 15% for standard term loading such as cantilever construction
- 2. When using structural composite lumber columns, Strong-Drive® SDS Heavy-Duty
 Connector screws must be applied to the wide



These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

					Dimensions				Fasten	ers		Factored F	Resistance		
					(i	n)			газісн	CIS		ir-L	S-I	P-F	Installed
	Joist	Model	Ga								Uplift	Normal	Uplift	Normal	Installed Cost
	Size	No.	uu	w	н	В	de ⁶	Min/	Header	Joist	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$	Index
				VV	"		ue	Max	licauci	Juist	lbs	lbs	lbs	lbs	
											kN	kN	kN	kN	
ĺ								SA	NN LUMB	ER SIZES					
ł											360	1020	320	725	
		LU24L	22	1%16	31/8	1%	211/16	—	4-10d	2-10dx1½	1.60	4.54	1.42	3.22	Lowest
											710	1625	645	1155	
		LUS24	18	1%16	31/8	1¾	21/4	—	4-10d	2-10d	3.16	7.23	2.87	5.14	+11%
	2x4										450	1340	355	1030	
		U24	16	1%16	31/8	11/2	1 13/16	—	4-10d	2-10dx1½	2.00	5.96	1.58	4.58	+90%
1											490	1525	450	1080	
		HU26	14	1%16	31/16	21/4	11/16	—	4-16d	2-10dx1½	2.18	6.78	2.00	4.80	+490%
ŀ											835	2020	590	1435	
		LUS24-2	18	31/8	31/8	2	117/32	—	4-16d	2-16d	3.71	8.99	2.62	6.38	Lowest
	DBL										480	1340	445	1030	
	2x4	U24-2	16	31/8	3	2	1 15/16	—	4-10d	2-10d	2.14	5.96	1.98	4.58	+59%
	-//	HU24-2/									525	1710	490	1585	
		HUC24-2	14	31/8	31/16	21/2	211/16	—	4-16d	2-10d	2.34	7.61	2.18	7.05	+244%
ŀ											720	1605	645	1140	
		LU26L	22	1%16	5	1%	419/32	—	6-10d	4-10dx1½	3.20	7.14	2.87	5.07	Lowest
											1420	2170	1290	1630	
		LUS26	18	1%16	43/4	1¾	325/32	—	4-10d	4-10d	6.32	9.65	5.74	7.25	+10%
			.								895	2005	780	1860	
	2x6	U26	16	1%16	4¾	2	315/16	—	6-10d	4-10dx1½	3.98	8.92	3.47	8.27	+70%
											830	1605	710	1140	
		LUC26Z	18	1%16	43/4	1¾	4	—	6-10d	4-10dx1½	3.69	7.14	3.16	5.07	+74%
			l								2705	4940	2065	3875	
		HUS26	16	1%	5%	3	315/16	—	14-16d	6-16d	12.03	21.97	9.20	17.24	+320%
Ì					_		4.0.1				760	1605	680	1140	
		LU26-2L	20	31/8	5	1%	419/32	—	6-10d	4-10dx1½	3.38	7.14	3.02	5.07	Lowest
		111000 0	40	01/	47/	_	41/		4.40.1	4.40.1	1720	2595	1545	1920	0.40/
		LUS26-2	18	31/8	41//8	2	41/32	—	4-16d	4-16d	7.65	11.54	6.87	8.54	+24%
	DBL	1100.0	10	01/		_	0127		0.40-1	4.40.1	960	2675	890	2475	40.40/
	2x6	U26-2	16	31/8	5	2	313/16	-	8-10d	4-10d	4.27	11.90	3.96	11.01	+124%
								N 41	0.40-1	4.40.1	1055	3420	980	2845	0500/
İ		HU26-2/	4.4	01/	F3/	01/	5	Min	8-16d	4-10d	4.69	15.21	4.36	12.66	+358%
İ		HUC26-2	14	31/8	5%	2½	5	Max	10.164	6 104	1580	4415	1470	3135	. 2720/
								Max	12-16d	6-10d	7.03	19.64	6.54	13.95	+372%
Ì		LUS26-3	18	45/8	41/8	2	3%32		4-16d	4-16d	1720	2595	1545	2340	Lowest
		LU320-3	10	478	478		3 732		4-10u	4-10u	7.65	11.54	6.87	10.41	Lowest
		U26-3	16	45/8	41/4	2	311/16	_	8-10d	4-10d	960	2675	890	2475	+87%
	TPL	020-3	10	478	4/4		J /16		0-10u	4-10u	4.27	11.90	3.96	11.01	+07 /0
	2x6							Min	8-16d	4-10d	1055	3420	980	2845	+193%
		HU26-3/	14	411/16	5%	21/2	5	IVIIII	0 100	- 10u	4.69	15.21	4.36	12.66	110070
		HUC26-3	'	7 710	078	L /2		Max	12-16d	6-10d	1580	4415	1470	3135	+198%
								IVIUX	12 100	0 100	7.03	19.64	6.54	13.95	110070
		LU26L	22	1%6	5	1%	419/32	_	6-10d	4-10dx1½	720	1605	645	1140	Lowest
				.,,,,		.,,	. / 02		0.00		3.20	7.14	2.87	5.07	
ь		LUS26	18	1%6	43/4	13/4	325/32	l —	4-10d	4-10d	1420	2170	1290	1630	+10%
											6.32	9.65	5.74	7.25	
		LU28L	20	1%6	6¾	1%	5%	_	8-10d	6-10dx1½	1140	2185	1020	1550	+29%
											5.07	9.72	4.54	6.89	
		LUS28	18	1%16	6%	1¾	325/32	_	6-10d	4-10d	1420	2520	1290	1790	+42%
	2x8							-			6.32	11.21	5.74	7.96	
		U26	16	1%16	43/4	2	315/16	—	6-10d	4-10dx1½	895	2005	780	1860	+70%
											3.98 830	8.92 1605	3.47 710	8.27	
		LUC26Z	18	1%16	43/4	13/4	4	—	6-10d	4-10dx1½	3.69	7.14		1140 5.07	+70%
											980	2565	3.16 905	2380	
		HU28	14	1 %16	51/4	21/4	47/8	—	6-16d	4-10dx1½	4.36	11.41	4.03	10.59	+415%
											3605	5365	2675	4345	
		HUS28	16	15/8	73/32	3	63/32	—	22-16d	8-16d	16.04	23.86	11.90	19.33	+457%
L							l				10.07	20.00	11.00	10.00	



- 1. 10d common nails may be used instead of the specified 16d nails at 0.83 of the tabulated value.
- Factored uplift resistances have been increased 15% for earthquake or wind loading with no further increase allowed. Reduce by 15%
- for standard term loading such as in cantilever construction.

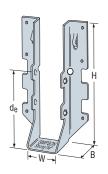
 3. MIN nailing quantity and factored resistances—fill all round holes

 MAX nailing quantity and factored resistances—fill all round and triangle holes
- 4. D.Fir-L factored resistances can be used for most LVL. Verify with manufacturer prior to selecting hanger.
- 5. See page 22 for hangers with reduced capacity due to installation with different nails.
- 6. de is the distance from the bearing seat to the top joist nail.
- 7. **NAILS:** 16d = 0.162" dia. x 3½" long, 10d = 0.148" dia. x 3" long, 10dx1½ = 0.148" dia. x 1½" long. See pages 22-23 for other nail sizes and information. Hangers do not have an Installed Cost Index.

					Dimer				Fasten	ers		Factored F			
					(i	n)			1 431011			ir-L		P-F	Installed
	Joist	Model	Ga								Uplift	Normal	Uplift	Normal	Installed Cost
	Size	No.		W	н	В	de ⁶	Min/	Header	Joist	· - /	$(K_D = 1.00)$,	,	Index
							46	Max	liouuoi	00.01	lbs	lbs	lbs	lbs	
											kN	kN	kN	kN	
								SAV	NN LUMB	ER SIZES					
		LUS26-2	10	01/	17/	0	41/		4 104	4 164	1720	2595	1545	1920	Lowest
		LU320-2	18	31/8	47/8	2	41/32	-	4-16d	4-16d	7.65	11.54	6.87	8.54	Lowest
			00	01/	02/	457	5 7/		0.40.1	0.401.41/	1140	2185	1020	1550	F0/
		LU 28-2L	20	31//8	6¾	1%	57/8	-	8-10d	6-10dx1½	5.07	9.72	4.54	6.89	+5%
		111000	40	04/		_	44.7		0.40.1	4.40.1	1720	3325	1545	2575	100/
		LUS28-2	18	31/8	7	2	41/32	-	6-16d	4-16d	7.65	14.79	6.87	11.45	+10%
	DBL				_		2.27				960	2675	890	2475	2.40/
	2x8	U26-2	16	31//8	5	2	313/16	-	8-10d	4-10d	4.27	11.90	3.96	11.01	+84%
											2540	3620	1805	2570	
		HUS28-2	14	31//8	73/16	2	515/16	_	6-16d	6-16d	11.30	16.10	8.03	11.43	+214%
											1055	4270	980	3135	
		HU28-2/						Min	10-16d	4-10d	4.69	18.99	4.36	13.95	+296%
		HUC28-2	14	31/8	7	21/2	6%				1580	5780	1470	4225	
								Max	14-16d	6-10d	7.03	25.71	6.54	18.79	+307%
_											1720	3325	1545	2375	
	TPL	LUS28-3	18	45/8	61/4	2	31/32	_	6-16d	4-16d	7.65	14.79	6.87	10.56	Lowest
	2x8	HU26-3/									1055	3420	980	2845	
		HUC26-3	14	411/16	5%	21/2	5	—	8-16d	4-10d	4.69	15.21	4.36	12.66	+192%
											1230	4270	1140	3135	
	OLIAD	111100 47						Min	10-16d	4-16d	5.47	18.99	5.07	13.95	*
	QUAD 2x8	HU28-4/ HUC28-4	14	61/8	7	21/2	67/16				1840	5780	1710	4225	
	2/10							Max	14-16d	6-16d	8.18	25.71	7.61	18.79	*
											1140	2185	1020	1550	
		LU28L	20	1 %16	6%	11/2	5½	—	8-10d	6-10dx1½	5.07	9.72	4.54	6.89	Lowest
												2520			
		LUS28	18	1 %16	6%	13/4	325/32	_	6-10d	4-10d	1420		1290	1790	+10%
											6.32	11.21	5.74	7.96	
		LU210L	20	1%16	8	1%	715/32	_	10-10d	6-10dx1½	1140	2495	1020	1770	+18%
											5.07	11.10	4.54	7.87	
		LUS210	18	1%16	713/16	13/4	3%	_	8-10d	4-10d	1420	2785	1290	2210	+23%
	2x10										6.32	12.39	5.74	9.83	
		LUC210Z	18	1%16	7¾	13/4	5½	—	10-10d	6-10dx1½	1240	2495	1130	1770	+85%
											5.52	11.10	5.03	7.87	
		U210	16	1 %16	713/16	2	53/4	—	10-10d	6-10dx1½	1345	2755	1235	1955	+92%
											5.98	12.25	5.49	8.70	
		HU210	14	1 %16	71/8	21/4	6¾	—	8-16d	4-10dx1½	980	3420	905	2865	+300%
								-			4.36	15.21	4.03	12.74	
		HUS210	16	15/8	93/32	3	731/32	—	30-16d	10-16d	4505	5795	4010	4740	+472%
											20.04	25.78	17.84	21.08	
		LUS28-2	18	31//8	7	2	41/32	—	6-16d	4-16d	1720	3325	1545	2575	Lowest
											7.65	14.79	6.87	11.45	
		LU210-2L	20	31//8	8	15/8	715/32	—	10-10d	6-10dx1½	1140	2495	1020	1770	+16%
											5.07	11.10	4.54	7.87	
		LUS210-2	18	31//8	9	2	61/32	—	8-16d	6-16d	2580	4500 20.02	2320	3195 14.21	+30%
											11.48		10.32		
	DD	U210-2	16	31/8	81/2	2	611/16	—	14-10d	6-10d	1440	4355	1340	3090	+99%
	DBL 2x10										6.41	19.37	5.96	13.75	
	LATO	HUS210-2	14	31/8	93/16	2	715/16	—	8-16d	8-16d	3795 16.88	5690 25.31	3450	4570	+252%
													15.35	20.33	
		1111040 61						Min	14-16d	6-10d	1580	5780	1470	4225	+339%
		HU210-2/ HUC210-2	14	31/8	813/16	21/2	87/16				7.03	25.71	6.54	18.79	
		1100210-2						Max	18-16d	10-10d	2635	5780	2450	4690	+352%
											11.72	25.71	10.90	20.86	
		HHUS 210-2	14	35/16	93/16	3	8	—	30-16d	10-16d	4745	9660	4310	7000	+385%
Į											21.11	42.97	19.17	31.14	



- These productsare available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.
- These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.



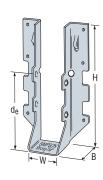
See footnotes on page 79.

SIMPSON
Strong-Tie

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

				Dimer	nsions			Easten	are		Factored F	Resistance		
					n)			Fasten	ers	D.F	ir-L	S-I	P-F	1
Joist	Model	0-								Uplift	Normal	Uplift	Normal	Installed
Size	No.	Ga			_	.1.6	Min/		1.1.4	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$	Cost Index
			W	Н	В	de ⁶	Max	Header	Joist	lbs	lbs	lbs	lbs	IIIucx
										kN	kN	kN	kN	
							CVI	A/N I IIME	ER SIZES					
							UA.	VIV LOWIL	LII OIZEO	1720	3325	1545	2375	
	LUS28-3	18	45/8	61/4	2	3%32	—	6-16d	4-16d	7.65	14.79			Lowest
										2580	3345	6.87 2320	10.56 2375	
	LUS210-3	18	45/8	83/16	2	5%16	—	8-16d	6-16d	11.48	14.88	10.32		+9%
										1440	4355	1340	10.56 3090	
TPL	U210-3	16	45/8	73/4	2	5¾	<u> </u>	14-10d	6-10d	6.41	19.37	5.96	13.75	+130%
2x10										1580	5780	1470	4225	
2,1.0	HU210-3/						Min	14-16d	6-10d	7.03	25.71	6.54	18.79	+296%
	HUC210-3	14	411/16	8%16	2½	87/16				2635	5780	2450	4690	
							Max	18-16d	10-10d	11.72	25.71	10.90	20.86	+303%
			444.7			745 (00.40.1	40.40.1	4745	10545	4310	7485	2000/
	HHUS210-3	14	411/16	9	3	715/16	—	30-16d	10-16d	21.11	46.91	19.17	33.29	+690%
							N // :	11101	0.404	1840	5780	1710	4225	Laurant
	HU210-4/	14	61/	09/	01/	07/	Min	14-16d	6-16d	8.18	25.71	7.61	18.79	Lowest
Quad	HUC210-4	14	61/8	8%16	2½	87/16	Max	18-16d	10-16d	2455	5780	2280	4690	+3%
2x10							Max	10-100	10-100	10.92	25.71	10.14	20.86	+370
	HHUS210-4	14	61/8	87/8	3	713/16	_	30-16d	10-16d	4745	10545	4310	7485	+275%
	11103210-4	14	078	078	J	1 716		30-10u	10-100	21.11	46.91	19.17	33.29	TZ13/0
	LU 210L	20	1%6	713/16	1%	715/32	_	10-10d	6-10dx1½	1140	2495	1020	1770	Lowest
	20 2:02		1710	. ,	.,,	. , , ,			0 104/172	5.07	11.10	4.54	7.87	20001
	LUS210	18	1%16	713/16	13/4	37/8	_	8-10d	4-10d	1420	2785	1290	2210	+5%
										6.32	12.39	5.74	9.83	
	LUC210Z	18	1%16	73/4	13/4	5½	l —	10-10d	6-10dx1½	1240 5.52	2495	1130	1770	+56%
2x12										1345	11.10 2755	5.03 1235	7.87 1955	
	U210	16	1%16	713/16	2	5¾	—	10-10d	6-10dx1½	5.98	12.25	5.49	8.70	+63%
										4505	5795	4010	4740	
	HUS210	16	1%	93/32	3	731/32	—	30-16d	10-16d	20.04	25.78	17.84	21.08	+384%
										1470	4020	1360	3135	
	HU212	14	1%16	9	21/4	85/8	—	10-16d	6-10dx1½	6.54	17.88	6.05	13.95	+487%
						=45.6				1140	2495	1020	1770	
	LU210-2L	20	31/8	8	1%	715/32	—	10-10d	6-10dx1½	5.07	11.10	4.54	7.87	Lowest
	1110010.0	10	01/	0	_	C1/		0.404	0.404	2580	4500	2320	3195	100/
7	LUS210-2	18	31/8	9	2	61/32	—	8-16d	6-16d	11.48	20.02	10.32	14.21	+12%
	U210-2	16	31/8	81/2	2	611/16	_	14-10d	6-10d	1440	4355	1340	3090	+72%
	0210-2	10	3 /8	0 /2		U /16		14-10u	0-100	6.41	19.37	5.96	13.75	+1 2 /0
	LUS214-2	18	31/8	1015/16	2	61/16	_	10-16d	6-16d	2580	5355	2320	3875	+110%
DBL	2002112		070	10 710		0710		10 100	0 100	11.48	23.82	10.32	17.24	111070
2x12	HUS210-2	14	31/8	93/16	2	715/16	_	8-16d	8-16d	3795	5690	3450	4570	+203%
										16.88	25.31	15.35	20.33	
	HUS212-2	14	31/8	10¾	2	97/8	—	10-16d	10-16d	4745	7015 31.20	3650	4980	+235%
							-			21.11 1580	5780	16.24 1470	22.15 4225	
	HU212-2/						Min	16-16d	6-10d	7.03	25.71	6.54	18.79	+333%
	HUC212-2/	14	31/8	10%6	2½	103/16				2635	5780	2450	4690	
							Max	22-16d	10-10d	11.72	25.71	10.90	20.86	+347%
					_					2580	3345	2320	2375	
	LUS210-3	18	4%	83/16	2	5%16	-	8-16d	6-16d	11.48	14.88	10.32	10.56	Lowest
	11040 0	10	45/	72/	_	F2/		11101	0.404	1440	4355	1340	3090	.1100/
TPL	U210-3	16	45/8	73/4	2	5¾	_	14-10d	6-10d	6.41	19.37	5.96	13.75	+112%
2x12							Min	16-16d	6-10d	1580	5780	1470	4225	+378%
	HU212-3/	14	411/16	105/16	2½	915/16	141111	10 100	U TOU	7.03	25.71	6.54	18.79	107070
	HUC212-3	'-	7 710	10/10	L /2	7 / 10	Max	22-16d	10-10d	2635	5780	2450	4690	+386%
							ax	00	.5 150	11.72	25.71	10.90	20.86	. 555 /0
Quad	HHUS210-4	14	61/8	87/8	3	713/16	_	30-16d	10-16d	4745	10545	4310	7485	*
2x12		Ė								21.11	46.91	19.17	33.29	
	U34	16	2%16	3%	2	23/8	_	4-10d	2-10dx1½	450	1340	355	1030	Lowest
3x4	11110.47									2.00	5.96	1.58	4.58	
	HU34/ HUC34	14	2%16	3%	2½	3	—	4-16d	2-10dx1½	490 2.18	1710 7.61	455 2.02	1585 7.05	+101%
	110004									2.18	7.01	2.02	7.00	



See footnotes on page 79.

					sions			Fasten	ore	Factored Resistance				
				(i	n)			rasten	ers	D.F	ir-L	S-I	P-F]
oist	Model	Ga								Uplift	Normal	Uplift	Normal	Instal Cos
Size	No.	- Gu	w	н	В	de ⁶	Min/	Header	Joist	` - /	$(K_D = 1.00)$	· - /	,	Inde
					"	ue	Max	liouuoi	00101	lbs	lbs	lbs	lbs	
										kN	kN	kN	kN	
							SAV	NN LUME	ER SIZES					
	U36	16	2%16	5%	2	45/16	_	8-10d	4-10dx1½	895	2675	780	2475	Low
	030	10	Z716	378		4716		0-10u	4-10ux172	3.98	11.90	3.47	11.01	LUW
3x6	LUS36	18	2%16	51/4	2	45/16	_	4-16d	4-16d	1720	2290	1545	1630	+26
OAO		10	2710	074	_	1710		1 100	1 100	7.65	10.19	6.87	7.25	120
	HU36/ HUC36	14	2%16	5%	2½	5	_	8-16d	4-10dx1½	980	3420	905	2845	+18
	пособ									4.36	15.21 2675	4.03	12.66 2475	
	U36	16	2%16	5%	2	415/16	—	8-10d	4-10dx1½	895 3.98	11.90	780 3.47	11.01	Low
3x8	HU38/									980	4270	905	3135	
	HUC38	14	2%16	71/8	2½	6¾	—	10-16d	4-10dx1½	4.36	18.99	4.03	13.95	+15
					_					1720	3325	1545	2575	
	LUS310	18	2%16	71/4	2	45/16	—	6-16d	4-16d	7.65	14.79	6.87	11.45	Low
x10	U310	16	2%16	87/8	2	53/4		14-10d	6-10dx1½	1345	4355	1235	3090	+39
XIU	0310	10	Z%16	078		3%4	_	14-100	0-100X172	5.98	19.37	5.49	13.75	+38
	HU310/	14	2%16	87/8	2½	81/2	_	14-16d	6-10dx1½	1470	5780	1360	4225	+15
	HUC310	17	2710	078	L /2	072		14 100	0 100X172	6.54	25.71	6.05	18.79	110
	U310	16	2%16	87/8	2	53/4	_	14-10d	6-10dx1½	1345	4355	1235	3090	Low
x12										5.98	19.37	5.49	13.75	
	HU312/ HUC312	14	2%16	10%	2½	101/4	—	16-16d	6-10dx1½	1470	5780 25.71	1360	4225	+11
	1100012									6.54 835	2020	6.05 590	18.79 1435	
	LUS44	18	3%16	3	2	123/32	—	4-16d	2-16d	3.71	8.99	2.62	6.38	Low
										480	1340	445	1030	
4x4	U44	16	3%16	27/8	2	111/16	—	4-10d	2-10d	2.14	5.96	1.98	4.58	+34
	HU44/	4.4	00/	07/	01/	01/		4.40.1	0.40.1	525	1710	490	1585	40
	HUC44	14	3%16	27/8	2½	2½	—	4-16d	2-10d	2.34	7.61	2.18	7.05	+18
	LUS46	18	3%16	43/4	2	313/16		4-16d	4-16d	1720	2595	1545	1920	Low
	L0040	10	J / 16	7 /4		J /16		4-100	4-100	7.65	11.54	6.87	8.54	LUV
	U46	16	3%16	47/8	2	313/16	_	8-10d	4-10d	960	2675	890	2475	+40
		1.0				-				4.27	11.90	3.96	11.01	
1x6	HUS46	14	3%16	5	2	35/8	—	4-16d	4-16d	1745 7.76	2845 12.66	1240 5.52	2570	+17
										1055	3420	980	11.43 2845	
	HU46/						Min	8-16d	4-10d	4.69	15.21	4.36	12.66	+18
	HUC46	14	3%16	5¾16	2½	413/16	<u> </u>			1580	4415	1470	3135	
							Max	12-16d	6-10d	7.03	19.64	6.54	13.95	+19
	LUS46	18	3%16	43/4	2	313/16	_	4-16d	4-16d	1720	2595	1545	1920	Lov
	LU340	10	3 / 16	474		J 716		4-10u	4-100	7.65	11.54	6.87	8.54	LOV
	LUS48	18	3%16	63/4	2	313/16	_	6-16d	4-16d	1720	3325	1545	2575	+29
,						0 7.10		0 .00		7.65	14.79	6.87	11.45	
	U46	16	3%16	47/8	2	313/16	l —	8-10d	4-10d	960	2675	890	2475	+38
4x8										4.27 2540	11.90 3620	3.96 1805	11.01 2570	
	HUS48	14	3%16	615/16	2	53/4	—	6-16d	6-16d	11.30	16.10	8.03	11.43	+19
}										1055	4270	980	3135	
	HU48/	, .	067	046.1	0	0=1	Min	10-16d	4-10d	4.69	18.99	4.36	13.95	+29
	HUC48	14	3%16	613/16	2½	67/16	N.4	14 10 1	6.40.1	1580	5780	1470	4225	0.5
							Max	14-16d	6-10d	7.03	25.71	6.54	18.79	+25
	LUS48	18	3%16	6¾	2	313/16		6-16d	4-16d	1720	3325	1545	2575	Low
ļ	L0040	10	J/16	0 /4		J /16		0-10u	- -100	7.65	14.79	6.87	11.45	LUV
	LUS410	18	3%16	8¾	2	527/32	_	8-16d	6-16d	2580	4500	2320	3195	+2
}		+-			_	- 702				11.48	20.02	10.32	14.21	
	U410	16	3%16	8%	2	61/4	—	14-10d	6-10d	1440	4355	1340	3090	+72
x10						-	-			6.41 3795	19.37 5690	5.96 3450	13.75 4570	
	HUS410	14	3%16	815/16	2	73/4	—	8-16d	8-16d	16.88	25.31	15.35	20.33	+19
						1	1	I	I	10.00	20.01	10.00	20.00	1

1580

7.03

2635

11.72

Min

Max

81/4

14-16d

18-16d

6-10d

10-10d

5780

25.71

5780

25.71

1470

6.54

2450

10.90

4225

18.79

4690

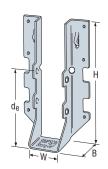
20.86

+228%

+239%



These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.



See footnotes on page 79.

HU410/ HUC410

14 3%16

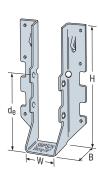
85/8

21/2



These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

LUS414																
Mode Size Mode Size Mode Size Mode Size Mode Mode Size Mode						Dimer	nsions			Faston	ore		Factored F	Resistance		
No. Case No. Case No. Case No. Case No. Case No. Case No. Case No. Case No. Case No. N						(i	n)			I asten	GIS		·· -	-		Inctalled
No. No.				Ga										- 1		
LUS-410 18 3% 8% 2 5% 8-18d 6-16d 2-580 4500 2320 3195 Lovest 1.48 2.002 10.32 14.21 1.28 1.		Size	NO.		W	Н	В	de ⁶		Header	Joist	1 - 7	(/	(- ,	(/	Index
Name									IVIAX							
LUS410 18 39% 89% 2 59% 8-16d 6-16d 114.8 2002 10.32 14.21 Lowest LUS414 18 39% 10% 2 59% 10-16d 6-16d 114.8 2002 10.32 14.21 14.1 14.1 14.1 15% 10% 2 59% 10-16d 6-16d 114.8 2002 10.32 14.21 14.1 14.1 14.1 15% 10% 2 59% 10-16d 6-16d 114.8 23.82 10.32 17.24 14.2 14.4 39% 87% 2 7% 8-16d 8-16d 6-16d 13.87 5.56 13.75 5.56 13.75 5.56 13.75 5.56 13.75 5.56 13.75 5.56 13.75 5.56 13.75 5.56 13.75 5.56 13.75 5.56 13.75 5.56 13.75 5.56 13.75 5.55 13.75 5.56 13.75									CAL	A/AL L LIMID	ED CIZEC	KIN	KN	KIV	KIN	
LUS-414		I							SAI	WIN LUIVID	EN SIZES	0500	4500	0000	0405	
LUS414			LUS410	18	3%16	83/4	2	527/32	—	8-16d	6-16d					Lowest
LISSIA 18 37% 1094 2 39% 101-108 5-1-08 11-148 23.82 10.32 17.24 42.5% LISSIA 18 37% 8% 2 674 - 14-10d 6-1-0d 6-1-1 14-140 4355 13-03 3090 447% LISSIA 18 37% 87% 2 77% - 8-1-0d 6-1-0d 6-11 19.37 5-96 13.75 13-07 3090 447% LISSIA 14 37% 107% 2 97% - 10-1-0d 10-1-0d 16-1-0d			1110444	40	00/	100/		F0/		40.40.1	0.401					000/
HUS410			LUS414	18	3%16	10%	2	5%11	_	10-160	6-160	11.48	23.82	10.32	17.24	+28%
Huself H			U410	16	3%16	83%	2	61/4	_	14-10d	6-10d					+47%
HUS412																
Huself H		4x12	HUS410	14	3%16	815/16	2	7¾	—	8-16d	8-16d					+142%
Hu Hu Hu Hu Hu Hu Hu Hu			LILIC A10	11	29/	101/	2	03/		10 164	10 164					.15/10/
HUG412			1103412	14	3716	1072		374		10-100	10-100					+134 /0
HUG102			11114407						Min	16-16d	6-10d					+208%
	1			14	3%16	105/16	2½	915/16								
Book Book									Max	22-16d	10-10d					+220%
Hu66/ HuC66	Ī		1166	16	51%	5	2	15%		8-10d	4-10d					Lowest
Huse Huse				10	J /2	3		7/0		0 100	4 100					LOWUST
HUG66		6x6	HIIEE/						Min	8-16d	4-16d					+37%
Wax 12-10d 6-10d 8-18d 19-10d				14	5½	43/16	2½	313/16	 -							4.107
Hu68/ HuC68									Max	12-160	6-160	8.18	19.64	7.61	13.95	+41%
Hu68/ Hu68			U66	16	5½	5	2	45/8	_	8-10d	4-10d					Lowest
Huese/Hucse																
HUG68		6x8	H1168/						Min	10-16d	4-16d					+41%
Bank Bank				14	5½	513/16	2½	5/16	May	14-164	6-16d					. 1/10/.
Hu610/ HuC610									IVIAX	14-10u	0-10u					+4470
Hu610/ Hu6610			U610	16	5½	81/2	2	715/16	_	14-10d	6-10d					Lowest
Hu610/ Hu6610 Hu610/ Hu6610 Hu610/ Hu6610 Hu6610																
HUC610		6x10	HU610/	1/1	514	754	214	71/.	Min	14-16d	6-16d					+53%
Hu612/ HuC612			HUC610	14	3 72	1 78	Z 7/2	1 74	Max	18-16d	8-16d					+56%
Hu612/ HuC612																
Hu6612			HU612/					_	Min	16-16d	6-16d					*
No. 10.92 25.71 10.14 20.86		6x12		14	5½	9%	2½	9	May	22-164	Q_16d					*
Hu614/ HuC614									IVIAX	22-10u	0-10u					
Name			1111044/						Min	18-16d	8-16d					*
Max 24-16d 16.39 31.25 15.21 27.51 27.51		6x14		14	5½	11%	2½	1111/4								
HU810/ HUC812 HU812/ HUC812 HU814/ HUC814 HU816/ HUC816 HU816/ HU81									Max	24-16d	12-16d					*
HU810/ HUC810 HU812/ HUC812 HU812/ HUC812 HU814/ HUC814 HU814/ HUC816 HU816/ H									Min	20-16d	8-16d					*
Nax 26-16d 12-16d 16.39 31.25 15.21 27.51 31.35 31.2		6x16		14	5½	1211/16	2½	125/16								
HU88/ HU688 14 7½ 6% 2½ 6¼ Min 10-16d 4-16d 1230 4270 1140 3135 *			1100010						Max	26-16d	12-16d					*
No. No.	Ì								Min	10-164	1-16d	1230	4270	1140	3135	*
HU810		8x8		14	71/2	65%	21/2	61/4	IVIIII	10-100	4-100					
8x10 HU810/ HUC810			HUU88						Max	14-16d	6-16d					*
8x10 HU810/ HUC810 14 7½ 8¾ 2½ 8 Min 14-16d 6-16d 8.18 25.71 7.61 18.79 8x12 HU812/ HUC812 14 7½ 10½ 2½ 9¾ Min 16-16d 6-16d 8.18 25.71 7.61 18.79 8x14 HU814/ HUC814 14 7½ 11⅓ 2½ 11½ Max 24-16d 12-16d 10.92 25.71 10.14 20.86 8x16 HU816/ HUC816 14 7½ 13⅓ 2½ 13⅓ 2½ 13⅓ Min 20-16d 8-16d 12-16d 3685 7025 3420 6185 * 8x16 HU816/ HUC816 14 7½ 13⅓ 2½ 13⅓ 2½ 13⅓ Min 20-16d 8-16d 12-16d 3685 7025 3420 6185 *	ŀ									44 40 1	0.401					
Nax 18-16d 8-16d 2455 5780 2280 4690 *		8v10		1/1	71/6	Q3/ ₆	216	Q	IVIII	14-160	6-160	8.18	25.71			•
8x12 HU812/ HUC812 14 7½ 10⅓ 2½ 9¾ Min 16-16d 6-16d 8.18 25.71 7.61 18.79 Max 22-16d 8-16d 10.92 25.71 10.14 20.86 10.92 25.71 10.14 20.86		0.10	HUC810	14	1 /2	078	2 /2	0	Max	18-16d	8-16d					*
8x12 HU812/ HUC812 14 7½ 10½ 2½ 9¾ Min 16-16d 6-16d 8.18 25.71 7.61 18.79 Max 22-16d 8-16d 10.92 25.71 10.14 20.86 Max 24-16d 12-16d 10.92 25.71 10.14 20.86 Max 24-16d 12-16d 3685 7025 3420 6185 * 8x16 HU816/ HUC816 14 7½ 13½ 2½ 13¼ Min 20-16d 8-16d 10.92 25.71 10.14 20.86 Max 24-16d 12-16d 3685 7025 3420 6185 * Min 20-16d 8-16d 10.92 25.71 10.14 20.86 Max 24-16d 12-16d 3685 7025 3420 6185 * Min 20-16d 8-16d 10.92 25.71 10.14 20.86 Max 24-16d 10.92 25.71 10.14 20.86 Max 24-16d 10.92 25.71 10.14 20.86 Max 24-16d 10.92 25.71 10.14 20.86 Max 26-16d 12-16d 3685 7025 3420 6185 *									-							
8X12 HUC812		040	HU812/		71/	1017	017	027	Min	16-16d	6-16d					*
8x14 HU814/ HUC814 14 7½ 11% 2½ 11½ Min 18-16d 8-16d 10.92 25.71 10.14 20.86 10.92 25.71 1		8X12		14	1 1/2	101/8	21/2	9%	May	22-164	8-16d	2455	5780	2280	4690	*
8x14 HU814/ HUC814 14 7½ 11½ 2½ 11½ Min 18-16d 8-16d 10.92 25.71 10.14 20.86 3685 7025 3420 6185 * 8x16 HU816/ HUC816 14 7½ 13% 2½ 13¼ Min 20-16d 8-16d 10.92 25.71 10.14 20.86 3685 7025 3420 6185 * Min 20-16d 8-16d 10.92 25.71 10.14 20.86 3690 * Max 26-16d 12-16d 3685 7025 3420 6185 *									IVIUA	22 10u	0 100					
8X14 HUC814			HI 1814/						Min	18-16d	8-16d					*
Max 24-16d 12-16d 16.39 31.25 15.21 27.5		8x14		14	71/2	111//8	21/2	11½	N.4	04.40.4	10.10.1					*
8x16 HU816/ HUC816 14 7½ 13¾ 2½ 13¼ Min 20-16d 8-16d 10.92 25.71 10.14 20.86 3685 7025 3420 6185 *									iviax	24-16d	12-160	16.39	31.25	15.21	27.51	.,
8x16 HU816/ HUC816 14 7½ 13% 2½ 13¼ 13¼ 13½ 25.16d 12.16d 3685 7025 3420 6185 *			11110407						Min	20-16d	8-16d					*
		8x16		14	71/2	13%	2½	131/4	_							
						L <u> </u>			Max	26-16d	12-16d			15.21	27.51	*

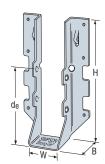


See footnotes on page 79.

FACE MOUNT HANGERS – ROUGH LUMBER



			Dimensions				Fac	teners	Factored Resistance			
				(i	n)		1 00			ir-L		P-F
Joist	Model	Ga							Uplift	Normal	Uplift	Normal
Size	No.		W	н	В	de ⁶	Header	Joist	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
			••			46	1100001	00.01	lbs	lbs	lbs	lbs
									kN	kN	kN	kN
					RC	UGH SA	WN LUMB	ER SIZES				
	LU2 4R-18	18	2	311/16	1½	2½	4-10d	2-10dx1½	415	1020	355	725
2x4 (R)	202 411 10	10		0 710	172	2/2	- 10u	L TOUXT72	1.65	4.54	1.58	3.22
,	U24R	16	2	35/8	2	25/8	4-16d	2-10dx1½	450	1340	355	1030
									2.00 830	5.96 1605	1.58 710	4.58 1140
	LU26R-18	18	2	4%16	1½	313/16	6-10d	4-10dx1½	3.69	7.14	3.16	5.07
2x6 (R)									895	2675	780	2475
	U26R	16	2	5%	2	45/8	8-16d	4-10dx1½	3.98	11.90	3.47	11.01
	11100D 40	10	0	C2/	41/	F5/	0.404	C 40441/	1240	2185	1130	1550
0v0 (D)	LU28R-18	18	2	6%	1½	5%	8-10d	6-10dx1½	5.52	9.72	5.03	6.89
2x8 (R)	U26R	16	2	55%	2	45/8	8-16d	4-10dx1½	895	2675	780	2475
	UZUN	10		378		478	0-10u	4-10ux172	3.98	11.90	3.47	11.01
	LU210R-18	18	2	7%16	2	5%16	10-16d	6-10dx1½	1240	2495	1130	1770
2x10 (R)	202101110	10		1710		0710	10 100	0 100X172	5.52	11.10	5.03	7.87
2/10 (11)	U210R	16	2	91/8	2	71/4	14-16d	6-10dx1½	1345	4355	1235	3090
			_	• • •	_				5.98	19.37	5.49	13.75
2x12 (R)	U210R	16	2	91/8	2	71/4	14-16d	6-10dx1½	1345	4355	1235	3090
. ,									5.98	19.37	5.49	13.75
2x14 (R)	U210R	16	2	91/8	2	71/4	14-16d	6-10dx1½	1345 5.98	4355 19.37	1235 5.49	3090 13.75
									565	1340	520	1030
4x4 (R)	U44R	16	4	25/8	2	111/16	4-16d	2-16d	2.51	5.96	2.31	4.58
					_				1130	3150	1045	2475
4x6 (R)	U46R	16	4	45/8	2	3¾	8-16d	4-16d	5.03	14.01	4.65	11.01
40 (D)	HACD	10	4	45/	0	03/	0.404	4 404	1130	3150	1045	2475
4x8 (R)	U46R	16	4	45/8	2	3¾	8-16d	4-16d	5.03	14.01	4.65	11.01
4x10 (R)	U410R	16	4	81/8	2	61/4	14-16d	6-16d	1695	4355	1495	3090
47.10 (11)	041011	10		078		074	14 100	0 100	7.54	19.37	6.65	13.75
4x12 (R)	U410R	16	4	81/8	2	61/4	14-16d	6-16d	1695	4355	1495	3090
17.12 (11)	0			• 70	_	07.		0.00	7.54	19.37	6.65	13.75
6x6 (R)	U66R	16	6	5	2	313/16	8-16d	4-16d	1130	3150	1045	2475
									5.03	14.01	4.65	11.01
6x8 (R)	U66R	16	6	5	2	313/16	8-16d	4-16d	1130	3150	1045	2475
									5.03 1695	14.01 4355	4.65 1495	11.01 3090
6x10 (R)	U610R	16	6	8½	2	8	14-16d	6-16d	7.54	19.37	6.65	13.75
									1695	4355	1495	3090
6x12 (R)	U610R	16	6	81/2	2	8	14-16d	6-16d	7.54	19.37	6.65	13.75
0.44 (5)	110405	40		0.17			44.40.1	0.401	1695	4355	1495	3090
6x14 (R)	U610R	16	6	81/2	2	8	14-16d	6-16d	7.54	19.37	6.65	13.75



- 1.10d common nails may be used instead of the specified 16d nails at 0.83 of the tabulated value.
- Factored uplift resistances have been increased 15% for earthquake or wind loading with no further increase allowed. Reduce by 15% for standard term loading such as in cantilever construction.
- 3. D.Fir-L factored resistances can be used for most LVL. Verify with manufacturer prior to selecting hanger.
- 4. See page 22 for hangers with reduced capacity due to installation with different nails.
- 5. d_{e} is the distance from the bearing seat to the top joist nail.
- 6. HU rough beam sizes are available by special order. Contact Simpson Strong-Tie for more information.
- 7. NAILS: 16d = 0.162" dia. x $3\frac{1}{2}$ " long, 10d = 0.148" dia. x 3" long, $10dx1\frac{1}{2} = 0.148$ " dia. x $1\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.

^{*} Hangers do not have an Installed Cost Index.

TOP FLANGE HANGERS JB/JBA/LB/LBA/BA/B/HHB Purlin Hangers



This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

The BA hanger is a cost-effective hanger featuring min/max joist nailing option. Min Nailing featuring Positive Angle Nailing targets moderate load conditions whereas the Max Nailing generates capacities for higher loads. The unique two level embossment provides added stiffness to the top flange.

The new, next-generation LBA and JBA hangers provide higher capacities for 2x10 and 2x12 members in 14 gauge and 18 gauge steel, respectively. The new nail locations on the JBA enable effective use with nailers.

See tables on pages 88-91. See Hanger Options on page 231 for hanger modifications, which may result in reduced resistances.

MATERIAL: See tables, pages 88-91.

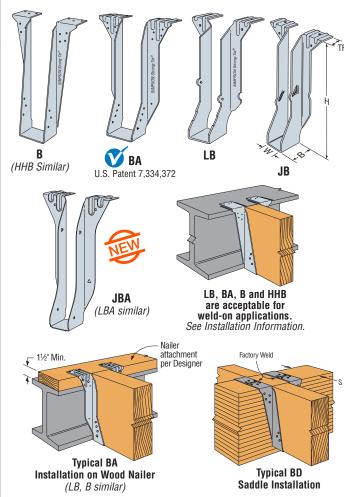
FINISH: JB, LB, B and BA—Galvanized; HHB-all saddle hangers and all welded sloped and special hangers—Simpson Strong-Tie® gray paint. LB, BA, B and HHB may be ordered hot-dip galvanized; specify HDG.

INSTALLATION: • Use specified fasteners. See General Notes and nailer table.

- LB and B may be used for weld-on applications. The minimum required weld to the top flanges is ½" x 2" (½" x 1½" for LB) fillet weld to each side of each top flange tab for 14 and 12 gauge and ½6" x 2" fillet weld to each side of each top flange tab for 7 and 10 gauge. Distribute the weld equally on both top flanges. Welding cancels the top and face nailing requirements. Consult the code for special considerations when welding galvanized steel. The area should be well-ventilated. Weld-on applications produce the maximum factored down resistance listed. Uplift resistances do not apply to welded applications. (Contact Simpson Strong-Tie for uplift information.)
- Ledgers must be evaluated for each application separately.
 Check TF dimension, nail length and nail location on ledger.

OPTIONS: • B and HHB

- Other widths are available; specify W dimension (the minimum W dimension is 1% of B and 25% for HHB).
- Saddle hangers are made to order; add "D" to model (e.g. HHBD412); specify S (for saddle) dimension. They may be used for most conditions except at end wall locations, and are preferred for nailer applications.
- B dimensions may be increased on some models.
- See Hanger Options, page 231.



NAILER TABLE

This table also applies to sloped-seat hangers.

				Resistance 1.00)
Model No.	Nailer	Header Fasteners	D.Fir-L	S-P-F
NU.		I dolellelo	lbs	lbs
			kN	kN
LB/JB	2x	4-10dx1½	1420	855
LD/JD	_ ZX	4-10ux172	6.32	3.80
JBA	2x	6-10dx1½	1785	1395
JDA	ZX	0-10UX172	7.94	6.21
LBA	2x	6-10dx1½	1835	1545
LDA	_ ZX	0-10ux172	8.16	6.87
	2x	10-10dx1½	3220	2870
	2X	10-10ux 1 /2	14.32	12.77
	2-2x	14-10d	3915	3660
BA	Z-2X	14-100	17.41	16.28
DA	3x	14-16dx21/2	4055	_
) JX	14-10UXZ72	18.04	_
	4x	14-16d	4055	_
	4X	14-10u	18.04	_
	2x	10-10dx1½	2835	2340
	_ ZX	10-100X172	12.63	10.42
	2-2x	14-10d	3915	3660
D	Z-ZX	14-100	17.41	16.28
В	3x	14-16dx2½	4055	_
	υX	14-10UXZ72	18.04	
	4x	14-16d	4055	/\A/
	4X	14-10u	18.04	/ VAV

B SERIES WITH VARIOUS HEADER APPLICATIONS

		Fastene	ers		Fact	tored Resist	ance	
				Uplift ¹		Normal (K _D = 1.00)	
Model Series	Top	Face	Joist	$(K_D = 1.15)$	D.Fir-L	S-P-F	LVL	PSL
001100	тор	race	Juist	lbs	lbs	lbs	lbs	lbs
				kN	kN	kN	kN	kN
	6-10d	10-10d	2-10dx1½	435	4470	3975	4695	5385
BA	0-10u	10-100		1.94	19.88	17.68	20.91	23.95
(Min)	6-16d	10-16d	2-10dx1½	435	4990	4370	5835	5385
	0-10u	10-100	2-100X1½	1.94	22.23	19.44	25.99	23.95
	6-10d	10-10d	8-10dx1½	1960	5265	4035	5825	5945
BA	0-10u	10-100	0-10ux172	8.72	23.42	17.95	25.91	26.44
(Max)	6-16d	10-16d	8-10dx1½	1960	5940	4370	6490	7075
	0-10u	10-100	0-10ux172	8.72	26.42	19.44	28.87	31.47
	6-10d	8-10d	6-10dv11/	1650	5265	3590	5825	5230
В	0-10u	0-100	6-10dx1½	7.34	23.42	15.97	25.91	23.26
ט	6-16d	8-16d	6-16dx2½	1650	5940	3910	6490	5230
	6-16d 8-16	0-10u	U-10UXZ/2	7.34	26.46	17.39	28.87	23.26

- 1. Factored uplift resistances shown are for D.Fir-L. Multiply tabulated loads x 0.71 for either SPF joist or header.
- Factored resistances shown are for header connection only. The Designer must ensure the joist is capable of generating the factored resistances shown.
- Applies to LVL headers made primarily from Douglas Fir or Southern Pine. For LVL made primarily from Spruce-Pine-Fir or similar less dense veneers, use the values found in the SPF column.
- 4. NAILS: 16d = 0.162" dia. x $3\frac{1}{2}$ " long, 10d = 0.148" dia. x 3" long, $10dx1\frac{1}{2} = 0.148$ " dia. x $1\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.

TOP FLANGE HANGERS W/WPU/WNP/WM/WMU/HW/HWU/GLT/HGLT

SIMPSON
Strong-Tie

The W, WPU, HWU and HW series purlin hangers offer the greatest design flexibility and versatility. WMs are designed for use on standard 8" grouted masonry block wall construction.

MATERIAL: See tables on pages 88-91; W—12 ga. top flange and stirrup; WM, WMU—12 ga. top flange and stirrup; WNP, WP, WPU—7 ga. top flange, 12 ga. stirrup; HW—3 ga. top flange, 11 ga. stirrup; HWU—3 ga. top flange, 10 ga, stirrup.

FINISH: Simpson Strong-Tie® gray paint; hot-dipped galvanized available: specify HDG, contact Simpson Strong-Tie.

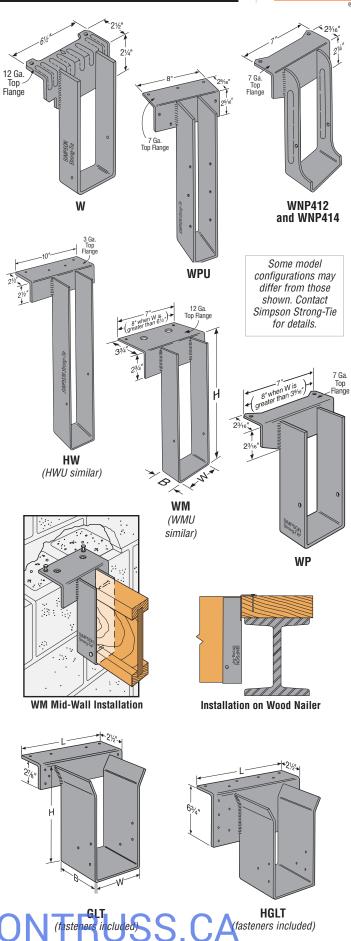
FACTORED RESISTANCES: For hanger heights exceeding the joist height, the factored resistance is 0.50 of the tabulated resistance.

- INSTALLATION: Use all specified fasteners. WM/WMU—two 16d duplex nails must be installed into the top flange and embedded into the grouted wall for mid-wall applications. Verify that the grouted wall can take the required fasteners specified in the table.
 - Hangers may be welded to steel headers with weld size to match material thickness (approximate thickness shown) ½" for W, ¾6" for WNP/WPU and ¾" for HW/HWU, by 1½" fillet welds located at each end of the top flange. Weld-on applications produce maximum factored resistance listed. Uplift resistances do not apply to this application. (Contact Simpson Strong-Tie for uplift information.)
 - Hangers can support multi-ply carried members; the individual members must be secured together to work as a single unit before installation into the hanger.
 - H dimensions are sized to account for normal joist shrinkage.
 W dimensions are for dressed timber widths.
 - Embed WM into block with a minimum of one course above and one course below the top flange with one 15M vertical rebar minimum 24" long in each cell. Minimum grout strength is 15 MPa.
 - See pages 103-104 for GLT and HGLT information.
 - See Hanger Options, page 231 for hanger modifications and associated load reductions.

NAILER TABLE

The table indicates the maximum factored normal resistances for W, WNP and HW hangers used on wood nailers. Nailers are wood members attached to the top of a steel I-beam, concrete or masonry wall. This table also applies to sloped-seat hangers.

		Top	Fac	tored Resista (K _D = 1.00)	ince
Model	Nailer	Flange	D.Fir-L	S-P-F	LSL
		Nailing	lbs	lbs	lbs
			kN	kN	kN
	0	0.40-1-41/	2470	2470	_
	2x	2-10dx1½	11.00	11.00	_
	2-2x	2-10d	2730	2730	_
W	Z-ZX	2-100	12.14	10.61	_
VV	3x	2-16dx2½	2895	2855	_
	JX.	Z-10UXZ72	12.88	12.70	_
	4x	2-10d	3025	2855	_
	44	Z-10u	13.46	12.70	_
	2x	2-10dx11/2	3665	3630	4900
	۷۸	Z-10UX172	16.30	16.15	21.82
	2-2x	2-10d	4475	3760	_
WP/	Z-ZX	2-10u	19.91	16.75	_
WNP	3x	2-16dx2½	4110	3760	_
	JX.	Z-10UXZ/2	18.28	16.75	_
	4x	2-10d	4475	3760	_
	4.4	2-10u	19.91	16.75	_
	2-2x	7-10d	4475	3760	_
	Z-ZX	7-10u	19.91	16.75	_
WPU/	3x	7-16dx2½	4110	3760	_
WNPU	JX	7-10ux2/2	18.28	16.75	_
	4x	7-10d	4475	3760	_
	48	7-10u	19.91	16.75	_
	2-2x	4-10d	7600	_	_
	Z-ZX	4-10u	33.81	_	_
HW	3x	4-16dx2½	7600	_	_
IIVV	JX.	4-10ux272	33.81	_	_
	4x	4-16d	7670	_	_
	47	4-100	34.16	_	_
	2-2x	8-16dx2½	7880	_	_
		0 100XZ/2	35.05	_	_
HWU	3x	8-16dx2½	7880	_	
11000		O TOUXZ /2	35.05		
	4x	8-16d	7880	\ \ 	
	4^	0-10u	35.05	VV	



TOP FLANGE HANGERS W/WPU/WNP/WM/WMU/HW/HWU/GLT/HGLT



W SERIES WITH VARIOUS HEADER APPLICATIONS

	Jo	ist		Fasteners				Fact	ored Resista	ince		
						Uplift1			Normal (K _D = 1.00)		
Model No.	Width	Donth	Ton	Гоос	laiat	(K _D = 1.15)	D.Fir-L	S-P-F	LVL	PSL	LSL	Masonry
	Width	Depth	Тор	Face	Joist	lbs	lbs	lbs	lbs	lbs	lbs	lbs
						kN	kN	kN	kN	kN	kN	kN
	11/ to 4	01/ to 00	0.104v11/		0.104v11/	_	2455	2375	2675	2850	_	_
	1½ to 4	3½ to 30	2-10dx1½	_	2-10dx1½	_	10.92	10.56	11.90	12.68	_	_
W	1½ to 4	3½ to 30	2-10d		2-10dx1½	_	2920	2375	3425	3305	_	_
VV	172 10 4	372 10 30	Z-10u		Z-100X172	_	12.99	10.56	15.24	14.70	_	_
	1½ to 4	3½ to 30	2-16d	_	2-10dx1½	_	2955	2375	3820	3190	_	_
	172 10 1	072 10 00	2 100		L TOUXT72		13.15	10.56	16.99	14.19	_	_
	1½ to 7½	3½ to 30	2-16d DPLX	_	2-10dx1½	_		MID-W	ALL INSTAL	LATION		6060
WM						_						26.96
	1½ to 7½	3½ to 30	2-1/4x13/4 Titen	_	2-10dx1½	_		TOP OF V	NALL INSTA	LLATION		5300
						_						23.58
	1½ to 7½	9 to 28	2-16d DPLX	4-1/4x13/4 Titen	6-10dx1½	860		MID-W	ALL INSTAL	LATION		6060
WMU						3.83						26.96
	1½ to 7½	9 to 28	2-1/4x13/4 Titen	4-1/4x13/4 Titen	6-10dx1½	745		TOP OF V	NALL INSTA	LLATION		5300
						3.31	4005	20.45	4005	4700		23.58
WP/	1½ to 7½	3½ to 30	3-10dx1½	_	2-10dx1½		4095	3345	4695	4720	_	_
							18.22 4095	14.88 3550	20.89 3665	21.00 4720	5980	_
WP/ WNP	1½ to 7½	3½ to 30	3-10d	_	2-10dx1½	_	18.22	15.79	16.30	21.00	26.60	
							4430	3855	5950	5430	5980	_
	1½ to 7½	3½ to 30	3-16d	_	2-10dx1½		19.71	17.15	26.47	24.15	26.60	_
						1665	6390	6390	6825	7085	5980	_
WPU/	1¾ to 5½	7¼ to 18	3-16d	4-16d	6-10dx1½	7.41	28.43	28.43	30.36	31.52	26.60	_
WNPU						595	6390	6390	6825	7085	5980	_
	1¾ to 5½	18½ to 28	3-16d	4-16d	6-10dx1½	2.65	28.43	28.43	30.36	31.52	26.60	_
						_	6900	5285	4695	5810	_	_
	1½ to 7½	3½ to 32	4-10d	_	2-10dx1½	_	30.69	23.51	20.89	25.85	_	_
HW	4471 747	04/1 00	4.40.1		0.401.44/	_	6900	5285	7695	5810	6870	_
	1½ to 7½	3½ to 32	4-16d	_	2-10dx1½	_	30.69	23.51	34.23	25.85	30.56	_
	12/ to 01/	0 += 10	4 404	4 404	0.40441/	1775	10170	8875	10170	8325	8925	_
	1¾ to 3½	9 to 18	4-16d	4-16d	6-10dx1½	7.90	45.24	39.48	45.24	37.03	39.70	_
	13/ to 21/	18½ to 28	4-16d	4-16d	6-10dx1½	1490	10170	8875	10170	8325	8925	_
	194 10 372	10 /2 10 20	4-100	4-10u	0-100X172	6.63	45.24	39.48	45.24	37.03	39.70	_
	13/, to 31/6	28½ to 32	4-16d	4-16d	8-10dx1½	1520	10170	8875	10170	8325	8925	_
HWU	174 (U 372	20/2 10 32	4-10u	4 -100	O-TOUX 172	6.76	45.24	39.48	45.24	37.03	39.70	_
11VV U	4½ to 7½	9 to 18	4-16d	4-16d	6-10dx1½	1775	8250	8250	8250	8250	8250	_
	1/2 (0 1 /8	3 10 10	7 100	7 100	O TOURT /2	7.90	36.70	36.70	36.70	36.70	36.70	_
	4½ to 7½	18½ to 28	4-16d	4-16d	6-10dx1½	1490	8250	8250	8250	8250	8250	_
	.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	.5,2 10 20			0.00//1/2	6.63	36.70	36.70	36.70	36.70	36.70	_
	4½ to 7½	28½ to 32	4-16d	4-16d	8-10dx1½	1520	8250	8250	8250	8250	8250	_
	.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,					6.76	36.70	36.70	36.70	36.70	36.70	_

- 1. Factored uplift resistances shown are for D.Fir-L. Multiply tablulated values x 0.71 for either SPF joist or header.
- Factored resistances shown are for header connection only. The Designer must ensure the joist is capable of generating the factored resistances shown.
- WMU, WPU and HWU factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase allowed. Reduce by 15% for standard term loading like cantilever construction.
- 4. Titen® 1/4x13/4 installed on top of wall after grout has cured.
- 5. NAILS: 16d = 0.162" dia. x $3\frac{1}{2}$ " long, 10d = 0.148" dia. x 3" long, $10dx1\frac{1}{2} = 0.148$ " dia. x $1\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.

HUSTF Heavy Duty and Double Shear Joist Hangers

See dimensions, material, capacities on table pages. HUSTF has the double shear nailing advantage – distributing the joist load through two points on each nail for greater strength.

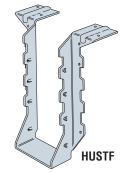
FINISH: Galvanized. Some products available with ZMAX® coating. See Corrosion Information, pages 14-17.

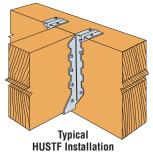
INSTALLATION:

- Use all specified fasteners. See General Notes.
- Not acceptable for nailer or welded applications; see W and B hangers.
- HUSTF—With 3x carrying members, use 16dx2½" nails into the header and 16d commons into the joist.

OPTIONS

 Available with flanges turned in (2-2x and 4x only for HUSCTF).







SIMPSON

Nailer application is NOT acceptable. Fasteners cannot be installed

Some model configurations may differ from those shown. Production models have projected seats. Square cut seats may be ordered. Contact Simpson Strong-Tie for details.



TOP FLANGE HANGERS – SOLID SAWN LUMBER

					sions		Faste	nare		Factored F			
				(i	n)	,	1 4510			ir-L		P-F	Installed
Joist	Model	Ga							Uplift	Normal	Uplift	Normal	Cost
Size	No.	uu	w	н	В	TF	Header	Joist	$(K_D = 1.15)$,	$(K_D = 1.15)$	$(K_D = 1.00)$	Index
			••	"			Houder	00101	lbs	lbs	lbs	lbs	
									kN	kN	kN	kN	
	JB26	18	1%	5%	1½	15/16	4-10d	2-PRONG		1555	_	1385	Lowest
	0020	10	1 / 16	J /8	1 / 2	1 / 16	4-10u	Z-I HONG	_	6.92	_	6.16	LUWEST
	LB26	14	1%6	5%	1½	1½	4-16d	2-10dx1½	490	2080	455	1405	+79%
2x6	LDZU	14	1716	J /8	1 / 2	1 /2	4-100	Z-10UX172	2.18	9.25	2.02	6.26	T1 3 /0
2.00	W26	12	1%	5%	2½	2½	2-10d	2-10dx1½		2920	_	2375	+710%
	VV20	12	1716	J /8	2 /2	2 /2	Z-10u	Z-100X172	_	13.00	_	10.58	T/ 10 /0
	WM26	12	1%	5%	41/2	33/4	2-16d DPLX	2-10dx1½		5995		4600	*
	VVIVIZO	12	1716	J /8	4/2	J 74	2-100 DI LX	Z-10UX172	_	26.67	_	20.46	
	HUS26-2TF	14	31/8	5%	2	13/4	6-16d	4-16d	1745	5130	1240	3645	Lowest
	110020 211	17	070	J / 0		1 /4	0 100	4 100	7.76	22.82	5.51	16.21	LOWGST
DBL	WNP26-2	12	31/8	53%	2½	23/16	2-10d	2-10d		4095		3550	+31%
2x6	VVIVI 20-2	12	J /8	J /8	2 /2	2/16	Z-10u	2-10u	_	18.24	_	15.81	TO 1 /0
	WM26-2	12	31/8	5%	2½	3/34	2-16d DPLX	2-10d		6060		5065	*
	VV IVIZO-Z	12	J /8	J /8	2 /2	3/34	2-100 DI LX	2-10u		26.96	_	22.53	
	JB28	18	1%	71/4	1½	15/16	4-10d	2 PRONG		1555	_	1385	Lowest
	0020	10	1710	1 /4	172	1 / 10	4 10u	21110110	_	6.92	_	6.16	LOWGST
	LB28	14	1%	71/4	1½	1½	4-16d	2-10dx1½	490	2080	455	1405	+69%
2x8	LDZO	' -	1710	1 /4	172	172	4 10u	2 10UX172	2.18	9.25	2.02	6.26	+0370
2.00	W28	12	1%6	71/8	2½	2½	2-10d	2-10dx1½		2895	_	2385	+541%
	WZ0	12	1710	1 /0	L /2	L /2	2 100	2 100X172	_	12.88	_	10.61	TJT1/0
	WM28	12	1%	71/8	41/2	33/4	2-16d DPLX	2-10dx1½		5995	_	4600	*
	WWIZO	12	1710	1 70	7/2	074	Z TOU DI EX	Z TOUXT72	_	26.67	_	20.46	
	HUS28-2TF	14	31/8	71/4	2	11//8	8-16d	6-16d	2540	6825	1805	4480	Lowest
	110020 211		0,0	1,74		170	0.100		11.30	26.91	8.03	19.93	Lowoot
DBL	WNP28-2	12	31/8	71/8	21/2	23/16	2-10d	2-10d		4095		3550	+30%
2x8	****** 20 2		0,0	170		2710	2 100	2 100		18.22	_	15.81	10070
	WM28-2	12	31/8	71/8	2½	33/4	2-16d DPLX	2-10d		6060	_	5065	*
	20 2		0,0	.,,	-/-	071	2 100 31 27		_	26.96	_	22.53	
	JB210A	18	1%16	93/16	2	17/16	6-16d	2-10dx1½	415	2430	370	1725	Lowest
	052.0.1		.,,,			.,,,	0 .00	2 1000172	1.85	10.81	1.65	7.67	20001
_	LB210A	14	1%16	93/16	2	17/16	6-16d	2-10dx1½	490	2710	490	1935	+28%
2x10		ļ.,	.,	2 7.3		.,	J	00,,2	2.18	12.06	2.18	8.61	
	W210	12	1%16	91/8	2½	2½	2-10d	2-10dx1½		2920	_	2375	+327%
			.,,,						_	13.00	_	10.58	
	WM210	12	1%16	91/8	41/2	33/4	2-16d DPLX	2-10dx1½		5995		4600	*
									_	26.67	_	20.46	



^{2.} **NAILS:** $16d = 0.162^{\circ}$ dia. x $3\frac{1}{2}^{\circ}$ long, $10d = 0.148^{\circ}$ dia. x 3° long, $10dx1\frac{1}{2} = 0.148^{\circ}$ dia. x $1\frac{1}{2}^{\circ}$ long. See pages 22-23 for other nail sizes and information.



Factored uplift resistances have been increased 15% for earthquake or wind loading with no further increase allowed. Reduce by 15% for standard term loading such as in cantilever construction.

TOP FLANGE HANGERS – SOLID SAWN LUMBER



					nsions n)		Faste	ners	D.E	Factored F ir-L	Resistance	P-F	
	Model			(1	'' <i>'</i>				Uplift	Normal	Uplift	Normal	Installed
Joist	No.	Ga			_		Hander	laint		$(K_D = 1.00)$	-	$(K_D = 1.00)$	Cost Index
			W	Н	В	TF	Header	Joist	lbs	lbs	lbs	lbs	IIIdox
									kN	kN	kN	kN	
	HUS210-2TF	14	31/8	91/4	2	11/2	10-16d	8-16d	3795	6755	3450	5435	Lowest
DBL									16.88	30.05 4095	15.35	24.18 3550	
2x10	WNP210-2	12	31//8	91/8	2½	2¾16	2-10d	2-10d	_	18.22	_	15.79	+ 32%
	WM210-2	12	31/8	91/8	2½	33/4	2-16d DPLX	2-10d		6060	_	5065	*
	WINIZIO Z	12	078	378	L /2	074	Z TOU DI EX	2 100	-	26.96	_	22.53	
	JB212A	18	1%16	111/8	2	1 ½16	6-16d	2-10dx1½	415 1.85	2430 10.81	370 1.65	1725 7.67	Lowest
	1 00101		407	4417		47/	0.40.1	0.401.41/	490	2710	455	1935	000/
2x12	LB212A	14	1%16	111/8	2	17/16	6-16d	2-10dx1½	2.18	12.06	2.02	8.61	+ 29%
2 1 1 2	W212	12	1%16	11	2½	2½	2-10d	2-10dx1½		2920	_	2375	+ 331%
			.,,,				2 .00	2 100/11/2		12.99 5995	_	10.56 4600	
	WM212	12	1%16	11	41/2	3¾	2-16d DPLX	2-10dx1½	_	26.67		20.46	*
	11110040 OTF	4.4	01/	441/	_	01/	40.40.1	0.40.1	3765	6755	2675	5435	1
	HUS212-2TF	14	31/8	111/8	2	21/4	10-16d	8-16d	16.75	30.05	11.90	24.18	Lowest
DBL	WNP212-2	12	31/8	11	2½	23/16	2-10d	2-10d	_	4095	_	3550	+ 20%
2x12							2.00		_	18.22	_	15.79	. 2070
	WM212-2	12	31//8	11	2½	3¾	2-16d DPLX	2-10d		6060 26.96	_	5065 22.53	*
	WOO	40	00/	F2/	0	01/	0.404	0.40-1-41/	_	2920	_	2375	*
3x6	W36	12	2%16	5%	2	2½	2-10d	2-10dx1½	_	12.99	_	10.56	
370	WM36	12	2%16	5%	3	3¾	2-16d DPLX	2-10dx1½		6060	_	5065	*
					-				1650	26.96	1170	22.53	
	B38	12	2%16	71/8	2½	2½	14-16d	6-16dx ² ½	1650 7.34	5940 26.42	1170 5.20	3910 17.39	Lowest
									7.04 —	2920	J.20 —	2375	
3x8	W38	12	29/16	71/8	2	2½	2-10d	2-10dx1½	_	12.99	_	10.56	+ 38%
	WM38	12	2%16	71/8	3	3¾	2-16d DPLX	2-10dx1½		6060		5065	*
	WWW		2710	170		074	E TOU DI EX	L TOUXT72	-	26.96		22.53	
	B310	12	29/16	91/8	21/2	21/2	14-16d	6-16dx ² ½	1650 7.34	5940 26.42	1170 5.20	3910 17.39	Lowest
					_				- 1.5 4	2920	J.20 —	2375	
3x10	W310	12	29/16	91/8	2	2½	2-10d	2-10dx1½	_	12.99	_	10.56	+ 46%
	WM310	12	2%16	91/8	3	3¾	2-16d DPLX	2-10dx1½		6060	_	5065	*
				0,0		071	2 100 51 27	2 100/172		26.96		22.53	
	B312	12	2%16	11	2½	2½	14-16d	6-16dx ² ½	1650 7.34	5940 26.42	1170 5.20	3910 17.39	Lowest
0.40	IMBIDO40	40	201	44	01/	02/	0.40.1	0.401.41/	— —	4095	—	3550	440/
3x12	WNP312	12	2%16	11	2½	23/16	2-10d	2-10dx1½	_	18.22	_	15.79	+ 41%
	WM312	12	2%16	11	3	3¾	2-16d DPLX	2-10dx1½		6060	_	5065	*
					_				1745	26.96 5130	1040	22.53 3645	
	HUS46TF	14	3%16	5%	2	1½	6-16d	4-16d	1745 7.76	22.82	1240 5.52	16.21	Lowest
	WAC	10	09/	F3/	01/	01/	0.404	0.404	_	2920		2375	. 440/
4x6	W46	12	3%16	5%	2½	2½	2-10d	2-10d	_	12.99	_	10.56	+ 41%
1/10	HW46	11	3%16	5%	2½	2½	4-10d	2-10d		7620		4695	+ 177%
										33.90 6060	_	20.89 5380	
	WM46	12	3%16	5%	21/2	3¾	2-16d DPLX	2-10d	_	26.96	_	23.93	*
	BA48 (min)	14	3%16	71/	3	2½	16-16d	2-10dx1½	435	4990	310	4370	Lowoot
	(ווווווו)	14	716 ن	71/8	٥	4 /2	10-100	Z-10UX I //2	1.94	22.20	1.38	19.44	Lowest
	BA48 (max)	14	3%16	71/8	2½	2½	16-16d	8-10dx1½	1960	5940	1565	4370	+ 4%
	` ,								8.72 1650	26.42 5940	6.96 1170	19.44 3910	
	B48	12	3%16	71/8	21/2	21/2	14-16d	6-16d	7.34	26.42	5.20	17.39	+ 71%
4x8	HUS48TF	14	3%16	71/4	2	111/16	8-164	6-164	2540	6285	1805	4480	± 760/.
430	11004017	14	716 ن	1 74		1 . 1/16	8-16d	6-16d	11.30	27.96	8.03	19.93	+ 76%
	W48	12	3%16	71/8	2½	2½	2-10d	2-10d		2920		2375	+ 105%
										12.99 7620	_	10.56 4695	
	HW48	11	3%16	71/8	2½	2½	4-10d	2-10d	_	33.90	_	20.89	+ 300%
	\\/\\/\/\Q	10	29/	71/	91/-	23/	3-164 DDI ∧	2-104		6060	_	5830	*
	WM48	12	3%16	71/8	2½	3¾	2-16d DPLX	2-10d		26.96		25.93	
			1 A		n / 1	Λ			7 /				• A

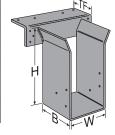


WM48 See footnotes on page 88.

TOP FLANGE HANGERS – SOLID SAWN LUMBER



				Dimer	sions		Fasts			Factored F	Resistance		
				(i	n)		Faste	ilers	D.F	ir-L	S-I	P-F	
Joist	Model	Ga							Uplift	Normal	Uplift	Normal	Installed Cost
JUIST	No.	ua	w	н	В	TF	Header	Joist	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$	Index
			VV	"	ь	"	licauci	Juist	lbs	lbs	lbs	lbs	
									kN	kN	kN	kN	
	DA 440 (:-)	4.4	00/	01/		01/	40.40.1	0.40-3-41/	435	4990	310	4370	1
	BA410 (min)	14	3%16	91/4	3	2½	16-16d	2-10dx1½	1.94	22.20	1.38	19.44	Lowest
									1960	5940	1565	4370	
	BA410 (max)	14	3%16	91/4	3	2½	16-16d	8-10dx1½	8.72	26.42	6.96	19.44	+ 4%
	_								3795	6755	3450	5435	
	HUS410TF	14	3%16	91/4	2	1½	10-16d	8-16d	16.88	30.05	15.35	24.18	+ 59%
									1650	5940	1170	3910	
	B410	12	3%16	91/8	2½	21/2	14-16d	6-16d	7.34	26.42	5.20	17.39	+ 72%
									_	2920	_	2375	
4x10	W410	12	3%16	91/8	21/2	21/2	2-10d	2-10d	_	12.99	_	10.56	+ 91%
									_	7620	_	4695	
	HW410	11	3%16	91/8	21/2	21/2	4-10d	2-10d	_	33.90	_	20.89	+ 270%
				7½					2905	9625	2060	5225	
	GLT4 ¹	7	3%16	Min.	5	21/2	10-N54A	6-N54A	12.92	42.82	9.16	23.24	*
									2905	14885	2060	9830	
	HGLT4 ¹	7	3%16	7½ Min.	6	21/2	18-N54A	6-N54A					*
				IVIIII.					12.92	66.21	9.16	43.73	
	WM410	12	3%16	91/8	21/2	3¾	2-16d DPLX	2-10d		6060		5830	*
									405	26.96		25.93	
	BA412 (min)	14	3%16	11	3	21/2	16-16d	2-10dx1½	435	4990	310	4370	Lowest
	, ,								1.94	22.20	1.38	19.44	
	BA412 (max)	14	3%16	11	3	2½	16-16d	8-10dx1½	1960	5940	1565	4370	+ 3%
	(**)								8.72	26.42	6.96	19.44	
	HUS412TF	14	3%16	1111/8	2	2	10-16d	8-16d	3795	6755	2675	5435	+ 28%
									16.88	30.05	11.90	24.18	
	B412	12	3%16	11	2½	2½	14-16d	6-16d	1650	5940	1170	3910	+ 72%
			07.10					0 .00	7.34	26.42	5.20	17.39	
4x12	WNP412	12	3%16	11	2½	23/16	2-10d	2-10d		4095		3550	+ 100%
			07.10			27.10	2.00			18.22		15.79	
	HW412	11	3%16	11	21/2	2½	4-10d	2-10d		7620	_	4695	+ 248%
	1100412	'''	0710	'''	L /2	L /2	7 100	2 100	_	33.90	_	20.89	1 240 /0
	GLT4 ¹	7	3%16	71/2	5	2½	10-N54A	6-N54A	2905	9625	2060	5225	*
	UL14		J / 16	Min.	J	2 /2	10-11347	0-11347	12.92	42.82	9.16	23.24	
	HGLT4 ¹	7	3%16	71/2	6	2½	18-N54A	6-N54A	2905	14885	2060	9830	*
	TIGE14	'	3 716	Min.	0	2 /2	10-11347	0-11347	12.92	66.21	9.16	43.73	
	WM412	12	3%16	11	21/2	33/4	2-16d DPLX	2-10d	_	6060	_	5830	*
	VVIVI412	12	3 716	11	2 /2	374	2-100 DFLX	2-10u	_	26.96	_	25.93	
	WNP66	12	E1/	5%	01/	25/	2 104	0.104	_	4095	_	3550	Lowoot
	VVINFOO	12	5½	J78	2½	25/16	3-10d	2-10d	_	18.22	_	15.79	Lowest
eve	LIMES	44	E1/	E3/	01/	01/	4 104	0.404	_	6900	_	5285	. E10/
6x6	HW66	11	5½	5%	21/2	2½	4-10d	2-10d	_	30.69	_	23.51	+ 51%
	MANAGO	40	F1/	F3/	01/	03/	0 404 DDI V	0.404	_	6060	_	6060	*
	WM66	12	5½	5%	2½	3¾	2-16d DPLX	2-10d	_	26.96	_	26.96	"
	DCO	10	E1/	71/	01/	01/	14.404	6 101	1650	5940	1170	3910	Louiset
	B68	12	5½	71/8	21/2	2½	14-16d	6-16d	7.34	26.42	5.20	17.39	Lowest
	MAIDOS	40	F1/	71/	017	OF /	0.401	0.40.1	_	4095	_	3550	F00/
0.0	WNP68	12	5½	71/8	2½	25/16	3-10d	2-10d	_	18.22	_	15.79	+ 52%
6x8	LIMICO	,,	F	7. /	C+ /	64.	4.40:	0.40	_	6900	_	5285	40.407
	HW68	11	5½	71/8	2½	2½	4-10d	2-10d	_	30.69	_	23.51	+ 134%
	14/14/05	4.5	F	7	0:1	0-1	0.40.1.55	0.40	_	6060	_	6060	
	WM68	12	5½	71/8	21/2	3¾	2-16d DPLX	2-10d	_	26.96	_	26.96	*
										_0.00		_0.00	



^{1.}N54A fasteners are supplied with hangers. Specify height required. 2. Factored uplift resistances have been increased 15% for earthquake or

wind loading with no further increase allowed. Reduce by 15% for standard term loading such as in cantilever construction.

^{3.} **NAILS:** 16d = 0.162" dia. x $3\frac{1}{2}$ " long, 10d = 0.148" dia. x 3" long, $10dx1\frac{1}{2} = 0.148$ " dia. x $1\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.

^{*} Hangers do not have an Installed Cost Index.

TOP FLANGE HANGERS – SOLID SAWN LUMBER



No. No.					Dimer	nsions		F1-			Factored I	Resistance		
Month Mont								Faste	ners		ir-L	S-I	P-F	
No. W	.lnist		Ga									-		Installed Cost
B610	00101	No.	uu	w	н	В	TF	Header	Joist	` - '	,	,	, ,	Index
Befig									••••					
Bell														
WNP610		B610	12	5½	91/8	21/2	2½	14-16d	6-16d					Lowest
HWF010										7.34		5.20		
HW610		WNP610	12	5½	91/8	2½	25/16	3-10d	2-10d			_		+ 34%
6x10 Hw610														
STILE GLT6 7 55% Min. 5 2½ 10-NS4A 6-NS4A 12-92 42-82 91-6 23-24		HW610	11	5½	91/8	2½	2½	4-10d	2-10d	_		_		+ 125%
Hell Hell	6x10	01.704	_	F 0/	71/2	_	04 (40 NE 44	0.115.44	2905		2060		_
Hollie		GL16'	/	5%16		5	21/2	10-N54A	6-N54A	12.92	42.82	9.16	23.24	Î
WM610		UCI TG1	7	5 9/.	71/2	6	21/	10 NE // A	C NE 4A	2905	14885	2060	9830	*
WM610		писто	′	3 716	Min.	0	2 72	10-N34A	0-N34A	12.92	66.21	9.16	43.73	
B612 12 5½ 11 2½ 2½ 14-16d		WM610	12	51/6	Q1/6	21/6	33/4	2-16d DPL X	2-10d			_		*
HW612		***************************************	1/2	072	378	L /2	074	Z TOU DI EX	2 100					
HW612		B612	12	5½	11	2½	2½	14-16d	6-16d					Lowest
HW612										7.34				
6x12		HW612	11	5½	11	21/2	21/2	4-10d	2-10d					+ 125%
6x12 HHB612 7 5½ 11 3 2½ 10-N54A 6-N54A 14.86 38.12														
GLT6' 7 5% 7% Min. 5 2% 10-N54A 6-N54A 2905 9625 2060 5225 224	6x12	HHB612	7	5½	11	3	21/2	10-N54A	6-N54A					*
Hear Hear					71/6							2060		
HGLT6'		GLT6 ¹	7	5%16		5	2½	10-N54A	6-N54A					*
Bell 12 5½ 13 2½ 2½ 14-16d 6-16d 150 5940 1170 3910 173 18 18 18 18 18 18 18 1			_	=0.4	71/2									
B614 12 5½ 13 2½ 2½ 4-10d 2-10d 6900 5285 9-96 9-9		HGL16'	/	5%16		6	21/2	18-N54A	6-N54A	12.92	66.21	9.16	43.73	*
HW614		DC1/	10	E1/	10	01/	01/	1/164	6 164	1650	5940	1170	3910	Lowoot
HHB614		D014	12	372	13	Z 72	2 72	14-10u	0-10u	7.34	26.42	5.20	17.39	Lowest
State		HW614	11	51/6	13	21/6	21/2	4-10d	2-10d		6900	_		+ 98%
BATE HHBB16		1100014	'''	072	10	2/2	2/2	7 100	2 100			_	23.51	+ 30 /0
State Stat	6x14	HHB614	7	5½	13	3	2½	10-N54A	6-N54A			_		+ 192%
Human Huma		-												
HGLTG1 7 5% Min. 6 2½ 18-N54A 6-N54A 2905 14885 2000 9830 ** B616 12 5½ 15 2½ 2½ 14-16d 6-16d 1650 5940 1170 3910 Low 7.34 26.42 5.20 17.39 Low 6900 — 5285 ** HHB616 7 5½ 15 3 2½ 10-N54A 6-N54A 14.86 38.12 — +17 GLTG1 7 5% Min. 5 2½ 12½ 10-N54A 6-N54A 2905 9625 2060 5225 ** HGLTG1 7 5% Min. 6 2½ 18-N54A 6-N54A 2905 14885 2060 9830 ** HKB86 7 7½ 5% 2½ 2½ 4-10d 2-10d — 6900 — 5285 ** 8x6 HW86 7 7½ 5% 2½ 2½ 4-10d 2-10d — 6900 — 5285 ** 8x8 HW88 7 7½ 5% 2½ 2½ 4-10d 2-10d — 6900 — 5285 ** 8x10 HW810 7 7½ 9% 2½ 2½ 4-10d 2-10d — 6900 — 5285 ** HHB812 7 7½ 11 3 2½ 2½ 4-10d 2-10d — 6900 — 5285 ** HW814 7 7½ 13 3 2½ 10-N54A 6-N54A 3340 8570 — +92 HHB814 7 7½ 13 3 2½ 10-N54A 6-N54A 3340 8570 — 5285 Low 7.50 Min. 6 2.50 Min. 6-N54A 3340 8570 — 5285 Low 7.50 Min. 6-N54A 3340 8570 — 3351 Low 7.50 Min. 6-N54A 3340 8570 — 3351 Low 7.50 Min. 6-N54A 3340 8570 — 3351 Low 7.50 Min. 6-N54A 3340 8570 — 3351 Low 7.50 Min. 6-N54A 3340 8570 — 3351 Low 7.50 Min. 6-N54A 3340 8570 — 3351 Low 7.50 Min. 6-N54A 3340 8570 — 3351 Low 7.50 Min. 6-N54A 3340 8570 — 3		GLT6 ¹	7	5%16		5	2½	10-N54A	6-N54A					*
HGL16 7 5% Min. 6 2½ 18-N54A 6-N54A 12.92 66.21 9.16 43.73 12.92 66.21 9.16 43.73 12.92 66.21 9.16 43.73 12.92 66.21 9.16 43.73 12.92 66.21 9.16 43.73 12.92 66.21 9.16 43.73 12.92 66.21 9.16 43.73 12.92 42.82 17.39 12.92 42.82 12.92 42.82 9.16 23.51 12.92 42.82 9.16 23.24														
B616 12 5½ 15 2½ 2½ 14-16d 6-16d 1650 5940 1170 3910		HGLT6 ¹	7	5%16		6	2½	18-N54A	6-N54A					*
HW616														
HW616		B616	12	5½	15	2½	2½	14-16d	6-16d					Lowest
Color Colo		11111040	44	F1/	45	01/	01/	4.40.1	0.40.4	_		_		000/
6X16 HHB616 7 5½ 15 3 2½ 10-N54A 6-N54A 14.86 38.12 — — +17 GLT6¹ 7 5%6 7½ 5 2½ 10-N54A 6-N54A 2905 9625 2060 5225 * HGLT6¹ 7 5%6 7½ 6 2½ 18-N54A 6-N54A 2905 14885 2060 9830 * 8X6 HW86 7 7½ 5%8 2½ 2½ 4-10d 2-10d — 6900 — 5285 * 8X8 HW88 7 7½ ½ 2½ ½ 4-10d 2-10d — 6900 — 5285 * 8x10 HW810 7 7½ 11 2½ 2½ 4-10d 2-10d — 6900 — 5285 * 8x12 HW812 7 7½ 11 3 2½ 10-N54A 6-N54A		HW616	11	51/2	15	21/2	21/2	4-100	2-100	_	30.69	_	23.51	+ 89%
Second Second	6v16	HHR616	7	516	15	2	216	10-N54A	6-N54A			_	_	+ 174%
HGLT61	0.10	111111111111111111111111111111111111111		J /2	10	J	2 /2	10-11347	0-11347	14.86	38.12	_	_	T 17470
HGLT61		GLT61	7	5%		5	21/2	10-N54A	6-N54A					*
Hole 7 5% Min. 6 2½ 18-N54A 6-N54A 12.92 66.21 9.16 43.73		02.0	ı.	07.10	-			10 110 111						
8x6 HW86 7 7½ 5% 2½ 2½ 4-10d 2-10d — 6900 — 5285 * 8x8 HW88 7 7½ 7½ 2½ 2½ 4-10d 2-10d — 6900 — 5285 * 8x10 HW810 7 7½ 9½ 2½ 2½ 4-10d 2-10d — 6900 — 5285 * 8x12 HW812 7 7½ 11 2½ 2½ 4-10d 2-10d — 6900 — 5285 * 8x12 HW812 7 7½ 11 2½ 2½ 4-10d 2-10d — 6900 — 5285 Low 8x12 HH8812 7 7½ 11 3 2½ 10-N54A 6-N54A 3340 8570 — — +9½ 8x14 HH8814 7 7½ 13 3 2½ 4-10d 2-10d — 6900 — 5285 Low 8x16 HW816		HGLT61	7	5%16		6	2½	18-N54A	6-N54A					*
8X6 HW86 7 7½ 5% 2½ 2½ 4-10d 2-10d — 30.69 — 23.51 ** 8X8 HW88 7 7½ 7½ 2½ 2½ 4-10d 2-10d — 6900 — 5285 * 8X10 HW810 7 7½ 9½ 2½ 2½ 4-10d 2-10d — 6900 — 5285 * 8X12 HW812 7 7½ 11 2½ 2½ 4-10d 2-10d — 6900 — 5285 Low 8X12 HH8812 7 7½ 11 3 2½ 10-N54A 6-N54A 3340 8570 — — + 9½ 8X14 HW814 7 7½ 13 2½ 2½ 4-10d 2-10d — 6900 — 5285 Low 8X14 HH8814 7 7½ 13 3 2½ 4-10d 2-10d — 6900 — 5285 Low 8X16 HW816					IVIIII.							9.16		
8x8 HW88 7 7½ 7½ ½½ ½½ 4-10d 2-10d — 6900 — 5285 * 8x10 HW810 7 7½ 9½ ½½ ½½ 4-10d 2-10d — 6900 — 5285 * 8x12 HW812 7 7½ 11 ½½ ½½ 4-10d 2-10d — 6900 — 5285 Low 8x12 HH8812 7 7½ 11 3 ½½ 10-N54A 6-N54A 3340 8570 — — + 9½ 8x14 HW814 7 7½ 13 3½ ½½ 4-10d 2-10d — 6900 — 5285 Low 8x14 HH8814 7 7½ 13 3 ½½ 4-10d 2-10d — 6900 — 5285 Low 8x16 HW816 7 7½ 15 2½ ½½ 4-10d <td>8x6</td> <td>HW86</td> <td>7</td> <td>71/2</td> <td>5%</td> <td>21/2</td> <td>2½</td> <td>4-10d</td> <td>2-10d</td> <td></td> <td></td> <td></td> <td></td> <td>*</td>	8x6	HW86	7	71/2	5%	21/2	2½	4-10d	2-10d					*
8x8 HW88 7 7½ 1½ 2½ 2½ 4-10d 2-10d — 30.69 — 23.51 * 8x10 HW810 7 7½ 9½ 2½ 2½ 4-10d 2-10d — 6900 — 5285 * HW812 7 7½ 11 2½ 2½ 4-10d 2-10d — 6900 — 5285 Low HHB812 7 7½ 11 3 2½ 10-N54A 6-N54A 3340 8570 — — +9½ HW814 7 7½ 13 2½ 2½ 4-10d 2-10d — 6900 — 5285 Low 8x14 HHB814 7 7½ 13 3 2½ 10-N54A 6-N54A 3340 8570 — — +87 8x16 HW816 7 7½ 15 2½ 2½ 4-10d 2-10d — 6900 </td <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>_</td> <td></td> <td>_</td> <td></td> <td></td>										_		_		
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8x12 HW812 7 7½ 11 2½ 2½ 4-10d 2-10d — 6900 — 5285 Low HHB812 7 7½ 11 3 2½ 10-N54A 6-N54A 3340 8570 — — + 9½ HW814 7 7½ 13 2½ 2½ 4-10d 2-10d — 6900 — 5285 Low HHB814 7 7½ 13 3 2½ 10-N54A 6-N54A 3340 8570 — — + 87 HW816 7 7½ 15 2½ 2½ 4-10d 2-10d — 6900 — 5285 Low 8x16 HHB816 7 7½ 15 3½ 2½ 4-10d 2-10d — 6900 — 5285 Low 8x16 HHB816 7 7½ 15 3 2½ 10-N54A 6-N54A 3340 8570 — — 835	0.40	104040	_	717	01/	01/	01/	4.40.1	0.40.1	_				_
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HHB812 7 7½ 11 3 2½ 10-N54A 6-N54A 3340 8570 — — +9½ HW814 7 7½ 13 2½ 2½ 4-10d 2-10d — 6900 — 5285 — HHB814 7 7½ 13 3 2½ 10-N54A 6-N54A 3340 8570 — — +8% HW816 7 7½ 15 2½ 2½ 4-10d 2-10d — 6900 — 5285 — HHB816 7 7½ 15 3 2½ 10-N54A 6-N54A 3340 8570 — — 8351	8x12	1100012		1 /2	'''	2/2	2/2	7 100	Z 100	_		_	23.51	LOWGST
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8x14 HW814 7 7½ 13 2½ 2½ 4-10d 2-10d — 30.69 — 23.51 Low HHB814 7 7½ 13 3 2½ 10-N54A 6-N54A 3340 8570 — — +87 HW816 7 7½ 15 2½ 2½ 4-10d 2-10d — 6900 — 5285 Low HHB816 7 7½ 15 3 2½ 10-N54A 6-N54A 3340 8570 — 83										14.86		_		
8x14 HHB814 7 7½ 13 3 2½ 10-N54A 6-N54A 3340 8570 — — +87 HW816 7 7½ 15 2½ 2½ 4-10d 2-10d — 6900 — 5285 Low HHB816 7 7½ 15 3 2½ 10-N54A 6-N54A 3340 8570 — 83		HW814	7	7½	13	2½	2½	4-10d	2-10d	_				Lowest
8x16 HW816 7 7½ 13 3 2½ 10-N54A 6-N54A 14.86 38.12 — — +8/2 HW816 7 7½ 15 2½ 2½ 4-10d 2-10d — 6900 — 5285 Low HH8816 7 7½ 15 3 2½ 10-N54A 6-N54A 3340 8570 — 83	8x14				-					3340				
8x16 HW816 7 7½ 15 2½ 2½ 4-10d 2-10d — 6900 — 5285 Low HHB816 7 7½ 15 3 2½ 10-N54A 6-N54A 3340 8570 — — 83		HHB814	7	71/2	13	3	2½	10-N54A	6-N54A					+ 87%
8x16 HH8816 7 7½ 15 2½ 2½ 4-100 2-100 — 30.69 — 23.51 Low										—		_		
HHB816 7 7½ 15 3 2½ 10-N54A 6-N54A 3340 8570 — — 83		HW816	7	7½	15	21/2	2½	4-10d	2-10d	_		_		Lowest
HHB81b	8x16	IIIIDO40	7	71/	15	0	01/	40 N/C 4A	C NE 4A	3340				000/
14.00 30.12		HHR816	1	11/2	15	3	21/2	10-N54A	6-N54A	14.86	38.12	_	_	83%



See footnotes on page 90.

LRUZ Rafter Hanger

The LRUZ offers an economic alternative for those applications requiring a sloped hanger for rafter-to-ridge connections. Used with solid-sawn rafters, the LRUZ's unique design enables the hanger to be installed either before or after the rafter is in place. The field-adjustable seat helps improve job efficiency by eliminating mismatched angles in the field and lead times associated with special orders. The

rafter hangers at a reduced cost while using fewer fasteners. FEATURES:

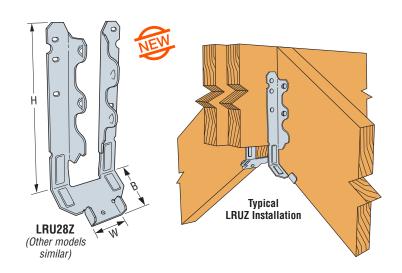
· The open design and ability to field-adjust the slope makes the LRUZ ideal for both retrofit or new applications

LRUZ offers comparable or better load capacity to other

- Accommodates roof pitches from 0:12 to 14:12
- Slopes up or down to 45°. For slopes greater than 45° to 49° (14:12), allowable loads are 0.85 of table loads
- For added versatility, the fasteners on the face of the hanger are placed high enabling the bottom of the rafter to hang below the ridge beam (see "Max. C1" dimension)
- Can be installed using nails or Simpson Strong-Tie® Strong-Drive® SD Connector screws

MATERIAL: 18 gauge FINISH: ZMAX® coating INSTALLATION:

- · Use all specified fasteners. See General Notes.
- · Joist fasteners must be installed at an angle through the rafter or joist into the header to achieve the table loads.
- · See alternate installation on page 86 for retrofit applications.







Dome Double Shear Nailing Side View U.S. Patent 5,603,580

These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

Standard Installation 1,2,3

								Factored F	Resistance	
		Dimens	sions (in.)	Faste	ners ^{4,5}	D.F	ir-L	S-I	P-F
Model							Uplift	Normal	Uplift	Normal
No.							$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
	W	Н	В	Max C ₁	Header	Joist	lbs	lbs	lbs	lbs
				01			kN	kN	kN	kN
					4-10d	5-10d	945	1360	670	965
LRU26Z	15/8	51/4	115/16	13/4	4-100	5-100	4.20	6.05	2.98	4.29
LNUZUZ	178	J 74	1 '716	174	4-16d	5-16d	1130	1985	800	1410
					4-100	J-10u	5.03	8.83	3.56	6.27
					6-10d	5-10d	1180	1360	840	965
LRU28Z	15/8	6 ¹⁵ ⁄16	115/16	25/8	0-100	J-10u	5.25	6.05	3.74	4.29
LNUZUZ	178	0.216	1.216	278	6-16d	5-16d	1180	1985	840	1410
					0-100	J-10u	5.25	8.83	3.74	6.27
					6-10d	7-10d	1610	2095	1145	1485
LRU210Z	15/8	83/16	1 15/16	1¾	0-100	7-10u	7.16	9.32	5.09	6.61
LNUZIUZ	178	0 716	I 716	1 /4	6-16d	7-16d	1610	2375	1145	1685
					0-100	7-10u	7.16	10.56	5.09	7.50
					6-10d	7-10d	1910	2095	1355	1485
LRU212Z	15/8	1011/16	1 15/16	3½	0-100	7-10u	8.50	9.32	6.03	6.61
LNUZIZZ	178	10.716	1 716	372	6-16d	7-16d	1910	2805	1355	1990
					0-16u	1-10u	8.50	12.48	6.03	8.85

- 1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed. Reduce where other load durations govern.
- 2. Factored resistances shown are applicable for roof slopes up to and including 45° (12:12). For roof slopes greater than 45° up to and including 49° (14:12) multiply the tabulated resistances by 0.85.
- 3. LRUZ's may be installed using Strong-Drive® SD#10x1½" Connector screws into the header and Strong-Drive SD#10x2½" Connector screws into the joist at full 16d capacity.
- 4. For single 2x headers, use 10dx1½" nails into the header and 10d commons into the joist. Multiply the tablulated 10d capacity x 0.77. Alternately, install Strong-Drive SD Connector screws (see footnote 3 above).
- 5. For alternate installation under retrofit applications, Strong-Drive SD#10x2½" Connector screws may be installed sloped upwards to match the roof slope (45° max). Multiply the tabulated 16d values x 0.73. See Alternate Installation for Retrofit Applications detail on page 93.
- 6. NAILS: 16d = 0.162" dia. x 31/2" ong
- 7. SCREWS: SD#10x2½" (SD10212)

LRUZ Rafter Hanger

SIMPSON Strong-Tie

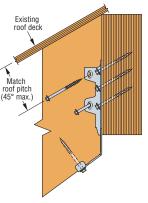
Alternate Installation for Retrofit Applications

When an existing roof deck prevents the horizontal installation of fasteners, Strong-Drive® SD #10x2½" Connector screws may be installed sloped upward to match the roof pitch (45° max.). Use table values for an installation with 16d nails x 0.73 (see footnote 5) when SD screws are sloped. Nails may not be installed sloped upward.

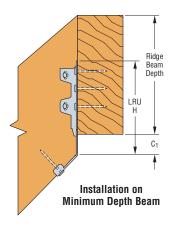
Minimum Ridge Beam Depth (in inches)

	LRI	J26		LRU28			LRU210		LRU	1212
Roof Pitch	Rafte	r Size		Rafter Size)		Rafter Size)	Rafte	r Size
	2x6	2x8	2x6	2x8	2x10	2x8	2x10	2x12	2x10	2x12
2:12	37/8	5%	_	5½	71/4	_	7%	9%	_	91/4
3:12	37/8	5¾	_	5½	71/4	_	73/4	9%	_	91/4
4:12	4	5%	_	5½	71/4	_	8	101//8	_	91/4
5:12	41/4	61/8	_	5½	7%	_	81/4	10½	_	91/4
6:12	43/8	6%	_	5½	7¾	_	85%	10%	_	91/4
7:12	45/8	65%	_	5¾	81//8	65%	9	111/4	_	9½
8:12	47/8	7	_	61/8	81/2	7	9%	11¾	7%	10
9:12	51//8	7%	_	6½	9	7%	9%	12%	81/8	10%
10:12	5%	7¾	41/2	67//8	9%	7¾	101/4	127/8	8½	1111//8
11:12	5¾	81/8	47/8	71/4	9%	81/8	10¾	13½	9	11¾
12:12	6	81/2	51//8	75/8	10½	8½	11%	141//8	9%	12%
13:12	6%	9	5½	81/8	111//8	9	12	14%	101/4	131//8
14:12	6¾	9½	57//8	85/8	11%	9½	12½	15%	10¾	13%

- 1. Minimum ridge beam depths shown assume rafter and ridge beam are flush at the top.
- Minimum ridge beam depths have been determined to ensure the Max C₁ dimension for the LRU is not exceeded.
 Deeper ridge beams may be required to support the rafter loads as determined by the Designer.



Alternate Installation for Retrofit Applications



HH Header Hangers

For fast, accurate installation of door and window headers and other cross members. HH header hangers can speed up the job, strengthen the frame, and eliminate the need for trimmers.

MATERIAL: 16 gauge FINISH: Galvanized

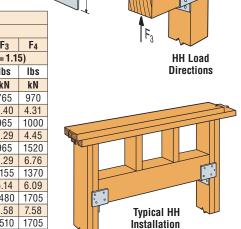
INSTALLATION: • Use all specified fasteners. See General Notes.

 Attachment to 2x studs will result in two round holes not being filled in the studs and reduction in capacity. See table for capacities and nailing requirements.

	Dimer	nsions		Footo				Fac	tored I	Resistance			
	(i	n)		Faste	illers		D.Fir-	L			S-P-F	=	
Model			Post			F ₁	F ₂	F ₃	F4	F ₁	F ₂	F ₃	F ₄
No.	w	н	Size	Doot	Header	$(K_D = 1.00)$	(K	D = 1.1	5)	$(K_D = 1.00)$	(K	(_D = 1.1	5)
	W	"		Post	пеацеі	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs
						kN	kN	kN	kN	kN	kN	kN	kN
			2x	7-10dx1½	4-10dx1½	1240	_	890	1370	1125	_	765	970
			2 X	7-10ux 1 72	4-10ux172	5.52	_	3.96	6.09	5.00	_	3.40	4.31
HH4	3½	213/16	2-2x	7-16dx2½	4-16dx2½	1715	_	1125	1410	1580	_	965	1000
11114	3 /2	2 716	Z-ZX	7-10UXZ /2	4-10ux2/2	7.63	_	5.00	6.27	7.03	_	4.29	4.45
			4x	9-16d	4-16d	2205	1125	1125	2140	2035	1040	965	1520
			47	9-10u	4-10u	9.81	5.00	5.00	9.52	9.05	4.63	4.29	6.76
			2x	10-10dx11/2	6-10dx1½	1930	_	1330	1930	1585	_	1155	1370
			۷۸	10-10ux172	0-10ux172	8.59	_	5.92	8.59	7.05	_	5.14	6.09
HH6	5½	51/8	2-2x	10-16dx21/2	6-16dx2½	2450	_	1690	2405	2260	_	1480	1705
11110	372	J /8	2-21	10-10ux272	0-10ux2/2	10.90	_	7.52	10.70	10.05	_	6.58	7.58
			4x	12-16d	6-16d	2940	1690	1690	2405	2710	1370	1510	1705
			7/	12 100	0 100	13.08	7.52	7.52	10.70	12.06	6.09	6.72	7.58



2. **NAILS**: 16d = 0.162" dia. x 3½" long. See pages 22-23 for other nail sizes and information.



HH4

LSU/LSSU Adjustable Light Slopeable/Skewable U Hangers





This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

The LSU and LSSU series of hangers may be sloped and skewed in the field, offering a versatile solution for attaching joists and rafters. These hangers may be sloped up or down and skewed left or right, up to 45°.

MATERIAL: See table

FINISH: Galvanized. Some products available in ZMAX® coating;

see Corrosion Information, pages 14-17.

INSTALLATION: • Use all specified fasteners. See General Notes.

- Attach the sloped joist at both ends so that the horizontal force developed by the slope is fully supported by the supporting members.
 - To see an installation video on this product, visit www.strongtie.com.
 - 10dx1½" nails cannot be substituted for the specified face nails when skewed or sloped and skewed combinations.

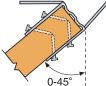
LSU and LSSU INSTALLATION SEQUENCE

(For Skewed or Sloped/Skewed Applications)



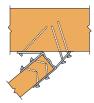
STEP 1

Nail hanger to slope-cut carried member, installing seat nail first. No bevel necessary for skewed installation. Install joist nails at 45° angle.



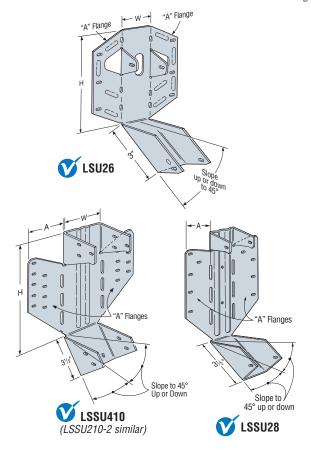
STEP 2

Skew flange from 0-45°. Bend other flange back along centerline of slots until it meets the header. Bend one time only.



STEP 3

Attach hanger to the carrying member, acute angle side first. Install nails at an angle.



These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

				Dimension	S	Foot	teners		Factored I	Resistance	
				(in)		Fasi	teners	D.F	ir-L	S-I	P-F
Joist Width	Model	Ga						Uplift	Normal	Uplift	Normal
(in)	No.	ua	w	н	A	Face	Joist	(K _D = 1.15)	$(K_D = 1.00)$	(K _D = 1.15)	$(K_D = 1.00)$
()			VV	- "	A	race	Juist	lbs	lbs	lbs	lbs
								kN	kN	kN	kN
						Sloped Or	nly Hangers				
1½	LSU26	18	1%6	47/8	11/2	6-10d	5-10dx1½	830	1255	715	895
1 72	LSU20	10	I 7/16	478	1 72	6-100	3-100X172	3.69	5.58	3.18	3.98
1½	LSSU28	18	1%6	71/8	1½	10-10d	5-10dx1½	800	3000	565	2130
1 /2	L33020	10	I 716	1 /8	1 /2	10-100	J-100X172	3.56	13.34	2.51	9.47
11/2	LSSU210	18	1%6	81/2	15/8	10-10d	7-10dx1½	1240	3090	1130	2325
1 /2	L330210	10	I 716	0 /2	1 78	10-100	7-10ux172	5.52	13.75	5.03	10.34
21/2	LSSUH310	16	2%16	81/2	31/8	18-16d	6d 12-10dx1½	1625	3675	1155	2780
Z /2	L33011310	10	Z 716	0 /2	3 /8	10-100	12-10ux172	7.23	16.35	5.14	12.37
3	LSSU210-2	16	31/8	8½	27/8	18-16d	12-10dx1½	1625	3675	1155	2780
J	L000210-2	10	J /8	072	2/8	10-100	12-10UX172	7.23	16.35	5.14	12.37
31/2	LSSU410	16	3%16	8½	25/8	18-16d	12-10dx1½	1625	4520	1155	3210
072	2000410	10	0710	072	270	10 100	12 100X172	7.23	20.11	5.14	14.28
					Skewe	ed Hangers o	r Sloped and Sk	rewed			
417	1 01100	10	40/	47/	41/	0.404	E 40441/	830	1255	715	895
1½	LSU26	18	1%16	47/8	1½	6-10d	5-10dx1½	3.69	5.58	3.18	3.98
41/	1 001100	10	10/	71/	41/	0.104	E 10dv11/	735	1360	525	965
1½	LSSU28	18	1%16	71/8	1½	9-10d	5-10dx1½	3.27	6.05	2.34	4.29
41/	LSSU210	18	10/	81/2	45/	9-10d	7.10dv11/	1240	2090	910	1485
1½	L350210	10	1%16	0 //2	15/8	9-100	7-10dx1½	5.52	9.30	4.05	6.61
2½	LSSUH310	16	29/16	8½	31/8	14-16d	12-10dx1½	1625	2345	1155	1665
Z 72	LOOUROIU	10	Z 716	0 72	378	14-100	12-10ux 1 72	7.23	10.43	5.14	7.41
3	LSSU210-2	16	31/8	8½	21/8	1/164	16d 12-10dv11/6	1625	2345	1155	1665
3	L330210-2	10	378	0 72	Z78	14-100	4-16d 12-10dx1½	7.23	10.43	5.14	7.41
3½	LSSU410	16	3%16	81/2	25/8	14-16d	12-10dx1½	1625	2345	1155	1665
3 72	L330410	10	3 7716	0 72	Z78	14-100	12-10UX172	7.23	10.43	5.14	7.41

- 1. Factored uplift resistances include a 15% increase for earthquake or wind loading; no further increase is allowed; reduce when other loads govern.
- 2. NAILS: 16d = 0.162" dia. x 31/ long, 10d = 0.148 dia. x 3" long, 10dx 1½ = 0.148" dia. x 1 2 long. See pages 2 -23 for other hail sizes and information.

SUR/SUL/HSUR/HSUL Skewed 45° Hangers



This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

The SUR/L and HSUR/L series of hangers are skewed 45° left or right. Angled nail slots direct nails for proper installation.

MATERIAL: SUR and SUL—16 gauge; HSUR and HSUL—14 gauge

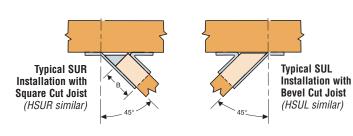
FINISH: Galvanized. Some products available in ZMAX® coating; see Corrosion Information, pages 14-17.

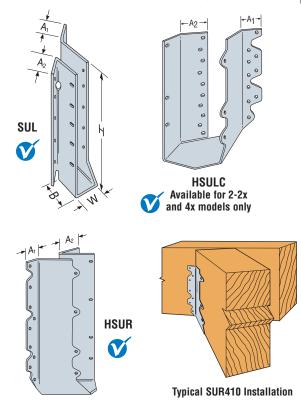
INSTALLATION: • Use all specified fasteners. See General Notes.

- These hangers will normally accommodate a 40° to 50° skew.
- Illustration shows left and right skews SUR/L (SUR = skewed right; SUL = skewed left).
- The joist end may be square cut or bevel cut.

OPTIONS: • Available with the A₂ flange (acute side) turned in on the 2-2x and 4x models only (see illustration).

- To order, add "C" (for concealed) to the product name.
- For example, specify HSURC46, HSULC46, SURC46, or SULC46.





🗾 These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

				D	imensior	18		Eact	eners		Factored F	Resistance	
					(in)			гази	GIIGI S	D.F	ir-L	S-F	P-F
	Joist	Model								Uplift	Normal	Uplift	Normal
	Size	No.	w	н	В	A ₁	A ₂	Face	Joist	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
				"		_ ^1	_ ^Z	1 400	00131	lbs.	lbs.	lbs.	lbs.
										kN	kN	kN	kN
	2x4	SUR/L24	1%16	3½	2	11/8	11/4	4-16d	4-10dx1½	850	1210	600	860
	27.1	0011/221	1710	072		170	174	1 100	1 TOUXT72	3.78	5.38	2.67	3.83
	2x6, 8	SUR/L26	1%16	5	2	11/8	15/16	6-16d	6-10dx1½	1255	2130	890	1530
					_					5.58	9.47	3.96	6.81
	2x10, 12	SUR/L210	19/16	81/8	2	11/8	15/16	10-16d	10-10dx1½	2085	3820	1480	2710
	,									9.27	16.99	6.58	12.05
	2x12, 14	SUR/L214	1%16	10	2	11//8	1 5/16	12-16d	12-10dx1½	2690	4585	2175	3255
										11.97	20.40	9.67 275	14.48
	3x10, 12	SUR/L2.56/9	2%16	813/16	33/16	11//8	21/8	14-16d	2-10dx1½	385 1.71	3950 17.57	1.22	2805 12.48
	510, 12									1130	2035	1045	1380
	(0) 0 0 0	SUR/L26-2	31/8	415/16	2%	1 ½16	2%	8-16d	4-16dx2½	5.03	9.05	4.65	6.14
	(2) 2x6, 8	LICHE /LOC 0	31/8	415/16	27/16	11/4	23/16	10.164	4.10dv01/	1230	2750	1090	1955
		HSUR/L26-2	3 1/8	4 '916	Z'/16	I 74	Z%16	12-16d	4-16dx2½	5.47	12.23	4.85	8.70
		SUR/L210-2	31/8	811/16	25/8	1 ½6	2%	14-16d	6-16dx2½	1695	4065	1540	2875
	(2) 2x10, 12	3011/ LZ 10-Z	J /8	0 /16	278	1 / 16	2/8	14-100	0-100X272	7.54	18.08	6.85	12.81
	(2) 2×10, 12	HSUR/L210-2	31/8	811/16	27/16	11/4	23/16	20-16d	6-16dx2½	1840	5270	1540	3745
		110011/12210 2	070	0 710	2710	174	2710	20 100	O TOUNE / E	8.18	23.44	6.85	16.66
		SUR/L46	3%16	43/4	25/8	1	2%	8-16d	4-16d	1130	2035	1045	1380
	4x6, 8									5.03	9.05	4.65	6.14
	4x10, 12	HSUR/L46	3%16	43/4	27/16	1	23/16	12-16d	4-16d	1230	2750	1090	1955
										5.47	12.23	4.85	8.70
		SUR/L410	3%16	81/2	27/16	1	23/16	14-16d	6-16d	1695 7.54	4065 18.08	1540 6.85	2875 12.81
										1840	5270	1540	3745
		HSUR/L410	3%16	81/2	27/16	1	23/16	20-16d	6-16d	8.18	23.44	6.85	16.66
										0.10	20.44	0.00	10.00

^{1.} Factored uplift resistances have been increased by 15% for earthquake or wind loading with no further increase allowed; reduce for other load durations as required by code.

2. NAILS: 16d = 0.162" dia x 3/2" long, 16dx2/2 = 0.162" dia x 2/2" long, 10dx1/2 = 0.140" dia x 1/2" long. See pages 22 23 for other rail sizes and information.

HRC/HHRC Hip Ridge Connectors

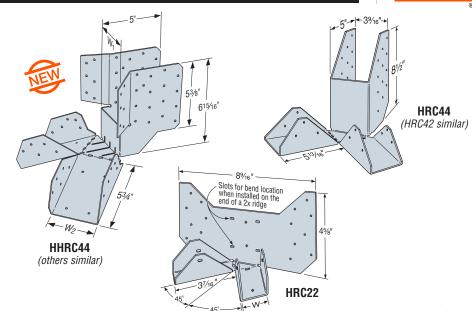
The HRC series are field slopeable connectors that attach hip roof beams to the end of a ridge beam. The HRC may be sloped downward a maximum of 45°.

MATERIAL: HRC22, HRC42—16 gauge; HRC44—14 gaugel; HHRC—12 gauge

FINISH: Galvanized

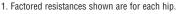
INSTALLATION:

- · Use all specified fasteners. See General Notes.
- On end of ridge—use optional diamond holes on HRC22 and HRC42 to secure the HRC. Bend face flanges on HRC22 back flush with ridge, and complete nailing.
- HRC22 on face of ridge-adjust to correct height and install nails.
- . Double bevel-cut hip members to achieve full bearing capacity.



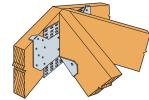
HRC Factored Resistances

	Me	mber Size	Faste	noro		Factored F	Resistance	
		(in)	гази	ilei2	D.F	ir-L	S-I	P-F
Model					Uplift	Down	Uplift	Down
No.	w	Ridge	Carrying	Each	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
	VV	niuye	Member	Hip	lbs	lbs	lbs	lbs
					kN	kN	kN	kN
HRC22	1%6	2x or 1¾"	16-10dx1½	2-10dx11//	445	1340	400	950
TINUZZ	I 716	2X UI 174	10-10ux 1 72	Z-10ux 1 72	1.98	5.96	1.78	4.23
HDC43	1%6	4x	16-16d	2-10dx11//	445	1515	400	1075
HRC42	I 716	41	10-100	Z-10ux 1 /2	1.98	6.74	1.78	4.78
HRC44	3%16	4x	24-16d	6-16d	790	2625	560	2035
1111044	J 716	77	24-100	U-10U	3.51	11.68	2.49	9.05



Total resistance carried by the connector is double this number.

- 2. Factored uplift resistances include a 15% increase for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
- 3. **NAILS:** 16d = 0.162" dia. x $3\frac{1}{2}$ " long, $10dx1\frac{1}{2} = 0.148$ " dia. x $1\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.





Typical HHRC Installation on the End of a Ridge

Optional Installation for HRC22 only

HHRC Factored Resistances

		Membe	r Width	Dime	nsions	East	eners		Factored Resi	stance per Hip	
		ıi)	1)	(i	n)	rasi	CIICIS	D.F	ir-L	S-F	P-F
	Model							Uplift	Normal	Uplift	Normal
	No.	Ridge	Hip	w.	W.	Ridge	Each	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
		niuye	пір	W ₁	W ₂	niuye	Hip	lbs	lbs	lbs	lbs
								kN	kN	kN	kN
Z	HHRC44	3½	3½	35/8	35/8	40-SD#10x2½	22-SD#10x2½	3365	4185	2390	2970
5	IIIINU44	372	372	378	378	40-30#1082/2	22-3D# 10X272	14.97	18.62	10.63	13.21
<u>~</u>	HHRC5.37/3.56	51/4	3½	53/8	33/8	40-SD#10×21/6	22-SD#10x2½	3930	4205	2790	2985
	1111103.37/3.30	J /4	372	378	378	40-SD#10x2½	22-3D# 10X2/2	17.48	18.71	12.41	13.28
\ \	HHRC64	5½	3½	5 5 ⁄8	35/8	40-SD#10x2½	22-SD#10x2½	3930	4205	2790	2985
7	111111004	372	372	378	J78	40-0D#10XZ/2	22-00# 10X272	17.48	18.71	12.41	13.28

- 1. Factored resistances shown are per hip, the total load carried by the connector is double this number. Load must be equally distributed to both hips.
- 2. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed. Reduce where other load durations govern.
- 3. Factored resistances shown are applicable for roof slopes up to and including 45° (12:12).
- 4. Do not attach HHRC to columns or stude
- 5. **SCREWS**: SD#10x2½" (SD10212)

VPA Adjustable Variable Pitch Connectors

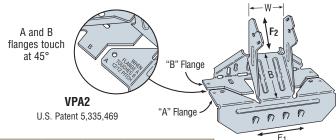
SIMPSON
Strong-Tie

The VPA may be sloped in the field, offering a versatile solution for attaching rafters to the top plate. It will adjust to accommodate slopes between 3:12 and 12:12, making it a complement to the versatile LSSU. This connector eliminates the need for notched rafters, beveled top plates and toe nailing.

MATERIAL: 18 gauge FINISH: Galvanized

INSTALLATION: • Use all specified fasteners.

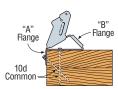
See General Notes.



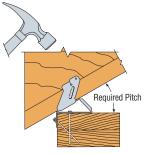
			East	eners	Factored Resistance									
	Actual		Газі	GIIGIS		D.I	Fir-L			S-	P-F			
Model	Joist	W			Wind/Ear	thquake (K _D = 1.15)	Normal	Wind/Ear	thquake (K _D = 1.15)	Normal		
No.	Width (in)	(in)	Carrying	Carried	Uplift	F ₁	F ₂	$(K_D = 1.00)$	Uplift	F ₁	F ₂	$(K_D = 1.00)$		
	(in)		Member	Member	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs		
					kN	kN	kN	kN	kN	kN	kN	kN		
VPA2	11/2	1%6	8-10d	2-10dx1½	405	695	405	1695	370	615	370	1555		
VFAZ	1 /2	I 716	0-10u	Z-100X172	1.80	3.09	1.80	7.54	1.65	2.74	1.65	6.92		
VPA3	21/2	2%16	9-10d	2-10dx1½	405	695	405	2050	370	615	370	1855		
VIAS	2/2	Z 716	3-10u	Z-10UX172	1.80	3.09	1.80	9.12	1.65	2.74	1.65	8.25		
VPA4	3½	3%16	11-10d	2-10dx1½	405	695	405	2050	370	615	370	1855		
V11/A4	372	J 716	11-10u		1.80	3.09	1.80	9.12	1.65	2.74	1.65	8.25		

- 1. Factored uplift and lateral resistances have been increased 15% for earthquake or wind loading; no further increase is allowed.
- 2. Resistances may not be increased for short-term load duration.
- 3. NAILS: 10d = 0.148" dia. x 3" long, $10dx1\frac{1}{2} = 0.148$ " dia. x $1\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.

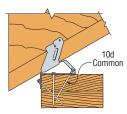
VPA INSTALLATION SEQUENCE



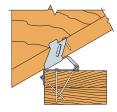
STEP 1 Install top nails and face PAN nails in "A" flange to outside wall top plate.



STEP 2
Seat rafter with a hammer, adjusting "B" flange to the required pitch.



STEP 3
Install "B" flange nails in the obround nail holes, locking the pitch.



STEP 4
Install 10dx1½" nail into tab
nail hole. Hammer nail in at a
slight angle to prevent splitting.

HCP Hip Corner Plates

The HCP connects a rafter or joist to double top plates at a 45° angle.

MATERIAL: 18 gauge

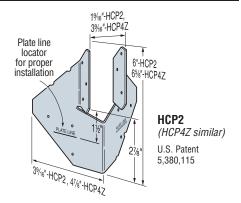
FINISH: HCP2—galvanized or ZMAX® coating; HCP4Z—ZMAX coating INSTALLATION: • Use all specified fasteners. See General Notes.

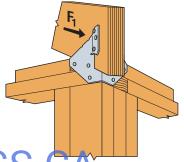
- Attach HCP to double top plates; birdsmouth not required for table values.
- Install rafter and complete nailing. Rafter may be sloped to 45°.
- These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

		Faste	nore	Factored Resistance						
		rasit	111613	D.F	ir-L	S-I	P-F			
Model	Hip			Uplift	F ₁	Uplift	F ₁			
No.	Size	To	To	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$			
		Hip	Plates	lbs	lbs	lbs	lbs			
				kN	kN	kN	kN			
HCP2	2x	6-10dx11/2	6-10dx1½	1020	355	890	325			
11072	2.1	0-10ux172	0-10ux172	4.54	1.58	3.96	1.45			
LICD47	4x	8-10d	8-10d	1485	435	1300	310			
HCP4Z	48	0-10u	0-10u	6.61	1.94	5.78	1.38			

- The HCP can be installed on the inside and the outside of the wall with a flat bottom chord truss and achieve twice the factored resistance.
- Factored uplift resistances include a 15% increase for earthquake or wind loading; no further increase allowed.
- 3. NAILS: 10d = 0.148" dia, x 3 long, 10tx1½ = 0.148" dia, x 1½ long.

 See pages 22-23 for other nail sizes and information.





This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

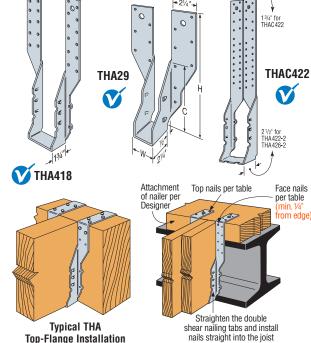
The THA series' extra long straps allow full nailing and can be field-formed to give top flange hanger convenience.

MATERIAL: See table FINISH: Galvanized

INSTALLATION: • Use all specified fasteners. See General Notes.

Two different installation methods may be used:

- Face Mount—Install all face nails according to the table. Nails used for the joist attachment must be driven at an angle so that they penetrate through the corner of the joist into the header. With single 2x carrying members, use 10dx11/2" nails into the carrying member, and 10d commons into the carried member, when 10d nails are specified and use 0.77 of the table value. When 16d nails are specified use 10dx1½" nails into the carrying member and 10d commons into the carried member for 0.64 of the table value.
- Top Flange—For the THA29, the top-flange nailing schedule requires the use of joist double shear nailing as detailed above, and that the strap be fieldformed over the header a minimum of 21/2". A minimum of four top and four face nails must be used. For all models except the THA29, the top-flange nailing schedule may be followed where double shear nailing is not possible, provided the strap is field-formed over the top of the header a minimum of . 1½" for the THA213 and THA413, and 2" for all others, and a minimum of four top and two face nails are used. The joist double shear nailing tabs are easily straightened so that the nails can be driven straight into the joist.
- Uplift—Lowest face nails must be installed to achieve uplift resistances. **OPTIONS:** • THA hangers available with the header flanges turned in for 35/8" (except THA413) and larger, with no reduction in capacity - order THAC hanger.



Top-Flange Installation

D.Fir-L

Factored Resistance

Typical THA Top Flange Installation on a Nailer (except THA29)

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details. **Fasteners** Dimensions

Header

Joist

(in)

,,	
N	
20	
.10	
60	
83	`
CO.	

Double Shear Nailing Top View



Double Shear Nailing Side View Do not bend tab unless otherwise noted



Dome Double Shear Nailing prevents tabs breaking off (available on some models) Catalogue C-C-CAN2015 © 2015 SIMPSON STRONG-TIE COMPANY INC

U.S. Patent 5,603,580

No. W		Min Joist	Model	Ga								Uplift	Normal	Uplift	Normal
Than Than			No.	ua	w	ш	r	Ton	Eann	Straight	Clant	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
Than Than		0.20			VV	"	U	TOP	Гасс	Straight	Siaiit	lbs	lbs	lbs	lbs
Thache												kN	kN	kN	kN
THA213								TOP	-FLANGE	INSTALLAT	ΓΙΟΝ				
THA213 18 1% 13% 5½ 4-10d 2-10d 4-10dx1½ — 9.90 — 7.83 THA218 18 1% 17% 5½ 4-10d 2-10d 4-10dx1½ — 9.90 — 7.83 THA218 18 1% 17% 5½ 4-10d 2-10d 4-10dx1½ — 9.90 — 7.83 THA218-2 16 3% 171% 8 4-16d 2-16d 6-16dx2½ — 9.90 — 7.83 THA218-2 16 3% 22% 8 4-16d 2-16d 6-16dx2½ — 11.90 — 10.70 THA222-2 16 3% 22% 8 4-16d 2-16d 6-16dx2½ — 11.90 — 10.70 4x6 THA413 18 3% 13% 4½ 4-10d 2-10d 4-10d — 2225 — 1655 THA418 16 3% 17½ 7% 4-16d 2-16d 6-16d — 9.90 — 7.36 THA418 16 3% 17½ 7% 4-16d 2-16d 6-16d — 11.90 — 10.70 THA422 16 3% 22 7% 4-16d 2-16d 6-16d — 2675 — 2405 THA426 14 3% 26 7% 4-16d 4-16d 6-16d — 11.90 — 10.70 THA428 18 1% 13% 5½ — 16-10d — 4-10d 1050 3440 750 2455 THA213 18 1% 13% 5½ — 14-10d — 4-10d 1420 2785 1290 2210 THA218 18 1% 17% 5½ — 18-10d — 4-10d 1420 2785 1290 2210 THA218 18 1% 17% 5½ — 18-10d — 4-10d 6.32 12.39 5.74 9.83 THA218 18 3% 13% 4½ — 14-10d — 4-10d 6.32 12.39 5.74 9.83 THA218 18 3% 13% 4½ — 14-10d — 4-10d 6.32 12.39 5.74 9.83 THA218 18 3% 13% 4½ — 14-10d — 4-10d 6.32 12.39 5.74 9.83 THA218 18 1% 17% 5½ — 18-10d — 4-10d 6.32 12.39 5.74 9.83 THA218 18 1% 17% 5½ — 18-10d — 4-10d 6.32 12.39 5.74 9.83 THA218 18 1% 17% 5½ — 18-10d — 4-10d 6.32 12.39 5.74 9.83 THA218 18 1% 17% 5½ — 18-10d — 4-10d 6.32 12.39 5.74 9.83 THA218 18 1% 17% 5½ — 18-10d — 4-10d 6.32 12.39 5.74 9.83 THA218 18 1% 17% 5½ — 18-10d — 4-10d 6.32 12.39 5.74 9.83 THA218 18 1% 17% 5½ — 18-10d — 4-10d 6.32 12.39 5.74 9.83 THA218 18 1% 17% 5½ — 18-10d — 4-10d 6.32 12.39 5.74 9.83 THA218 18 16 3% 17½ 7% — 16-16d — 6-16d 11.30 21.20 8.03 15.06 THA222-2 16 3% 22% 8 — 22-16d — 6-16d 11.30 21.20 8.03 15.06 THA418 16 3% 17½ 7% — 16-16d — 6-16d 11.30 21.20 8.03 15.06 THA418 16 3% 17½ 7% — 16-16d — 6-16d 11.30 21.20 8.03 15.06 THA418 16 3% 17½ 7% — 16-16d — 6-16d 11.30 21.20 8.03 15.06 THA418 16 3% 17½ 7% — 16-16d — 6-16d 11.30 21.20 8.03 15.06		2×4	THASO	10	15/4	011/	5 1/ ₂	1-10d	1-10d		1-10d	1050		750	2720
THA213		2,44	IIIAZS	10	178	3.716	J 78	4-10u	4-10u		4-10u	4.67		3.34	
Than Than			THA213	18	15%	135/16	51/2	4-10d	2-10d	4-10dx11/2	_				
THAZ18 18 1% 17% 5½ 4-100 2-100 4-100X1½ — 9.90 — 7.83 THAZ18-2 16 3½ 171½ 8 4-16d 2-16d 6-16dx2½ — — 11.90 — 10.70 THAZ22-2 16 3½ 22% 8 4-16d 2-16d 6-16dx2½ — — 11.90 — 10.70 4x6 THA413 18 3½ 13% 4½ 4-10d 2-10d 4-10d — — 2675 — 2405 THA418 16 3½ 17½ 7% 4-16d 2-16d 6-16d — — 2675 — 2405 THA422 16 3½ 22 7% 4-16d 2-16d 6-16d — — 2675 — 2405 THA422 16 3½ 22 7% 4-16d 2-16d 6-16d — — 11.90 — 10.70 THA422 16 3½ 26 7½ 4-16d 4-16d 6-16d — — 11.90 — 10.70 THA423 18 1½ 91½ 5½ — 16-10d — 4-10d 10.50 3440 750 2455 THA213 18 1½ 13% 5½ — 14-10d — 4-10d 10.32 12.39 5.74 9.83 THA218 18 1½ 17½ 5½ — 18-10d — 4-10d 1420 2785 1290 2210 THA218-2 16 3½ 22½ 8 — 16-16d — 6-16d 11.30 21.20 8.03 15.06 THA22-2 16 3½ 22½ 8 — 22-16d — 6-16d 11.30 21.20 8.03 15.06 THA218 18 13 3½ 13% 4½ — 14-10d — 4-10d 1420 3555 1290 2525 THA218 18 13 3½ 13% 4½ — 14-10d — 4-10d 1420 3555 1290 2525 THA218 16 3½ 22½ 8 — 22-16d — 6-16d 11.30 21.20 8.03 15.06 THA418 16 3½ 17½ 7% — 16-16d — 6-16d 11.30 21.20 8.03 15.06 THA418 16 3½ 22½ 8 — 14-10d — 4-10d 1420 3555 1290 2525 THA418 16 3½ 22½ 8 — 22-16d — 6-16d 11.30 21.20 8.03 15.06 THA418 16 3½ 22½ 8 — 14-10d — 4-10d 1420 3555 1290 2525 THA418 16 3½ 22½ 8 — 14-10d — 4-10d 1420 3555 1290 2525 THA418 16 3½ 22½ 8 — 14-10d — 4-10d 1420 3555 1290 2525 THA418 16 3½ 22½ 8 — 14-10d — 4-10d 1420 3555 1290 2525 THA418 16 3½ 22½ 8 — 14-10d — 4-10d 1420 3555 1290 2525 THA418 16 3½ 22½ 8 — 14-10d — 4-10d 1420 3555 1290 2525 THA418 16 3½ 22½ 8 — 14-10d — 4-10d 1420 3555 1290 2525 THA418 16 3½ 22½ 8 — 22-16d — 6-16d 11.30 21.20 8.03 15.06 THA422 16 3½ 22½ 8 — 14-10d — 4-10d 13.00 21.20 8.03 15.06 THA428 14 3½ 36 17½ 7½ 7½ — 16-16d — 6-16d 13.30 21.20 8.03 15.06		2x6		-											
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Ax6		2-2x10	T114 000 0	40	01/	0007		4.40.1	0.40.1	0.401.01/	101.04/	_			
THA418			THA222-2	16	31/8	22%16	8	4-160	2-160	6-160X2½	_	_	11.90	_	10.70
THA418 16 3% 17½ 7% 4-16d 2-16d 6-16d — 2675 — 2405 THA422 16 3% 22 7% 4-16d 2-16d 6-16d — 2675 — 2405 THA426 14 3% 26 7% 4-16d 4-16d 6-16d — 15.97 — 10.70 THA26 14 3% 26 7% 4-16d 4-16d 6-16d — 15.97 — 11.83 FACE-MOUNT INSTALLATION 2x4 THA29 18 1% 9½ 5% — 16-10d — 4-10d 1050 3440 750 2455 THA213 18 1½ 13¾ 5½ — 14-10d — 4-10d 6.32 12.39 5.74 9.83 THA218 18 1½ 17¾ 5½ — 18-10d — 4-10d 6.32 12.39 5.74 9.83 THA218 18 1½ 17¾ 5½ — 18-10d — 4-10d 6.32 12.39 5.74 9.83 THA218 18 1½ 17¾ 5½ — 16-16d — 6-16d 2540 4765 1805 3385 THA218 18 1½ 17¼ 6 5½ — 14-10d — 4-10d 6.32 12.39 5.74 9.83 THA218 18 1½ 17¼ 6 5½ — 18-10d — 4-10d 6.32 12.39 5.74 9.83 THA218 18 1½ 17¼ 6 5½ — 18-10d — 4-10d 6.32 12.39 5.74 9.83 THA218 18 1½ 17¼ 6 5½ — 18-10d — 4-10d 6.32 12.39 5.74 9.83 THA218-2 16 3¼ 22¾ 6 8 — 22-16d — 6-16d 2540 4765 1805 3385 THA218 18 3¾ 13¼ 6 4½ — 14-10d — 4-10d 6.32 15.90 2525 THA218 18 3¾ 13¼ 6 5½ — 14-10d — 4-10d 6.32 15.90 5550 1805 4150 THA418 16 3¾ 17½ 7% — 16-16d — 6-16d 11.30 24.69 8.03 18.46 4x6 THA413 18 3¾ 13¼ 6 4½ — 14-10d — 4-10d 6.32 15.81 5.74 11.23 THA418 16 3¾ 27½ 7% — 16-16d — 6-16d 11.30 21.20 8.03 15.06 THA418 16 3¾ 22 7½ 6 — 22-16d — 6-16d 11.30 21.20 8.03 15.06 THA422 16 3½ 22 7½ 6 — 22-16d — 6-16d 11.30 21.20 8.03 15.06 THA428 14 3¾ 66 7½ — 22-16d — 6-16d 2540 4765 1805 3385 THA418 16 3½ 28 7½ 6 — 22-16d — 6-16d 11.30 21.20 8.03 15.06		1,46	TU / /12	10	25/	105/.	/ 11/	4 10d	2 104	4 104		_	2225	_	1655
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That That			11171410	10	078	11 /2	1 / 0	7 100	2 100	0 100				_	
THA426 14 3% 26 7% 4-16d 4-16d 6-16d — 3590 — 2660 — 15.97 — 11.83 FACE-MOUNT INSTALLATION 2x4 THA29 18 1% 911/16 5% — 16-10d — 4-10d 4.67 15.30 3.34 10.92 THA213 18 11/8 131/16 51/2 — 14-10d — 4-10d 6.32 12.39 5.74 9.83 THA218 18 11/8 171/16 51/2 — 18-10d — 4-10d 6.32 12.39 5.74 9.83 THA218 18 11/8 171/16 8 — 16-16d — 6-16d 11.30 21.20 8.03 15.06 THA222-2 16 31/8 223/6 8 — 22-16d — 6-16d 11.30 24.69 8.03 18.46 4x6 THA413 18 31/8 131/6 41/2 — 14-10d — 4-10d 6.32 15.81 5.74 11.23 THA418 16 31/8 171/7 7/6 — 16-16d — 6-16d 11.30 24.69 8.03 15.06 4x10 THA422 16 33/8 22 7/6 — 22-16d — 6-16d 11.30 21.20 8.03 15.06 4x10 THA422 16 33/8 22 7/6 — 22-16d — 6-16d 11.30 21.20 8.03 15.06 4x10 THA422 16 33/8 22 7/6 — 22-16d — 6-16d 11.30 21.20 8.03 15.06)	4x10	THA422	16	3%	22	71/8	4-16d	2-16d	6-16d	_				
THA26 14 3% 26 7% 4-16d 4-16d 6-16d — — 15.97 — 11.83 FACE-MOUNT INSTALLATION 2x4 THA29 18 1% 91½6 5½ — 16-10d — 4-10d 1050 3440 750 2455 4.67 15.30 3.34 10.92 2x6 THA213 18 1½ 13¾6 5½ — 14-10d — 4-10d 1420 2785 1290 2210 6.32 12.39 5.74 9.83 THA218 18 1½ 17¾6 5½ — 18-10d — 4-10d 6.32 12.39 5.74 9.83 THA218-2 16 3⅓ 171⅓6 8 — 16-16d — 6-16d 11.30 21.20 8.03 15.06 THA222-2 16 3⅓ 22¾6 8 — 22-16d — 6-16d 11.30 24.69 8.03 18.46 4x6 THA413 18 3⅓ 13⅙6 4½ — 14-10d — 4-10d 1420 3555 1290 2525 THA418 16 3⅓ 17½ 7⅓ — 16-16d — 6-16d 11.30 21.20 8.03 15.06 THA418 16 3⅓ 17½ 7⅓ — 16-16d — 6-16d 2540 4765 1805 3385 THA418 16 3⅓ 17½ 7⅓ — 16-16d — 6-16d 2540 4765 1805 3385 THA418 16 3⅓ 17½ 7⅓ — 16-16d — 6-16d 2540 4765 1805 3385 THA418 16 3⅓ 17½ 7⅓ — 16-16d — 6-16d 2540 5850 1805 4150 THA422 16 3⅓ 22 7⅓ — 22-16d — 6-16d 2540 5850 1805 4150 THA428 14 3⅙ 16 3⅓ 17½ 7⅙ — 16-16d — 6-16d 2540 5850 1805 4150 THA428 14 3⅙ 16 3⅓ 15.06 455															
THA218 18 15 171/16 8 — 16-16d — 4-10d			THA426	14	3%	26	71/8	4-16d	4-16d	6-16d	_				
2x4 THA29 18 1% 9½6 5½6 — 16-10d — 4-10d 1050 3440 750 2455 4.67 15.30 3.34 10.92 2x6 THA213 18 1½6 5½ — 14-10d — 4-10d 6.32 12.39 5.74 9.83 THA218 18 1½6 17¾6 5½ — 18-10d — 4-10d 6.32 12.39 5.74 9.83 2-2x10 THA218-2 16 3½8 17½6 8 — 16-16d — 6-16d 11.30 21.20 8.03 15.06 2-2x10 THA222-2 16 3½8 22¾6 8 — 22-16d — 6-16d 11.30 21.20 8.03 18.46 4x6 THA413 18 3½6 4½ — 14-10d — 4-10d 4250 3555 1290 2525 4x10 THA418								FACE	-MOIINT	LINSTALLA.	TION		13.37		11.03
2x6 THA213 18 1% 1% 13% 5½								170		INDIALLA		1050	3///0	750	2/55
THA213 18 1% 13% 5½ — 14-10d — 4-10d 6.32 12.39 5.74 9.83 THA218 18 1% 17% 5½ — 18-10d — 4-10d 6.32 12.39 5.74 9.83 THA218 18 1% 17% 5½ — 18-10d — 4-10d 6.32 12.39 5.74 9.83 THA218-2 16 3½ 17½ 8 — 16-16d — 6-16d 11.30 21.20 8.03 15.06 4x6 THA413 18 3½ 13½ 4½ — 14-10d — 4-10d 2540 4765 1805 3385 THA418 16 3½ 17½ 7½ — 16-16d — 6-16d 11.30 24.69 8.03 18.46 4x10 THA422 16 3½ 22 7½ — 22-16d — 6-16d 2540 4765 1805 3385 THA428 16 3½ 17½ 7½ — 16-16d — 6-16d 2540 4765 1805 3385 THA428 16 3½ 17½ 7½ — 16-16d — 6-16d 2540 4765 1805 3385 THA428 16 3½ 17½ 7½ — 16-16d — 6-16d 2540 4765 1805 3385 THA428 16 3½ 17½ 7½ — 16-16d — 6-16d 2540 4765 1805 3385 THA428 16 3½ 22 7½ — 22-16d — 6-16d 2540 5850 1805 4150 THA428 16 3½ 22 7½ — 22-16d — 6-16d 2540 5850 1805 4150 THA428 16 3½ 22 7½ — 22-16d — 6-16d 2540 5850 1805 4150 THA428 16 3½ 22 7½ — 22-16d — 6-16d 2540 5850 1805 4150 THA428 14 3½ 35 36 7½ 7½ 7½ — 22-16d — 6-16d 2540 5850 1805 4150 THA428 14 3½ 35 36 7½ 7½ 7½ — 22-16d — 6-16d 2540 5850 1805 4150 THA428 14 3½ 35 36 7½ 7½ 7½ — 22-16d — 6-16d 2540 5850 1805 4150 THA428 14 3½ 35 36 7½ 7½ 7½ — 22-16d — 6-16d 2540 5850 1805 4150 THA428 14 3½ 35 36 7½ 7½ 7½ — 22-16d — 6-16d 2540 5850 1805 4150		2x4	THA29	18	1%	911/16	51/8	_	16-10d	_	4-10d				
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2-2x10			ITAZIO	10	1 1 7/8	17 %16	3 ½	_	10-100	_	4-100	6.32	12.39	5.74	9.83
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THA222-2 16 31/8 223/16 8 — 22-16d — 6-16d 11.30 24.69 8.03 18.46 4x6 THA413 18 35/8 135/16 41/2 — 14-10d — 4-10d 1420 3555 1290 2525 THA418 16 35/8 171/2 77/6 — 16-16d — 6-16d 11.30 21.20 8.03 15.06 4x10 THA422 16 35/8 22 77/8 — 22-16d — 6-16d 2540 5850 1805 4150 THA426 14 35/4 36 77/4 — 20-184 6-186 2540 52540 5295 1805 4545		2-2×10	111AZ10-Z	10	J /8	17 /16	0		10-100		0-10u				
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4x10 THA422 16 35% 22 77% — 16-16d — 6-16d 21.20 8.03 15.06 35% 22 77% — 22-16d — 6-16d 2540 5850 1805 4150 11.30 26.02 8.03 18.46 2540 5850 1805 4545														-	
4x10 THA422 16 3% 22 7% — 22-16d — 6-16d 2540 5850 1805 4150 THA426 14 3% 26 7% 20-18d 6-18d 2540 5250 1805 4545			THA418	16	3%	17½	71/8	_	16-16d	_	6-16d				
4X10 THA422 16 3% 22 7% — 22-160 — 6-160 11.30 26.02 8.03 18.46 THA426 14 354 86 774 20.184 6.186 2540 6295 1806 4545															
THA 26 14 35 6 77 20.134 6.136 2540 6295 1806 4545	•	4x10	THA422	16	3%	22	1 1/8		22-16d	_	6-16d				
111.30 28.00 3.03 20.22			THV 436	1/	25/	26	77	\/	20.164		6.160				
			117A420	14	378	20	VI'V	V.	50-10u	NIU	0-100	11.30	28.00	8.03	20.22

- 1. Factored uplift resistances have been increased 15% for earthquake or wind loading with no further increase allowed; reduce where other loads govern.
- 2. **NAILS:** 16d = 0.162" dia. x 3½" long, 16dx2½ = 0.162" dia. x 2½" long, 10d = 0.148" dia. x 3" long, 10dx1½ = 0.148" dia. x 11/2" long. See pages 22-23 for other nail sizes and information.

Solid Sawn Lumber Connectors

FACE MOUNT HANGERS HU/HUCQ/HGUS Glulam Beam & Double Shear Joist Hangers

Strong-Tie

See Hanger Options on page 230 for hanger modifications, which may result in reduced loads.

HU—Most models have triangle and round holes. To achieve maximum factored resistances, fill both round and triangle holes with common nails.

HGUS—The highest factored resistances available for nailed face mount hangers.

All hangers in this series have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation, and the use of common nails for all connections. (Do not bend or remove tabs)

HUCQ – Heavy duty joist hangers that incorporate Simpson Strong-Tie® Strong Drive® SDS Heavy-Duty Connector screws

MATERIAL: See tables

FINISH: Galvanized. Some products available in ZMAX® coating; see Corrosion Information, pages 14-17.

INSTALLATION: • Use all specified fasteners. See General Notes.

- HU—can be installed filling round holes only, or filling round and triangle holes for maximum values.
- HGUS—Nails must be driven at an angle through the joist into the header to achieve the table values.
- HUCQ Install ¼"x2½" Strong Drive SDS Heavy-Duty Connector screws (included) in all round holes. Lag screws will not achieve the same capacities.
- · Not designed for nailer applications.
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated resistance.
- With 3x carrying members, use 16dx2½" (0.162" dia. x 2½" long) nails into the header and 16d commons into the joist with no reduction in resistances. With 2x carrying members, use 10dx1½" (0.148" dia. x 1½" long) nails into the header and 10d commons into the joist, and reduce the factored resistances to 0.64 of the table value.

<code>OPTIONS: • HÚ</code> hangers available with the header flanges turned in for $25\!\%6"$ and larger widths, with no reduction in resistances—order HUC hanger.

- See Hanger Options on page 230, for sloped and/or skewed HU models, and HUC (concealed flange) models.
- Concealed flanges are not available for HGUS.
- · Other sizes available; contact Simpson Strong-Tie.



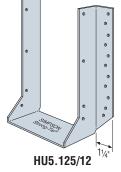


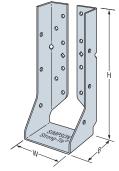
Double Shear Nailing Side View Do not bend tab back



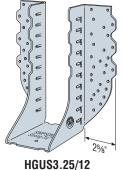
Dome Double Shear Nailing prevents tabs breaking off *(available on some models)*

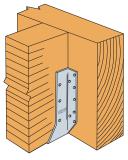
U.S. Patent 5,603,580





HUCQ5.25/9





2 Typical HU Installation

Model configurations may differ from those shown. Some HU models do not have triangle holes. Contact Simpson Strong-Tie for details. Projection seat on most models for maximum bearing and section economy.

			D	imensi	ions (i	n)	Faste	ners	Factored Resistance			
									D.Fir-L	Glulam	Spruce-Pi	ne Glulam
Joist	Model	0-							Uplift	Normal	Uplift	Normal
Size	No.	Ga	W	Н	В	de ⁴	Face	Joist	(K _D = 1.15)	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
									lbs	lbs	lbs	lbs
									kN	kN	kN	kN
	HU3.25/10.5	14	31/4	101/4	2½	97/8	22-16d	10-10d	2635	5780	2450	4690
	HUC3.25/10.5	14	374	1074	2 72	978	22-10u	10-100	11.72	25.75	10.90	20.86
	HU3.25/12	14	31/4	113/4	2½	11%	24-16d	12-10d	3160	5780	2695	5780
31/8	HUC3.25/12	14	J /4	1174	2 /2	1178	24-10u	12-10u	14.06	25.75	11.99	25.75
	HU3.25/16	14	31/4	13%	2½	13½	26-16d	12-10d	3160	5780	2695	5780
GLULAM	HUC3.25/16		074	1078		1072	20 100	12 100	14.06	25.75	11.99	25.75
	HGUS3.25/10	12	31/4	85/6	4	81/4	46-16d	16-16d	6840	14645	4855	10400
	110000.20/10	12	074	070		074	40 10u	10 100	30.47	65.23	21.60	46.26
	HGUS3.25/12	12	31/4	10%	4	101/4	56-16d	20-16d	7640	14995	5425	10645
					-				33.98	66.79	24.13	47.35
	HUCQ5.25/9-SDS	14	51/4	9	3	3 8	12-¼"x2½" SDS		3210	7270	2900	7645
		-						SDS	14.28	32.34	12.90	34.01
	HUCQ5.25/11-SDS	14	51/4	11	3	8	14-1/4"x21/2" SDS	6-1/4"x21/2" SDS	3210	9090	2900	7645
							อบอ	อบอ	14.28	40.44	12.90	34.01
	HU5.125/12 HUC5.12/12	14	51/4	101/4	21/2	9%	22-16d	8-16d	2455	5780	2280	4690
=									10.92	25.75	10.14	20.86
51/8 GLULAM	HU5.125/13.5 HUC5.125/13.5	14	51/4	131/4	21/2	12%	26-16d	12-16d	3685 16.39	7025 31.25	2615 11.63	6185 27.51
GLOLAW									3685	7025	2615	6185
	HU5.125/16 HUC5.125/16	14	51/4	13%	2½	13½	26-16d	12-16d	16.39	31.25	11.63	27.51
									6840	14645	4855	10400
	HGUS5.25/10	12	51/4	91/16	4	81/4	46-16d	16-16d	30.47	65.23	21.60	46.26
	1101105 05/40	10	\-A	6 No. 6		1007		200	7640	14995	5425	10645
	HGUS5.25/12	12	51/4	10% ₆	4	103/16	56-16d	20-16d	33.98	-66,79	24.13	47.35

- 1. 10d common nails may be used instead of the specified 16d nails at 0.83 of the tabulated resistance value
- Factored uplift resistances have been increased 15% for earthquake or wind loading with no further increase allowed. Reduce by 15% for standard term loading.
- MIN nailing quantity and load values— fill all round holes; MAX nailing quantity and load values fill all round and triangle holes.
- 4. d_e is the distance from the bearing seat to the top joist nail.
- For proprietary non-standard glulam sizes, see pages 114-116 for structural composite lumber.
- 6. NAILS: 16d = 0.162" dia. x 3½" long, 10d = 0.148" dia. x 3" long. See pages 22-23 for other nail sizes and nformation.

LGU/MGU/HGU/HHGU High Capacity Girder Hangers



The GU hangers are high-capacity girder hangers designed for situations where the header and joist are flush at the top. These products can be used for retrofit on the framing members after they are temporarily placed in position. Simpson Strong-Tie® Strong-Drive® SDS Heavy-Duty Connector screws make installation fast and easy, with no pre-drilling required.

MATERIAL: See table

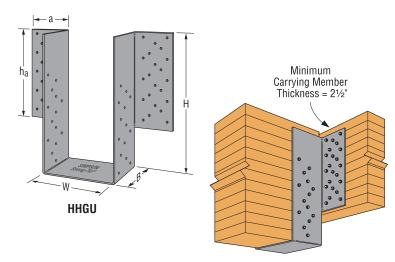
FINISH: LGU/MGU—Galvanized, HGU/HHGU—Simpson Strong-Tie® gray paint

INSTALLATION:

- Use all specified fasteners. See General Notes.
- Install with 1/4"x21/2" Strong-Drive SDS Heavy-Duty Connector screws, which are provided with the hangers. (Note: lag screws will not achieve the same loads.)
- · All multiple members must be fastened together to act as a single unit.
- · Multiple member headers may require additional fasteners at the hanger locations. The quantity and location of the additional fasteners must be determined by the Designer.

OPTIONS: • LGU, MGU and HGU hangers may be skewed up to 45°.

- · One flange can be concealed for some sizes.
- For proprietary non-standard glulam sizes, see page 117 for structural composite lumber.
- See Hanger Options page 230.



Typical HHGU Installation

			ı	Dimensio	ns		Faste	200	Factored Resistance				
				(in)			rasie	ners	D.F	ir-L	S-P-F		
Model	0-								Uplift	Normal	Uplift	Normal	
No.	Ga	w	В	Min.	h		Hoodor	laiat	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$	
		VV	В	Height (H)	ha	a	Header	Joist	lbs	lbs	lbs	lbs	
				(11)					kN	kN	kN	kN	
1 OUD OF CDCO F	10	01/	41/	0	71/	01/	16-¼"x2½" SDS	12-¼"x2½" SDS	7730	10170	5565	7320	
LGU3.25-SDS2.5	10	31/4	41/2	8	7½	31/4	16-74 XZ72 SDS	12-74 XZ72 SDS	34.39	45.24	24.76	32.56	
LGU5.25-SDS2.5	10	51/4	41/2	8	71/2	31/4	16-¼"x2½" SDS	12-1/4"x21/2" SDS	7730	10170	5565	7320	
LGU3.23-3D32.3	10	374	4 72	0	1 72	374	10-74 XZ72 3D3	12-74 XZ72 3D3	34.39	45.24	24.76	32.56	
MGU5.25-SDS2.5	10	51/4	41/2	91/4	83/4	4	24-¼"x2½" SDS	16-¼"x2½" SDS	10100	13140	7270	9460	
WIG03.23-3D32.3	10	J /4	4 /2	374	074	4	24-74 X272 3D3	10-74 XZ72 3D3	44.93	58.45	32.34	42.08	
HGU5.25-SDS2.5	7	51/4	51/4	11	10%	43/4	36-1/4"x21/2" SDS	24-1/4"x21/2" SDS	14300	20320	10295	14630	
11003.20 0002.0		J /4	J /4	'''	1078	7/4	00 74 XZ72 0D0	Z4 /4 XZ/2 0D0	63.61	90.39	45.80	65.08	
MGU7.00-SDS2.5	10	7	41/2	91/4	83/4	4	24-1/4"x21/2" SDS	16-¼"x2½" SDS	10100	13140	7270	9460	
WG07.00 0D02.3	10		7/2	374	074	7	24 /4 X2/2 0D0	10 74 XZ72 0D0	44.93	58.45	32.34	42.08	
HGU7.00-SDS2.5	7	7	51/4	11	10%	43/4	36-1/4"x21/2" SDS	24-1/4"x21/2" SDS	14300	20320	10295	14630	
11007.00 0002.0			J /4	'''	1078	7/4	00 74 XZ72 0D0	Z4 /4 XZ/2 0D0	63.61	90.39	45.80	65.08	
HHGU7.00-SDS2.5	3	7	51/4	13	12%	43/4	44-1/4"x21/2" SDS	28-1/4"x21/2" SDS	21740	26665	15655	19195	
1111007.00 0002.0	0	_ ′	J 74	10	12/0	7/4	44 74 XZ72 ODO	20 74 XZ72 0D0	96.71	118.62	69.64	85.39	
HGU9.00-SDS2.5	7	9	51/4	11	10%	43/4	36-1/4"x21/2" SDS	24-1/4"x21/2" SDS	14300	20320	10295	14630	
11009.00-0002.0	′	3	J /4	11	1078	474	30-74 XZ72 3D3	24-74 X272 3D3	63.61	90.39	45.80	65.08	
HHGU9.00-SDS2.5	3	9	51/4	13	12%	43/4	44-1/4"x21/2" SDS	28-1/4"x21/2" SDS	21740	26665	15655	19195	
111000.00 0002.0	U	J	J /4	10	12/6	77/4	11 /4 NZ /2 UDU	LO 74 NZ 72 0D0	96.71	118.62	69.64	85.39	
HHGU11.00-SDS2.5	3	11	51/4	13	12%	43/4	44-1/4"x21/2" SDS	28-1/4"x21/2" SDS	21740	26665	15655	19195	
11110011.00-3032.3	3		J /4	10	1278	4 74	44-74 AZ72 3D3	ZU-74 XZ/2 3D3	96.71	118.62	69.64	85.39	

- 1. Factored uplift resistances have been increased for earthquake and wind loading, with no further increase allowed.
- 2. Specify H dimension. The Designer should check the shear capacity of the carried member to make sure it matches the hanger's capacity. Maximum H = 30". 3. Header depth must exceed the ha dimension shown
- and is based on the size necessary to fit screw pattern. Use the next size up that meets the minimum depth requirement.

HCA Hinge Connectors

HCAs offer single-piece side plates, for fewer welds and higher horizontal resistances.

MATERIAL: Side plates-7 gauge;

Top and bottom plates-varies.

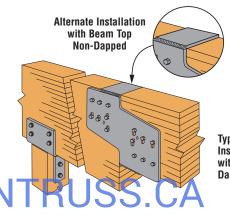
FINISH: Simpson Strong-Tie gray paint

INSTALLATION: • Use all specified fasteners. See General Notes.

- All bolts specified are 3/4" MBs. Bolt holes shall be a minimum of $\frac{1}{32}$ and a maximum of $\frac{1}{16}$ larger than the bolt diameter. (per 12.4.1.2 CSA 086-14)
- · Position bolts in slots away from bearing seat to allow for wood shrinkage.

OPTIONS:

• To order, add the width and bearing plate size designation after the model mension by the PT dimension for each dap



Typical HC4C3TA Installation with Beam Top Dapped

TOP FLANGE HANGERS LEG/MEG/EG Beam & Glulam Hangers



See Hanger Options on page 231 for hanger modifications, which may result in reduced capacities.

This whole series has precision fabrication which offers dimensional accuracy, and the funnel flanges which aid installation.

MATERIAL: See table

FINISH: Simpson Strong-Tie® gray paint. Hot-dip galvanized available; specify HDG.

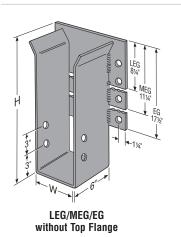
INSTALLATION: • Use all specified fasteners. See General Notes.

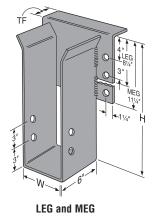
- Maintain minimum 4D end and edge distance from bolt to end of header or nearest loaded edge per CSA 086-14.
- Bolt holes in wood shall be a minimum of $\frac{1}{16}$ " to a maximum of $\frac{1}{16}$ " larger than the bolt hole per 12.4.1.2 and CSA 086-14.

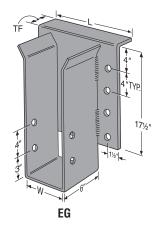
OPTIONS: • See Hanger Options, page 231.

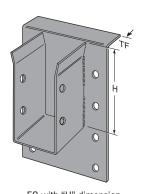
• Special models are available without top flanges; see table values.

Model	Top Flange Ga	Top Flange Length (L)		
LEG/MEG	7	12		
EG5		11¾		
EG7	3	13½		
EG9		15½		









EG with "H" dimension less than the face plate height. The EG's back plate is always 17½", regardless of the stirrup height.

				D	imension	ıs		Во	Its		Fac	tored Normal Re	esistance (K _D = 1.	00)	
Joist or			Min.		(in)		Hea	ader	Jo	ist	D.Fir-L	Glulam	Spruce-Pi	ne Glulam	
Purlin	Model No.	Stirrup Ga	Header Depth								No Top Flange	Top Flange	No Top Flange	Top Flange	
Size (in)			(in)	W	Min H	TF	Qty.	Dia.	Qty.	Dia.	lbs	lbs	lbs	lbs	
											kN	kN	kN	kN	
31/8	LEG3	7	101/2	31/4	9	21/2	4	3/4	2	3/4	5950	17510	5950	14490	
3 78	LEGS	,	10 72	374	9	2 72	4	94		94	26.47	77.89	26.47	64.46	
	LEG5	7	101/2	51/4	9	21/2	4	3/4	2	3/4	5950	19960	5950	17545	
	LEGS		10 72	574 9 272 4 94		2 /4	26.47	88.79	26.47	78.05					
51/8	MEG5	7	13	51/4	9	2½	6	3/4	2	3/4	7780	21785	7780	19370	
3 78	IVIEGO	'	13	3 7/4	9	2 72	0	74	2	74	34.61	96.91	34.61	86.17	
	EG5	7	21	51/4	11	21/2	8	1	2	1	13590	27305	13590	23765	
	EGS	1	21	374	11	2 72	0	'		l	60.45	121.46	60.45	105.72	
	LEG7	7	101/2	67/8	9	21/2	4	3/4	2	3/4	5950	19960	5950	17545	
	LEGI	_ ′	10 72	078	9	2 72	4	94		94	26.47	88.79	26.47	78.05	
63/44	MEG7	7	13	67/8	9	2½	6	3/4	2	3/4	7780	21785	7780	19370	
074	IVILGI	'	13	078	9	2 72	0	74		74	34.61	96.91	34.61	86.17	
	EG7	7	21	C 7/	11	2½	8	1	2	1	13590	29350	13590	26635	
	EUI	1	۷۱	6%	11	∠ 72	0	'		1	60.45	130.56	60.45	118.48	
81/24	EG9	7	21	87/8	11	21/2	8	1	2	1	4	13590	31685	13590	28565
0 72	EGS	/	21	078	- 11	∠72	0		2		60.45	140.95	60.45	127.07	

- 1. Factored resistances assume a minimum carrying member thickness of 5%.
- 2. Specify hanger height "H". "Min H" is the minimum height that may be ordered.
- 3. Minimum header depth below the lowest bolt hole is 3" for the LEG, MEG, and 4" for the EG.
- 4. For 6% and 8½" beam widths add "X" to the end of the model number and specify the width required.

TOP FLANGE HANGERS WPU/WNP/HW/HWU



The WPU, HWU and HW series purlin hangers offer the greatest design flexibility and versatility.

MATERIAL: WNP/WPU—7 ga. top flange, 12 ga. stirrup; HW—3 ga. top flange, 11 ga. stirrup; HWU—3 ga. top flange, 10 ga, stirrup.

FINISH: Simpson Strong-Tie® gray paint; hot-dipped galvanized available: specify HDG.

FACTORED RESISTANCES: For hanger heights exceeding the joist height, the factored resistance is 0.50 of the tabulated resistance.

INSTALLATION:

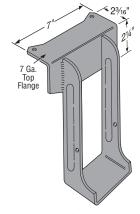
- Hangers may be welded to steel headers with %6" for WPU/WNP/ WP, and %" for HW/HWU, by 1½" fillet welds located at each end of the top flange. Weld-on applications produce maximum factored resistance listed. Uplift resistances do not apply to this application.
- Hangers can support multi-ply carried members; the individual members must be secured together to work as a single unit before installation into the hanger.
- H dimensions are sized to account for normal joist shrinkage. W dimensions are for dressed timber widths.

OPTIONS: • See Hanger Options, page 231, for hanger modifications and associated load reductions.

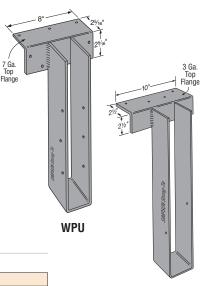
NAILER TABLE

The table indicates the maximum factored normal resistances for WP/WNP hanger used on wood nailers. Nailers are wood members attached to the top of a steel I-beam, concrete or masonry wall. This table also applies to sloped-seat hangers.

		Тор		red Resis K _D = 1.00	
Model	Nailer	Flange	D.Fir-L	S-P-F	LSL
		Nailing	lbs	lbs	lbs
			kN	kN	kN
	2x	2-10dx1½	3665	3630	4900
	_	Z-10UX 1 72	16.30	16.15	21.82
	2-2x	2-10d	4475	3760	_
WP/	Z-ZX	2-10u	19.91	16.75	_
WNP	3x	2-16dx21/2	4110	3760	_
	3x	Z-10UXZ72	18.28	16.75	_
	4x	2-10d	4475	3760	_
	4X	2-10u	19.91	16.75	_
	2-2x	7-10d	4475	3760	_
	Z-ZX	7-10u	19.91	16.75	_
WPU	3x	7-16dx2½	4110	3760	_
WFU	JX.	7-10UXZ 72	18.28	16.75	_
	4x	7-10d	4475	3760	_
	4X	7-10u	19.91	16.75	_
	2-2x	4-10d	7600	_	_
	2-2X	4-10u	33.81	_	_
HW	3x	4-16dx2½	7600	_	
ПVV	J SX	4-10UXZ72	33.81	_	_
	4x	4-16d	7670	_	_
	48	4-10u	34.16	_	_
	2-2x	8-10d	7880		
	Z-ZX	0-10u	35.05		
HWU	3x	8-16dx2½	7880		
пии	OX.	0-10UXZ 72	35.05		
	4x	8-16d	7880		
	48	0-10u	35.05	_	_



WNP412 and WNP414



Some model configurations may differ from those shown. Contact Simpson Strong-Tie for details.

HW

(HWU similar)

- Factored Resistance Joist **Fasteners** Uplift1 Normal $(K_D = 1.00)$ Model $(K_D = 1.15)$ D.Fir-L S-P-F LVL PSL LSL No. Width Depth Top Face Joist lbs lbs lbs lbs lbs lbs kN kΝ kΝ kΝ kΝ kΝ 4095 3345 4695 4720 3-10dx11/2 3½ to 30 1½ to 7½ 2-10dx11/2 18.22 14.88 20.89 21.00 4720 5980 WP/ 4095 3550 3665 1½ to 7½ 31/2 to 30 3-10d 2-10dx11/2 WNP 18.22 21.00 26.60 15.79 16.30 4430 5950 5430 5980 3855 1½ to 7½ 3½ to 30 3-16d 2-10dx11/2 19.71 24.15 17.15 26.47 26.60 1665 6390 6390 6825 7085 5980 71/4 to 18 3-16d 4-16d 6-10dx1½ 1¾ to 5½ 7.41 28.43 28.43 30.36 31.52 26.60 WPU 595 6390 6390 6825 7085 5980 4-16d 1¾ to 5½ 181/2 to 28 3-16d 6-10dx11/2 28.43 28.43 30.36 31.52 2.65 26.60 6900 5285 4695 5810 1½ to 7½ 31/2 to 32 4-10d 2-10dx11/2 30.69 23.51 20.89 25.85 HW 6900 5285 7695 5810 6870 1½ to 7½ 3½ to 32 4-16d 2-10dx11/2 30.69 23.51 34.23 25.85 30.56 10170 8325 8925 1775 8875 10170 1¾ to 3½ 9 to 18 4-16d 4-16d 6-10dx1½ 7.90 45.24 39.48 45.24 37.03 39.70 8325 1490 10170 8875 10170 8925 1¾ to 3½ 181/2 to 28 4-16d 4-16d 6-10dx11/2 6.63 45.24 39.48 45.24 37.03 39 70 1520 10170 8875 10170 8325 8925 4-16d 1¾ to 3½ 28½ to 32 4-16d 8-10dx11/2 6.76 45.24 39.48 45.24 37.03 39.70 HWU 1775 8250 8250 8250 8250 8250 4½ to 71/8 9 to 18 4-16d 4-16d 6-10dx11/2 36.70 7.90 36.70 36.70 36.70 36.70 1490 8250 8250 8250 8250 8250 181/2 to 28 4-16d 4-16d 6-10dx1½ 4½ to 7% 6.63 36.70 36.70 36.70 36.70 36.70 8250 8250 8250 8250 8250 1520 4-16d 4½ to 7½ 28½ to 32 8-10dx1 6.76 36.70 36.
- 1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase allowed. Reduce by 15% for standard term loading ($K_D = 1.00$) like cantilever construction.
- 2. Factored uplift resistances shown are for D.Fir-L. Multiply tablulated loads x 0.71 for either S-P-F joist or header.
- Factored resistances shown are for header connection only. The Designer must ensure the joist is capable of generating the factored resistances shown.
- 4. Applies to LVL headers made primarily from Douglas Fir or Southern Pine. For LVL made primarily from Spruce Pine Fir or similar less dense veneers, use the values found in the S-P-F column.
- 5. NAILS: 16d = 0.162" dia x 3½" long, 10d = 0.148" dia x 3" long, 10dx1½ = 0.148" dia. x 1½" long. See pages 22-23 for other nail sizes and information.

TOP FLANGE HANGERS GLS/HGLS/GLT/HGLT Beam & Glulam Saddle Hangers



GLT and HGLT accommodate typical structural requirements for timber and glulam beams. Top flange depth allows installation on minimum 4x ledger (3½" net). Funnel flanges allow easy installation of beams.

GLS and HGLS are heavy glulam saddle hangers.

MATERIAL: See table on page 104.

FINISH: Simpson Strong-Tie® gray paint. Hot-dip galvanized available; specify HDG.

INSTALLATION: • Use all specified fasteners. See General Notes. **GLT/HGLT**

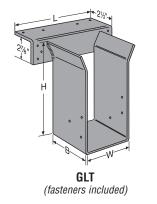
- All GLTs used with sawn timbers have a 12" L dimension.
- Fasteners are included.
- GLT may be attached to steel headers by 3/16" x 21/2" fillet welds at each end of the header angle to obtain the tabulated loads. HGLT may be attached to steel headers by 1/4" x 21/2" fillet welds at each end of the header angle to obtain the tabulated factored resistances. Factored uplift resistances do not apply to this weld-on application.

GLS/HGLS

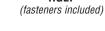
- N54A nails are included with the hangers.
- Minimum header width for saddle hangers is 51/4".
- Factored resistances listed are per stirrup.

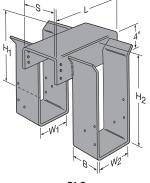
TO ORDER: GLS/HGLS—Specify H₁, H₂ and S dimensions (see illustrations). **OPTIONS:** See Hanger Options page 231.

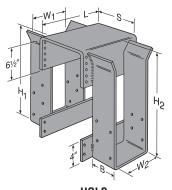
Model	Top Flange Ga	Stirrup Width (W)	Top Flange Length (L)
GLT	3	2% - 5½	10
GLI	<u>ى</u>	5% - 6%	12
HOLT	0	25/8 - 81/4	12
HGLT	3	81/8	14
		31/4	6
GLS	3	51/4	9
		61/8	12
HGLS	3	51/4 - 87/8	12











GLS (fasteners included)

HGLS (fasteners included)

TOP FLANGE HANGERS HHB/GB/HGB Beam & Purlin Hangers

Precision forming with manufacturing quality control provides dimensional accuracy and helps ensure proper bearing area and connection.

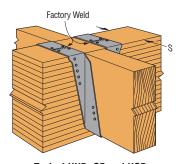
MATERIAL: See table on page 104.

FINISH: HHB, GB, HGB, all saddle hangers and all welded sloped and special hangers— Simpson Strong-Tie gray paint. HHB may be ordered hot-dip galvanized; specify HDG.

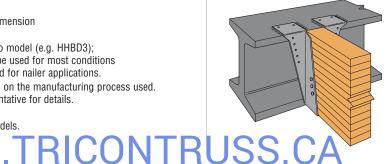
INSTALLATION:

- Use specified fasteners. See General Notes and nailer table.
- HHB, GB and HGB may be used for weld-on applications. The minimum required weld to the top flanges is 1/8" x 2" fillet weld to each side of each top flange tab for 14 and 12 gauge and $\frac{3}{16}$ " x 2" fillet weld to each side of each top flange tab for 7 gauge. Distribute the weld equally on both top flanges. Welding cancels the top and face nailing requirements. Consult the code for special considerations when welding galvanized steel. The area should be well-ventilated. Weld on applications produce the maximum factored resistance listed. Uplift resistances do not apply to welded applications.
- · Ledgers must be evaluated for each application separately. Check TF dimension, nail length and nail location on ledger.

- HHB-other widths are available; specify W dimension (the minimum W dimension is 21/2").
- Saddle hangers are made to order; add "D" to model (e.g. HHBD3); specify S (for saddle) dimension. They may be used for most conditions except at end wall locations, and are preferred for nailer applications.
- The coating on special B hangers will depend on the manufacturing process used. Check with your Simpson Strong-Tie representative for details. Hot-dip galvanized available: specify HDG.
- · B dimensions may be increased on some models.
- See Hanger Options, page 231.



Typical HHB, GB and HGB Saddle Installation



HHB, GB and HGB are acceptable for weld-on applications. See Instructions to the Installer, page 20, note m.

TOP FLANGE HANGERS – GLULAM BEAM



				Dimens	ions		F		Factored Resistance			
laiat au				(in)			Faste	eners	D.Fir-L	Glulam	Spruce-Pine Glulam	
Joist or Purlin	Model								Uplift	Normal	Uplift	Normal
Size	No.	Ga								$(K_D = 1.00)$	•	
(in)			W	Н	В	TF ⁹	Header	Joist	lbs	lbs	lbs	lbs
									kN	kN	kN	kN
									2905	9625	2060	5225
	GLT3	7	31/4	7½ Min.	5	21/2	10-N54A	6-N54A				
									12.92	42.82	9.16	23.24
	HGLT3	7	31/4	7½ Min.	6	21/2	18-N54A	6-N54A	2905	14885	2060	9830
									12.92	66.21	9.16	43.73
	GLS3-5	7	31/4	8½ Min.	5	51/4	6-N54A	6-N54A	2905	16740	2060	13195
									12.92	74.47	9.16	58.70
	GLS3-7	7	31/4	8½ Min.	5	67/8	6-N54A	6-N54A	2905	16740	2060	13195
31/8 LAM		Ľ		072		0,0	0		12.92	74.47	9.16	58.70
070 E7111	GLS3-9	7	31/4	8½ Min.	5	87/8	6-N54A	6-N54A	2905	16740	2060	13195
	GL00 3		074	072 Willi.		078	0 110-171	0 110-171	12.92	74.47	9.16	58.70
	HW3.25	11	31/4	5 Min.	4	2½	4-10d	2-10d	_	6900	_	5285
	11003.23	''	3 /4	J IVIIII.	4	2/2	4-10u	2-10u	_	30.69	_	23.51
	HHB3	7	01/	71/ Min	3	2½	10 NE 4A	C NE 4A	3340	8575	2370	6085
	ппвз	/	31/4	7½ Min.	3	21/2	10-N54A	6-N54A	14.86	38.15	10.54	27.07
	0.00	_	047	74 (8.8)	04 /	047	44 NE 44	0.115.44	3340	12935	2370	9710
	GB3	7	31/4	7½ Min.	3½	2½	14-N54A	6-N54A	14.86	57.54	10.54	43.19
									2905	9625	2060	5225
	GLT5	7	51/4	7½ Min.	5	21/2	10-N54A	6-N54A	12.92	42.82	9.16	23.24
									2905	14885	2060	9830
	HGLT5	7	51/4	7½ Min.	6	2½	18-N54A	6-N54A	12.92	66.21	9.16	43.73
									2905	20190	2060	14365
	GLS5-5	7	51/4	8½ Min.	5	51/4	6-N54A	6-N54A	12.92	89.81	9.16	63.90
									2905	20190	2060	14365
	GLS5-7	7	51/4	8½ Min.	5	67/8	6-N54A	6-N54A	12.92	89.81	9.16	63.90
51/8 LAM	HGLS5	7	51/4	10½ Min.	6	SPEC	14-N54A	8-N54A	4095	27570	2905	19575
									18.22	122.64	12.92	87.08
	HW5.25	11	51/4	5 Min.	2½	21/2	4-10d	2-10d		6900		5285
									_	30.69		23.51
	HHB5	7	51/4	7½ Min.	3	2½	10-N54A	6-N54A	3340	8575	2370	6085
									14.86	38.15	10.54	27.07
	GB5	7	51/4	7½ Min.	3½	2½	14-N54A	6-N54A	3340	13675	2370	9710
		Ľ	074	7 /2 101111.	0,2	-/-	11110111	0 110 171	14.86	60.83	10.54	43.19
	HGB5	7	51/4	7½ Min.	4	2½	14-N54A	6-N54A	3340	16050	2370	11395
	Паво	'	074	7 72 WIIII.		L /2	IT NOTA	O NOTA	14.86	71.40	10.54	50.69
	шшр7	7	67/	71/4 Min	3	2½	10-N54A	6-N54A	3340	8575	2370	6085
	HHB7	'	67/8	7½ Min.	3	272	10-N54A	6-N54A	14.86	38.15	10.54	27.07
	0.07	7	C7/	71/ 84:	01/	01/	14 NE 44	C NE 4A	3340	13675	2370	9710
	GB7	7	67/8	7½ Min.	3½	2½	14-N54A	6-N54A	14.86	60.83	10.54	43.19
	11007	_	07/	74 (8.8)		047	44 NE 44	0.1544	3340	16050	2370	11395
	HGB7	7	67/8	7½ Min.	4	21/2	14-N54A	6-N54A	14.86	71.40	10.54	50.69
									2905	9625	2060	5225
	GLT7	7	67/8	7½ Min.	5	2½	10-N54A	6-N54A	12.92	42.82	9.16	23.24
6¾ LAM									2905	14885	2060	9830
	HGLT7	7	6%	7½ Min.	6	21/2	18-N54A	6-N54A	12.92	66.21	9.16	43.73
									2905	20190	2060	14365
	GLS7-7	7	67/8	8½ Min.	5	61/8	6-N54A	6-N54A	12.92	89.81	9.16	63.90
									2905	20190	2060	14365
	GLS7-9	7	6%	8½ Min.	5	81/8	6-N54A	6-N54A				
									12.92	89.81	9.16	63.90
	HGLS7	7	6%	10½Min.	6	SPEC	14-N54A	8-N54A	4095	27570	2905	19575
									18.22	122.64	12.92	87.08
	HGLT9	7	87/8	7½ Min.	6	21/2	18-N54A	6-N54A	2905	14885	2060	9830
8¾ LAM									12.92	66.21	9.16	43.73
	HGLS9	7	87/8	10½ Min	6	SPEC	14-N54A	8-N54A	4095	27570	2905	19575
			M		1 A				18.22	122.64	12.92	87.08

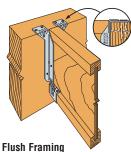
- 1. N54A fasteners are supplied with hangers.
- 2. Factored uplift resistances have been increased 15% for earthquake or wind loading with no further increase allowed. Reduce by 15% for standard term loading $(K_D = 1.00)$ such as in cantilever construction.
- 3. GLT, HGLT, GLS, HGLS uplift resistances only apply when "H" is 28" or less.
- 4. Factored resistances for glulam sizes are based on 812 psi (5.6 MPa) D.Fir-L and 672 psi (4.64 MPa) Spruce-Pine wood bearing (φFcp).
- 5. GLS and HGLS loads must be distributed evenly on each side of the header, as they are saddle-style hangers.
- 6. GLS and HGLS fasteners listed are for one side only. Fasteners supplied are for both sides of the saddle.
- 7. For attachment to SCL, see GLTV/HGLTV on page 123.
- 8. Resistances shown are for each side of the hanger for GLS and HGLS.
- 9. For saddle hangers dimension shown is "S". Minimum "S" is 51/4".
- 10. **NAILS:** 10d = 0.148" dia. x 3" long, N54A = 0.250" dia. x 21/2" long – annular ring. See pages 22-23 for other nail sizes and information.

Glulam Beam Connectors

GENERAL CONNECTOR INSTALLATION



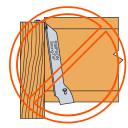
TOP FLANGE HANGERS



Top flange configuration and thickness of top flange need to be considered for flush frame conditions.

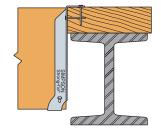


Hanger Over-Spread
If the hanger is over-spread, it can
raise the I-joist above the header
and may cause uneven surfaces and
squeaky floors.



Hanger Not Plumb A hanger "kicked-out" from the header can cause uneven surfaces and squeaky floors.

WOOD NAILERS

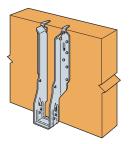


Correct Attachment

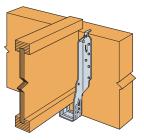


Nailer Too Wide
The loading may cause
cross-grain bending. As
a general rule, the maximum
allowable overhang is 1/4",
depending on nailer thickness.

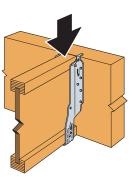
IUS INSTALLATION SEQUENCE



<u>STEP 1</u> Attach the IUS to the header



STEP 2 Slide the I-joist downward into the IUS until it rests above the large teardrop.



STEP 3
Firmly push or snap
I-joist fully into the seat
of the IUS.

WOOD I-JOISTS

SLOPED JOISTS

For sloped joists up to $\frac{1}{2}$:12 there is no reduction in capacity. For slopes greater than $\frac{1}{2}$:12 see individual product pages.

MULTIPLE JOISTS

Multiple joists should be adequately connected together to act as one unit.

FASTENERS

Use the correct nails. Wood may split if the nails are too large. Hanger nails into flanges should not exceed 10d common (0.148 dia.), no longer than 1½". Nails into web stiffeners should not exceed 16d commons (0.162 dia.).

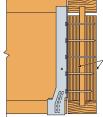
ECCENTRICALLY-LOADED I-JOISTS

Supporting a top flange hanger may require bottom flange restraining straps, blocking or directly-applied ceiling systems to prevent rotation at the hanger location.

SKEWED JOISTS

Joists may be skewed up to $2\frac{1}{2}$ degrees in a non-skewed hanger without any reduction in capacity. Refer to individual hanger descriptions for information allowing any further skew applications.

I-JOIST AS A HEADER INSTALLATIONS



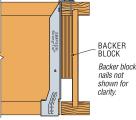
BACKER
BLOCK
EACH SIDE

Backer block
nails not
shown for
clarity.

Face Mount Hanger

When face mount hangers are attached to I-joist headers, backer blocks must be installed to provide a nailing surface for the hanger nails. The backer blocks should be installed on both sides of the web and attached together with a minimum of 10-10d nails. The hanger nails should extend through the web. Contact the I-Joist manufacturer for additional design considerations.

I-JOIST HEADERS

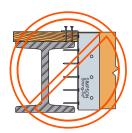


When top flange hangers are attached to I-joist headers, a backer block must be installed to prevent the top flange from rotating under load. The backer blocks should be installed with a minimum of 10-10d nails clinched. Check with the joist manufacture for additional design considerations.

V. TRICONTRUSS CA



Nailer Too Narrow Nailer should be full width.



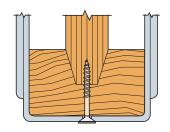
Nailer Too Thin or the wrong hanger for the application.

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GENERAL CONNECTOR INSTALLATION



LF & LT INSTALLATION



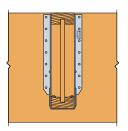
Use 8 gauge (0.164" diameter) x 1¼ wood screw (#8x1¼") to secure joist to hanger. (Two screws required for joist widths > 2½".)

To avoid stripping of the bottom chord screw hole, DO NOT over tighten screw.

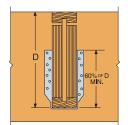
Use specified screw to seat joist into hanger (required only for LF and LT hangers).

PREVENT ROTATION

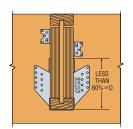
Hangers provide some joist rotation resistance; however, additional lateral restraint may be required for deep joists.



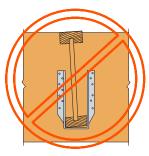
Rotation Prevented by Lateral Flange Support Side of hanger laterally support the top flange of the I-joist. No web stiffeners required!



Rotation Prevented by Web Stiffeners Hanger height should be at least 60% of joist height.



Rotation Prevented by Web Stiffeners If hanger height is less than 60% of the joist height, add clips or blocking near the top.



No Rotation Resistance
Lack of web stiffeners
combined with short hanger
allows unwanted rotation.

POSITIVE ANGLE NAILING



Approx. 45° angle

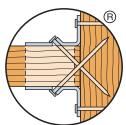


Nail at wrong angle



Nail too long

DOUBLE SHEAR NAILING



The nail is installed into joist and header, distributing load through two points on each nail for greater strength. Do not use hangers with double shear nailing with I-joists.

TOE-NAILING



Toe nailing causes squeaks and improper hanger installations. Do not toe nail I-joists before installing top flange or face mount hangers.

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IUS/LF/MIU 1-Joist Hangers



This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

The improved IUS is now fully compatible with shallow flange I-joists! I-joists with flange thicknesses between 1½" and 1½" achieve the full tabulated factored resistances including uplift values and joist nails are not required! The IUS is a hybrid hanger that incorporates the advantages of the face mount and top mount hanger. Installation is fast with the Strong-Grip™ seat, easy-to-reach face nails and self-jigging locator tabs.

The MIU series hangers are designed for commercial and high load I-joist applications without requiring web stiffeners. The MIU features Positive Angle Nailing (PAN), which minimizes splitting of the flanges while permitting time-saving nailing from a better angle.

The LF series is ideal for applications not requiring web stiffeners. The economical LF series comes with a height designed to support the top flange of the I-joist. This feature reduces installation time as well as material costs.

MATERIAL: See table pages 109-113.

FINISH: Galvanized

UPLIFT RESISTANCES: • Models have optional triangle joist nail holes for additional uplift. Properly attached web stiffeners are required.

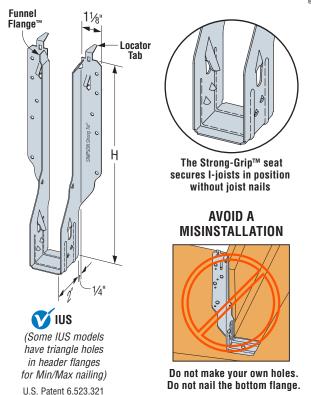
- LF/IUS—add two additional 10dx1 $\frac{1}{2}$ " joist nails for a total factored uplift resistance of 415 lbs D.Fir-L and 375 lbs S-P-F (K_D = 1.15).
- MIU—add four additional 10dx1½" joist nails for a total factored uplift resistance of 1345 lbs D.Fir-L and 1175 lbs S-P-F ($K_D = 1.15$).

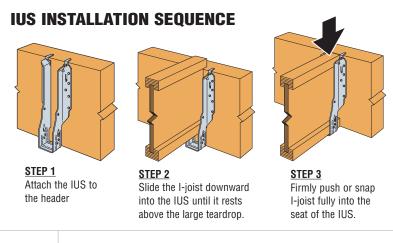
INSTALLATION: • Use all specified fasteners. Verify that the header can take the required fasteners specified in the table. See pages 105-106 for more installation information.

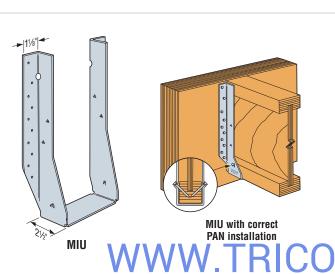
- IUS—fasten hanger to header. Position I-joist into hanger and snap into place. No joist nailing required. Some IUS models have triangle and round header nail holes. To achieve Max. download, fill both round and triangle holes.
- IUS—Locator tabs are not structural. They may be bent back to adjust for hanger placement.
- IUS— I-joists with web stiffeners or rectangular sections can be used with the installation of 2-10dx1½" nails into the optional triangle joist nails.
- Web stiffeners are not required with I-joists when the joist top flange is laterally supported by the sides of the hanger. I-joist manufacturers may require web stiffeners.

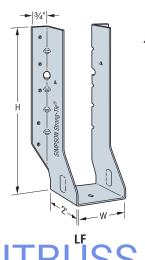
OPTIONS:

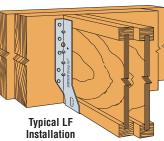
These hangers cannot be modified. However, these models will normally accommodate a skew of up to 5°. For a sloping joist to %:12, tests show a 10% reduction in ultimate hanger strength. Local crushing of the bottom flange or excessive deflection may be limiting; check with joist manufacturer for specific limitations on bearing of this type.

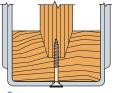














FACE MOUNT HANGERS U/HU 1-Joist & Structural Composite Lumber Hangers

U—The standard U hanger provides flexibility of joist to header installation. Versatile fastener selection with tested factored resistances.

HU—Most models have triangle and round holes. To achieve maximum factored resistances, fill both round and triangle holes with common nails. These heavy-duty connectors are designed for schools and other structures requiring additional strength, longevity and safety factors.

MATERIAL: See tables on pages 109-116.

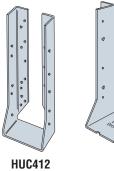
FINISH: Galvanized

INSTALLATION: • Use all specified fasteners. See General Notes.

- HU-can be installed filling round holes only, or filling round and triangle holes for maximum values.
- · Web Stiffeners are required for all I-joists used with these hangers.

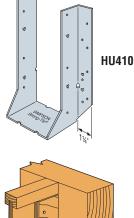
OPTIONS: • HU hangers available with the header flanges turned in for 25/16" and larger widths, with no reduction in resistances—order HUC hanger.

· See Hanger Options on page 230 for sloped and/or skewed U/HU models, and HUC (concealed flange) models.

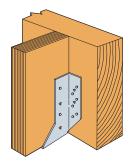




Typical HU Installation



Model configurations may differ from those shown. Some HU models do not have triangle holes. Contact Simpson Strong-Tie for details.



Typical HU Installation

FACE MOUNT HANGERS HUS/HHUS/HGUS Double Shear SCL Hangers

These hangers are designed for applications where higher factored resistances are needed.

All hangers in this series have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation, and the use of common nails for all connections. (Do not bend or remove tabs)

MATERIAL: See tables, pages 114-116.

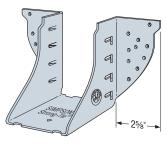
FINISH: Galvanized. Some products available in stainless steel or ZMAX®; see Corrosion Information, pages 14-17.

INSTALLATION: • Use all specified fasteners. See General Notes.

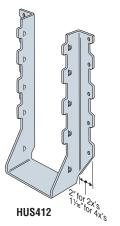
- Do not use double shear hangers with I-joists.
- Nails must be driven at an angle through the joist into the header to achieve the tabulated values.
- Not designed for welded or nailer applications.
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- With 3x carrying members, use 16dx21/2" nails into the header and 16d commons into the joist with no reduction in resistances. With 2x carrying members, use 10dx11/2" nails into the header and 10d commons into the joist, and reduce the tabulated factored resistance to 0.64 of the table value.

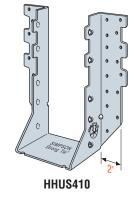
OPTIONS: • HUS hangers available with the header flanges turned in for 4x (3½") only, with no reduction in resistances. See HUSC Concealed Flange illustration.

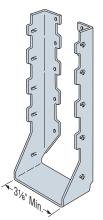
- · Concealed flanges are not available for HGUS, HHUS and HUS1.81/10.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See hanger options on page 230.



HGUS46







HUSC **Concealed Flanges** (not available for HHUS. HGUS and HUS1.81/10)





Double Shear Nailing Side View Do not bend tab back



Dome Double Shear Nailing prevents tabs breaking off (available on some models)

U.S. Patent 5,603,580

FACE MOUNT HANGERS – I-JOISTS



				D	imensio	ns		Faste	nare			Resistance	
		Web			(in)			rasiei	iers		ir-L	S-	
Joist	Model	Stiff	Ga							Uplift	Normal	Uplift	Normal
Size	No.	Reqd		w	н	В	Min/	Header	Joist	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
							Max			lbs	lbs	lbs	lbs
										kN 105	kN 0405	kN	kN
447	LF159	_	18	1 %16	91/4	2	_	10-10d	1-#8x11/4 WS	0.47	2435 10.83	105 0.47	1845 8.21
1½ x 9¼ - 9½										375	3045	375	2305
J/4 J/2	MIU1.56/9	_	16	1 %16	815/16	21/2	_	16-16d	2-10dx1½	1.67	13.56	1.67	10.27
										105	2435	105	1845
1½ x	LF1511	_	18	1%16	111/4	2	-	12-10d	1-#8x11/4 WS	0.47	10.83	0.47	8.21
111/4 - 117/8										375	3045	375	2305
	MIU1.56/11	-	16	1%16	111/16	2½	-	20-16d	2-10dx1½	1.67	13.56	1.67	10.27
41/ 4.4	1.54544		10	40/	101/	_		11 10 1	4 //041/ M/C	105	2435	105	1845
1½ x 14	LF1514	_	18	1%16	13½	2	-	14-10d	1-#8x11/4 WS	0.47	10.83	0.47	8.21
1¾ x 9½	IUS1.81/9.5	_	18	1%	9½	2		8-10d		175	2385	175	1690
1/4 A 3/2	1001.01/9.0		10	178	372			0-10u		0.78	10.61	0.78	7.52
	LF179	l _	18	113/16	91/4	2	_	10-10d	1-#8x11/4 WS	105	2525	105	2155
1¾ x										0.47	11.23	0.47	9.60
91/4 - 91/2	MIU1.81/9	_	16	1 13/16	813/16	2½	_	16-16d	2-10dx1½	375	3555	375	2690
										1.67	15.84	1.67	11.98
	IUS1.81/11.88	_	18	11//8	111//8	2	—	10-10d	_	175 0.78	2565 11.41	175 0.78	1820 8.10
										105	2845	105	2155
1¾ x 11¾	LF1711	—	18	1 13/16	111/4	2	—	12-10d	1-#8x11/4 WS	0.47	12.66	0.47	9.60
										375	3555	375	2690
	MIU1.81/11	-	16	1 13/16	11 1/16	2½	-	20-16d	2-10dx1½	1.67	15.84	1.67	11.98
										175	2565	175	1820
			40	47/			Min	12-10d	_	0.78	11.41	0.78	8.10
	IUS1.81/14	_	18	1%	14	2	N/a	11101		175	2725	175	1935
1¾ x 14							Max	14-10d	_	0.78	12.12	0.78	8.61
174 X 14	LF1714		18	1 13/16	13½	2	_	14-10d	1-#8x1½ WS	105	2845	105	2155
	LI 1/ 14		10	1 /10	1072			14 100	1 # 0X174 WO	0.47	12.66	0.47	9.59
	MIU1.81/14	_	16	1 13/16	135/16	2½	_	22-16d	2-10dx1½	375	3555	375	2690
										1.67	15.84	1.67	11.98
	IUS1.81/16	_	18	1%	16	2	_	14-10d	_	175	2725	175	1935
1¾ x 16										0.78 375	12.12 3555	0.78 375	8.61 2690
	MIU1.81/16	-	16	1 13/16	155/16	21/2	—	24-16d	2-10dx1½	1.67	15.84	1.67	11.98
										175	2385	175	1690
	IUS2.06/9.5	_	18	21/8	9½	2	-	8-10d	_	0.78	10.61	0.78	7.52
2 x 9½	1 5000		40	01/	01/			40.40.1	4 (0.41/ 100	105	2525	105	2155
	LF209	_	18	21/16	91/4	2	-	10-10d	1-#8x11/4 WS	0.47	11.23	0.47	9.60
	IUS2.06/11.88	_	18	21/8	1111//8	2	_	10-10d		175	2565	175	1820
2 x 11%	1032.00/11.00		10	2/8	1178			10-100		0.78	11.41	0.78	8.10
2 X 1170	LF2011	_	18	21/16	1111/4	2	_	12-10d	1-#8x11/4 WS	105	2880	105	2270
						_				0.47	12.81	0.47	10.11
							Min	12-10d	_	175	2565	175	1820
	IUS2.06/14	_	18	21/8	14	2				0.78 175	11.41 2725	0.78 175	8.10 1935
2 x 14							Max	14-10d	_	0.78	12.12	0.78	8.61
										105	3235	105	2385
	LF2014	-	18	21/16	13½	2	-	14-10d	1-#8x11/4 WS	0.47	14.39	0.47	10.61
010	11100 00 42		40	017	40	_		44.40.1		175	2725	175	1935
2 x 16	IUS2.06/16	_	18	21/8	16	2	_	14-10d	_	0.78	12.12	0.78	8.61
	IUS2.06/9.5	_	18	21/8	9½	2	_	8-10d	_	175	2385	175	1690
	1002.00/3.0		10	∠78	372		\perp	0-10U		0.78	10.61	0.78	7.52
21/16 x 91/2	LF219	_	18	21//8	815/16	2	_	10-10d	1-#8x11/4 WS	105	2525	105	2155
E/10 X 3/2				_/0	3 / 10			10 100	. # 5/(1/4 1/0	0.47	11.23	0.47	9.60
	HU2.1/9	✓	14	21/8	9	2½	_	14-16d	6-10dx1½	1470	5465	1360	4225
ļ.										6.54	24.31	6.05	18.79



^{1. 10}d common nails may be used instead of the specified 16d nails at 0.83 of the tabulated value.

^{2.} Uplift loads have been increased 15% for earthquake or wind 2. Opint loads have been incleased 15% for earlingake of which loading with no further increase allowed. Reduce by 15% for standard term loading (Kp = 1.00) such as in capitile ver construction.

3. MIN nailing quantity and load values—fill all round holes; MAX nailing quantity and load values—fill all round and triangle holes.

^{4.} D-Fir.L factored resistances can be used for most LVL. Verify with manufacturer prior to selecting hanger.

 ^{4.} D-Fir.L factored resistances can be used for most LVL. Verify with manufacturer prior to select 5. Web stiffeners are required when top flange isn't supported laterally by the hanger.
 6. Web stiffeners are required when supporting double I-joists with flanges less than 15% thick.
 7. For 16 and 18 gauge, 3½ wide I-joist hangers, web stiffeners are required when the factor of reaction is greater han 2330 lbs. (10.36 kN).
 8. NAILS: 16d = 0.162 dia. x 3½ non., 10 d = 0.148 dia. x 3 long. 10 dx1½ = 0.148 dia. x 1½ nong. See pages 22-23 for other nail sizes and information.

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FACE MOUNT HANGERS – I-JOISTS



				D	Dimensions (in)			Faster	nare		Factored I	Resistance	
		Web			(in)			1 45151	1613		ir-L		P-F
Joist	Model	Stiff	Ga							Uplift	Normal	Uplift	Normal
Size	No.	Reqd		w	н	В	Min/	Header	Joist	(K _D = 1.15)	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
							Max			lbs	lbs	lbs	lbs
										kN	kN	kN	kN
	IUS2.06/11.88	—	18	21/8	111//8	2	_	10-10d	_	175	2565	175	1820
										0.78 105	11.41 2880	0.78 105	8.10 2270
<u>į</u>	LF2111	_	18	21/8	111/4	2	_	12-10d	1-#8x11/4 WS	0.47	12.81	0.47	10.11
21/16 x 111//8										375	4550	375	3230
	MIU2.1/11	_	16	21/8	1111/16	2½	_	20-16d	2-10dx1½	1.67	20.24	1.67	14.37
										1470	5465	1360	4225
	HU2.1/11	✓	14	21/8	11	21/2	_	16-16d	6-10dx1½	6.54	24.31	6.05	18.79
							N.41	40.40.1		175	2565	175	1820
	11100 06/44		10	01/	14	0	Min	12-10d	_	0.78	11.41	0.78	8.10
2½16 x 14	IUS2.06/14	-	18	21/8	14	2	Max	14-10d		175	2725	175	1935
2716 X 14							IVIAX	14-10u		0.78	12.12	0.78	8.61
i .	LF2114	_	18	21/8	13%	2	_	14-10d	1-#8x1½ WS	105	3235	105	2385
	212114		10	278	10710			14 100	1 // 0/174 110	0.47	14.39	0.47	10.61
2½ x 16	IUS2.06/16	_	18	21/8	16	2	_	14-10d	_	175	2725	175	1935
										0.78	12.12	0.78	8.61
	IUS2.37/9.5	_	18	27/16	9½	2	_	8-10d	_	175	2385	175	1690
										0.78	10.61 2525	0.78	7.52 2155
	LF239	_	18	23/8	91/4	2	_	10-10d	1-#8x11/4 WS	105 0.47	11.23	105 0.47	9.60
										375	4550	375	3230
	MIU2.37/9	_	16	23/8	9	21/2	_	16-16d	2-10dx1½	1.67	20.24	1.67	14.37
25/16 x 91/2					_	_				1345	4355	1235	3090
	U3510/14	✓	16	25/16	9	2	_	14-16d	6-10dx1½	5.98	19.37	5.49	13.75
							N dia	11101	0.40441/	1470	5780	1360	4225
	HU359/	1	14	2%	815/16	2½	Min	14-16d	6-10dx1½	6.54	25.71	6.05	18.79
	HUC359	•	14	278	0.916	Z 72	Max	18-16d	10-10dx1½	2450	5780	2265	4690
							Ινιαλ	10 100	10 10ux172	10.90	25.71	10.08	20.86
	IUS2.37/11.88	_	18	27/16	117/8	2	_	10-10d	_	175	2565	175	1820
	1002101711100					_				0.78	11.41	0.78	8.10
	LF2311	_	18	23/8	1111/4	2	_	12-10d	1-#8x11/4 WS	105	2880	105	2270
										0.47	12.81 4550	0.47	10.11 3230
	MIU2.37/11	_	16	23/8	111/16	21/2	_	20-16d	2-10dx1½	375 1.67	20.24	375 1.67	14.37
25/16 x 117/8										1345	4355	1235	3095
	U3516/20	✓	16	25/16	10%16	2	_	16-16d	6-10dx1½	5.98	19.37	5.49	13.77
										1470	5780	1360	4225
	HU3511/			02/	4447	01/	Min	16-16d	6-10dx1½	6.54	25.71	6.05	18.79
	HUC3511	✓	14	2%	1111/16	2½	Max	00.164	10-10dx1½	2450	5780	2265	4690
							Max	22-16d	10-100X172	10.90	25.71	10.08	20.86
							Min	12-10d	_	175	2565	175	1820
	IUS2.37/14	_	18	27/16	14	2		12 100		0.78	11.41	0.78	8.10
						_	Max	14-10d	_	175	2725	175	1935
										0.78	12.12	0.78	8.61
	LF2314	_	18	23/8	13½	2	_	14-10d	1-#8x11/4 WS	105	3235	105	2385
25/16 x 14										0.47 375	14.39 4695	0.47 375	10.61 3485
	MIU2.37/14	—	16	23/8	13½	21/2	-	22-16d	2-10dx1½	1.67	20.91	1.67	15.52
										1960	5780	1810	4690
	HU3514/	,			,		Min	18-16d	8-10dx1½	8.72	25.71	8.05	20.86
	HUC3514	✓	14	2%	13½	21/2		04.40.1	40.40 447	2940	5780	2695	5780
							Max	24-16d	12-10dx1½	13.08	25.71	11.99	25.71
	IUS2.37/16	_	18	27/16	16	2	_	14-10d		175	2725	175	1935
	1032.37/10		10	∠716	10			14-100	_	0.78	12.12	0.78	8.61
25/16 x 16	MIU2.37/16	_	16	23/8	15½	2½	_	24-16d	2-10dx1½	375	4695	375	3485
2710 X TU	101102.07/10		10	2/0	1072	-/-		_ T 100	L IVUNI/2	1.67	20.91	1.67	15.52
	HU3516/22	✓	14	2%	141/4	21/2	_	20-16d	8-10dx1½	1960	5780	1810	4690
	1	<u> </u>								8.72	25.71	8.05	20.86

See footnotes on page 109.

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FACE MOUNT HANGERS – I-JOISTS



				Di	imensio	ns		Faster	ners			Resistance	
		Web			(in)			1 43101	1013	D.F		S-I	
Joist	Model	Stiff	Ga							Uplift	Normal	Uplift	Normal
Size	No.	Reqd		w	Н	В	Min/	Header	Joist	(K _D = 1.15)	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
							Max			lbs	lbs	lbs	lbs
										kN 375	kN 4695	kN 375	kN 3485
	MIU2.37/18	_	16	23/8	17½	2½	_	26-16d	2-10dx1½	1.67	20.91	1.67	15.52
										1960	5780	1810	4690
25/16 x 18	HU3524/30/						Min	18-16d	8-10dx1½	8.72	25.71	8.05	20.86
	HUC3524/30	✓	14	25/16	18	21/2				3430	5780	2695	5780
							Max	24-16d	14-10dx1½	15.26	25.71	11.99	25.71
05/ 1/ 00	MILIO 27/20	_	16	03/	101/	01/		00.164	2-10dx1½	375	4695	375	3485
25/16 x 20	MIU2.37/20		16	2%	19½	2½	_	28-16d	Z-100X172	1.67	20.91	1.67	15.52
							Min	18-16d	8-10dx1½	1960	5780	1810	4690
25/16 x 22-30	HU3524/30/	✓	14	25/16	18	2½	IVIIII	10 100	0 100X172	8.72	25.71	8.05	20.86
	HUC3524/30						Max	24-16d	14-10dx1½	3430	5780	2695	5780
										15.26 175	25.71 2385	11.99 175	25.71 1690
2½ x 9½	IUS2.56/9.5	—	18	25/8	9½	2	-	8-10d	_	0.78	10.61	0.78	7.52
	LF259		10	09/	91/4	2		10 104	1 #0v11/ MC	105	2525	105	2155
	LF209		18	2%16	974		_	10-10d	1-#8x1¼ WS	0.47	11.23	0.47	9.60
2½ x	MIU2.56/9	_	16	2%16	815/16	2½	_	16-16d	2-10dx1½	375	4550	375	3230
91/4 - 91/2			_							1.67	20.24 5780	1.67	14.37
	HU310/ HUC310	✓	14	29/16	81/8	21/2	—	14-16d	6-10dx1½	1470 6.54	25.71	1360 6.05	4225 18.79
01/ 117/			40	05/	447/	_		10.10.1		175	2565	175	1820
2½ x 11%	IUS2.56/11.88	_	18	25/8	11%	2	_	10-10d	_	0.78	11.41	0.78	8.10
	LF2511	_	18	2%16	111/4	2	_	12-10d	1-#8x11/4 WS	105	2880	105	2270
0.7						_				0.47	12.81	0.47	10.11
2½ x 11¼ - 11%	MIU2.56/11	_	16	2%16	11 ½16	21/2	_	20-16d	2-10dx1½	375 1.67	4550 20.24	375 1.67	3230 14.37
1174 1176	HU312/		4.4	00/	105/	01/		10.10.1	0.401.41/	1470	5780	1360	4225
	HUC312	√	14	2%16	10%	2½	_	16-16d	6-10dx1½	6.54	25.71	6.05	18.79
							Min	12-10d	_	175	2565	175	1820
	IUS2.56/14	_	18	25/8	14	2				0.78 175	11.41	0.78	8.10 1935
							Max	14-10d	_	0.78	2725 12.12	175 0.78	8.61
01/ 4.4	1.50544		40	00/	401/			44404	4 //0 41/ 14/0	105	3235	105	2385
2½ x 14	LF2514		18	2%16	13½	2	_	14-10d	1-#8x11/4 WS	0.47	14.39	0.47	10.61
	MIU2.56/14	_	16	29/16	137/16	2½	_	22-16d	2-10dx1½	375	4930	375	3485
	1111014/									1.67 1960	21.96 5780	1.67 1810	15.52 4690
	HU314/ HUC314	✓	14	29/16	12¾	21/2	_	18-16d	8-10dx1½	8.72	25.71	8.05	20.86
	IIICO EC/1C		10	25/8	16	2		14-10d		175	2725	175	1935
	IUS2.56/16		18	278	10			14-10u	_	0.78	12.12	0.78	8.61
2½ x 16	MIU2.56/16	_	16	2%16	157/16	2½	_	24-16d	2-10dx1½	375	4930	375	3485
	HU316/									1.67 1960	21.96 5780	1.67 1810	15.52 4690
	HUC316	√	14	29/16	141/8	2½	-	20-16d	8-10dx1½	8.72	25.71	8.05	20.86
2½ x 18	MIU2.56/18		16	29/16	177/.	2½		26-16d	2-10dx1½	375	4930	375	3485
272 X 10	WIIU2.30/10		10	Z716	177/16	2 72	_	20-10u	Z-100X172	1.67	21.96	1.67	15.52
2½ x 20	MIU2.56/20	_	16	29/16	197/16	21/2	_	28-16d	2-10dx1½	375	4930	375	3485
										1.67 375	21.96 4930	1.67 375	15.52 3485
2½ x 22 - 26	MIU2.56/20	✓	16	29/16	197/16	2½	_	28-16d	2-10dx1½	1.67	21.96	1.67	15.52
	LEO 150	_	10	21/	01/	2	_	10 104	2-#8x1½ WS	105	2525	105	2150
	LF2-159		18	31/8	91/4			10-10d	2-#0X174 VV3	0.47	11.23	0.47	9.60
	MIU3.12/9		16	31/8	91/16	21/2	_	16-16d	2-10dx1½	375	4550	375	3230
3 x 91/4 - 91/2										1.67 1580	20.24 5780	1.67 1470	14.37 4225
	HU210-2 /				040:	64.	Min	14-16d	6-10d	7.03	25.71	6.54	18.79
	HUC210-2	✓	14	31/8	813/16	2½	Max	10 164	10-10d	2635	5780	2450	4690
							Max	18-16d	10-100	11.72	25.71	10.90	20.86
	LF2-1511	_	18	31/8	111/4	2	_	12-10d	2-#8x11/4 WS	105	2880	105	2270
										0.47 375	12.81 4550	0.47 375	10.11 3230
3 x	MIU3.12/11	-	16	31/8	111//8	2½	_	20-16d	2-10dx1½	1.67	20.24	1.67	14.37
111/4 - 117/8							Min	16-16d	6-10d	1580	5780	1470	4225
	HU212-2/	1	14	_31/8	10%6	21/2	IVIIII	10 100	3 100	7.03	25.71	6.54	18.79
	HUC212-2	///		ΛĬ			Max	22-16d	10-100	2635	5780 25.71	2450	4690
	V	VV	V							11.72	25.71	10.90	20.86

FACE MOUNT HANGERS – I-JOISTS



'														8
					Di	mension	18		Faster	iers		Factored F		
	laiat	Madal	Web			(in)					D.F			P-F Normal
	Joist Size	Model No.	Stiff	Ga				Min/			Uplift (K _D = 1.15)	Normal (K _D = 1.00)	Uplift (K _D = 1.15)	Normal (K _D = 1.00)
	0.20		Reqd		W	Н	В	Max	Header	Joist	lbs	lbs	lbs	lbs
											kN	kN	kN	kN
								Min	18-16d	8-10d	2105	5780	1960	4690
9	3 x 14 - 20	HU214-2/ HUC214-2	✓	14	31/8	12 ¹³ / ₁₆	2½				9.36 3160	25.71 5780	8.72 2695	20.86 5780
		1100214 2						Max	24-16d	12-10d	14.06	25.71	11.99	25.71
ונה	3½ x 9½	IUS3.56/9.5	✓7	18	35%	9½	2	_	10-10d		175	2370	175	1685
	3/2 X 3/2	1033.30/9.3		10	378	3/2			10-100	_	0.78	10.54	0.78	7.50
2	3½ x	LF359	✓7	18	3%16	91/4	2		10-10d	2-#8x11/4 WS	105 0.47	2525 11.23	105 0.47	2155 9.60
201	91/4 - 91/2	NAULIO 50/0	✓ 7	40	00/	0127	01/		40.40:1	0.40.1.41/	375	4550	375	3230
		MIU3.56/9	V	16	3%16	813/16	2½		16-16d	2-10dx1½	1.67	20.24	1.67	14.37
ottuctural composite Lumber comfectors	3½ x 11%	IUS3.56/11.88	✓7	18	35%	11%	2	_	12-10d	_	175 0.78	2370	175	1685 7.50
180			/7				_				105	10.54 2880	0.78 105	2270
4	3½ x	LF3511	✓7	18	3%16	1111/4	2	_	12-10d	2-#8x1¼ WS	0.47	12.81	0.47	10.11
5	111/4 - 117/8	MIU3.56/11	√ 7	16	3%16	1111/8	21/2	_	20-16d	2-10dx1½	375	4550	375	3230
ā											1.67 175	20.24	1.67 175	14.37 1685
in.		IUS3.56/14	✓7	18	3%	14	2	_	12-10d	_	0.78	10.54	0.78	7.50
ומר	3½ x 14	LF3514	✓7	18	3%16	13½	2	_	14-10d	2-#8x11/4 WS	105	3235	105	2385
5	072 X 14	L1 0014	•	10	0710	1072			14 100	2 # 0X174 WO	0.47	14.39	0.47	10.61
Liigiiiceicu Moou &		MIU3.56/14	✓ 7	16	3%16	135/16	21/2	_	22-16d	2-10dx1½	375 1.67	4930 21.96	375 1.67	3485 15.52
		11100 56/16	✓7	18	25/	16	2		14 104		175	2370	175	1685
	3½ x 16	IUS3.56/16	V	10	35%	10			14-10d	_	0.78	10.54	0.78	7.50
3	072 X 10	MIU3.56/16	✓7	16	3%16	155/16	2½	_	24-16d	2-10dx1½	375 1.67	4930 21.96	375 1.67	3485 15.52
			1 7		-0.4	.=					375	4930	375	3485
lig.	3½ x 18	MIU3.56/18	✓7	16	3%16	175/16	2½		26-16d	2-10dx1½	1.67	21.96	1.67	15.52
1	3½ x 20	MIU3.56/20	✓7	16	3%16	195/16	2½	_	28-16d	2-10dx1½	375	4930	375	3485
											1.67 375	21.96 4930	1.67 375	15.52 3485
	3½ x 22-30	MIU3.56/20	✓	16	3%16	195/16	2½	_	28-16d	2-10dx1½	1.67	21.96	1.67	15.52
NEW		MIU4.12/9	_	16	41/8	9	21/2		16-16d	2-10dx1½	375	4550	375	3230
											1.67 1580	20.24 5780	1.67 1470	14.37 4225
	4 x 9½	HU4.12/9/			4+7	05/	01/	Min	14-16d	6-10d	7.03	25.71	6.54	18.79
		HUC4.12/9	✓	14	41//8	85%	2½	Max	18-16d	10-10d	2635	5780	2450	4690
_								WICK	10 100	10 100	11.72 375	25.71 4550	10.90 375	20.86 3230
愈		MIU4.12/11	_	16	41//8	1111/8	21/2	_	20-16d	2-10dx1½	1.67	20.24	1.67	14.37
	4 x 11%	UII/ 10/11/						Min	16-16d	6-10d	1580	5780	1470	4225
		HU4.12/11/ HUC4.12/11	✓	14	41//8	105/16	2½				7.03	25.71	6.54	18.79
								Max	22-16d	10-10d	2635 375	5780 4930	2450 375	4690 3485
寧	4 x 14	MIU4.12/14	_	16	41/8	13½	2½	_	22-16d	2-10dx1½	1.67	21.96	1.67	15.52
愈	4 x 16	MIU4.12/16	_	16	41/8	15½	21/2	_	24-16d	2-10dx1½	375	4930	375	3485
											1.67 375	21.96 4550	1.67 375	15.52 3230
	41/ 01/	MIU4.28/9	_	16	49/32	9	21/2	_	16-16d	2-10dx1½	1.67	20.24	1.67	14.37
	41/8 x 91/2	HU4.28/9	1	14	4%2	9	2½	_	18-16d	8-10d	2105	5780	1960	4690
		HUC4.28/9	·	• •	1732		L /2		10 100	0 100	9.36 375	25.71 4550	8.72	20.86 3230
	41/ 447/	MIU4.28/11	_	16	4%2	1111//8	2½	_	20-16d	2-10dx1½	1.67	20.24	375 1.67	14.37
	4½ x 11½	HU4.28/11/	√	14	4%2	11	21/2		22-16d	8-10d	2455	5780	2280	4690
		HUC4.28/11	•	17	-T / 32	""	£12		22 10U	J 100	10.92	25.71	10.14	20.86
	41/8 x 14	MIU4.28/14	_	16	49/32	13½	2½	_	22-16d	2-10dx1½	375 1.67	4930 21.96	375 1.67	3485 15.52
	4% x 16	MILIA 20/16	_	16	4%2	15½	21/2		24-164	2-10dx1½	375	4930	375	3485
	478 X 10	MIU4.28/16		10	4732	1072	∠ 72		24-16d	Z-1UUX 1 /2	1.67	21.96	1.67	15.52
		MIU4.75/9	_	16	43/4	91/16	2½	_	16-16d	2-10dx1½	375	4550	375	3230
	4% x		,								1.67 1440	20.24 4355	1.67	14.37 3090
	91/4 - 91/2	U3510-2	✓	16	43/4	83/4	2	_	14-16d	6-10d	6.41	19.37	5.96	13.75
		HU4.75/9/	1	14	43/4	9	2½		18-16d	8-10d	2105	5780	1960	4690
		HUC4.75/9	A		7/4	3	2/2		10-10u	0-10d	9.36	25.71	8.72	20.86

FACE MOUNT HANGERS – I-JOISTS



				Di	imensio	ns		Faster	iers			Resistance	
		Web			(in)	,		raster	1013		ir-L	_	P-F
Joist	Model	Stiff	Ga							Uplift	Normal	Uplift	Normal
Size	No.	Reqd		w	н	В	Min/	Header	Joist	(K _D = 1.15)	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
							Max			lbs kN	lbs kN	lbs kN	lbs kN
										375	4550	375	3230
	MIU4.75/11	_	16	43/4	111/16	2½	-	20-16d	2-10dx1½	1.67	20.24	1.67	14.37
4% x						_				1440	4355	1340	3095
111/4 - 117/8	U3512-2	√	16	4¾	111/4	2	_	16-16d	6-10d	6.41	19.37	5.96	13.77
	HU4.75/11/	✓	14	43/4	11	2½	_	22-16d	8-10d	2455	5780	2280	4690
	HUC4.75/11	V	14	474	11	Z /2		22-10u	0-10u	10.92	25.71	10.14	20.86
	MIU4.75/14	_	16	43/4	13½	2½	_	22-16d	2-10dx1½	375	4930	375	3485
4% x14	11110511.07									1.67	21.96	1.67	15.52
	HU3514-2/ HUC3514-2	✓	14	4¾	131/4	21/2	_	18-16d	8-10d	2105 9.36	5780 25.71	1960 8.72	4690 20.86
										3.30	4930	375	3485
	MIU4.75/16	_	16	4¾	15½	2½	_	24-16d	2-10dx1½	1.67	21.96	1.67	15.52
45/ v.10							Min	00.164	0.104	2105	5780	1960	4690
4% x 16	HU3516-2/	✓	14	43/4	151/4	2½	Min	20-16d	8-10d	9.36	25.71	8.72	20.86
	HUC3516-2	•	14	774	1074	2/2	Max	26-16d	12-10d	3160	5780	2695	5780
										14.06	25.71	11.99	25.71
4% x 18	MIU4.75/18	_	16	43/4	17½	21/2	_	26-16d	2-10dx1½	375 1.67	4930	375	3485
										375	21.96 4930	1.67 375	15.52 3485
	MIU4.75/20	_	16	4¾	19½	2½	_	28-16d	2-10dx1½	1.67	21.96	1.67	15.52
45/00							N 4:	00.404	0.404	2105	5780	1960	4690
4% x 20	HU3520-2/	1	14	43/4	191/4	2½	Min	20-16d	8-10d	9.36	25.71	8.72	20.86
	HUC3520-2	V	14	474	1374	2 /2	Max	26-16d	12-10d	3160	5780	2695	5780
							- Triun	20 .00	.2 .00	14.06	25.71	11.99	25.71
	MIU4.75/20	✓	16	43/4	19½	21/2	_	28-16d	2-10dx1½	375 1.67	4930 21.96	375 1.67	3485 15.52
										2105	5780	1960	4690
4% x 22-30	HU3520-2/						Min	20-16d	8-10d	9.36	25.71	8.72	20.86
	HUC3520-2	✓	14	43/4	191/4	2½	Max	00.104	10 104	3160	5780	2695	5780
							Max	26-16d	12-10d	14.06	25.71	11.99	25.71
	MIU5.12/9	_	16	51/8	813/16	2½	_	16-16d	2-10dx1½	375	4550	375	3230
5 x 91/4 - 91/2										1.67	20.24	1.67	14.37
	HU310-2/ HUC310-2	✓	14	51//8	8%	21/2	_	14-16d	6-10d	1580 7.03	5780 25.71	1470 6.54	4225 18.79
										375	4550	375	3230
5 x	MIU5.12/11	-	16	51/8	111//	2½	_	20-16d	2-10dx1½	1.67	20.24	1.67	14.37
111/4 - 117/8	HU312-2/	/	14	51//8	105/8	2½		16-16d	6-10d	1580	5780	1470	4225
	HUC312-2	•	14	J /8	1078	2 /2		10-100	0-100	7.03	25.71	6.54	18.79
5 x 14	MIU5.12/14	_	16	51//8	135/16	2½	_	22-16d	2-10dx1½	375	4930	375	3485
										1.67 375	21.96 4930	1.67 375	15.52 3485
5 x 16	MIU5.12/16	_	16	51/8	155/16	21/2	_	24-16d	2-10dx1½	1.67	21.96	1.67	15.52
- 10				=	.=					375	4930	375	3485
5 x 18	MIU5.12/18	_	16	51/8	175/16	2½	_	26-16d	2-10dx1½	1.67	21.96	1.67	15.52
5 x 20	MIU5.12/20	_	16	51//8	195/16	2½	_	28-16d	2-10dx1½	375	4930	375	3485
3 X Z 0	W103.12/20		10	3 78	13710	L /2		20 10u	2 100X172	1.67	21.96	1.67	15.52
5 x 22 - 30	MIU5.12/20	✓	16	51//8	195/16	21/2	_	28-16d	2-10dx1½	375	4930	375	3485
										1.67 1840	21.96 5780	1.67 1710	15.52 4225
	HU410-2/						Min	14-16d	6-16d	8.18	25.71	7.61	18.79
7 x 9¼ - 9½	HUC410-2	√	14	71/8	91/8	2½	N.A	40.40.1	0.404	2455	5780	2280	4690
							Max	18-16d	8-16d	10.92	25.71	10.14	20.86
							Min	16-16d	6-16d	1840	5780	1710	4225
7 X	HU412-2/	✓	14	71//8	1111/8	2½		10 100	3 100	8.18	25.71	7.61	18.79
111/4 - 117/8	HUC412-2						Max	22-16d	8-16d	2455	5780	2280	4690
										10.92 2455	25.71 5780	10.14 2280	20.86 4690
-	HU414-2/	,			400		Min	20-16d	8-16d	10.92	25.71	10.14	20.86
	HUC414-2	✓	14	71/8	13%	2½	Max	26-16d	12-16d	3685	7025	3420	5780

See footnotes on page 109.

FACE MOUNT HANGERS – STRUCTURAL COMPOSITE LUMBER



			D	imensio	18		Faster	ers			Resistance	
I-i-A	Model			(in)				 	Uplift	ir-L Normal	Uplift	P-F Normal
Joist Size	Model No.	Ga				N/im/			(K _D = 1.15)	$(K_D = 1.00)$	(K _D = 1.15)	$(K_D = 1.00)$
0120	No.		W	Н	В	Min/ Max	Header	Joist	(K) = 1.13) lbs	(KD = 1.00)	lbs	(KD = 1.00)
						IVIAA			kN	kN	kN	kN
						Min	12-16d	4-10dx1½	980	2785	905	1975
1¾ x 5½	HU1.81/5	14	1 13/16	5%	2½				4.36	12.39	4.03	8.79
						Max	16-16d	6-10dx1½	1470	3715	1360	2635
						· · · · ·		0.1047.172	6.54	16.53	6.05	11.72
						Min	12-16d	4-10dx1½	980	3775	905	2670
1¾ x 7¼	HU7	14	1 13/16	611/16	2½		12 100	4 100X172	4.36	16.82	4.03	11.89
174 X 7 74	1107	'7	1 /16	0 716	L /2	Max	16-16d	8-10dx1½	1960	5445	1810	4225
						IVIAX	10-100	0-100X172	8.72	24.25	8.06	18.79
	111104 04/40	16	113/	9	3		20.464	10.164	4505	6405	4010	5200
	HUS1.81/10	16	1 13/16	9	3		30-16d	10-16d	20.05	28.48	17.84	23.13
42/ 01/						D.41.	40.40.1	0.40.141/	1470	4830	1360	3875
1¾ x 9½			4407	05/	047	Min	18-16d	6-10dx1½	6.54	21.48	6.05	17.24
	HU9	14	1 13/16	95/16	21/2				2450	5685	2265	4660
						Max	24-16d	10-10dx1½	10.90	25.29	10.08	20.73
									4505	6405	4010	5200
	HUS1.81/10	16	1 13/16	9	3	—	30-16d	10-16d	20.05	28.48	17.84	23.13
1¾ x									1470	4830	1360	3875
174 X 111/4 - 117/8						Min	22-16d	6-10dx1½	6.54	21.48	6.05	17.24
1171 1170	HU11	14	1 13/16	111/16	21/2				2450	5685	2265	4660
						Max	30-16d	10-10dx1½	10.90	25.29	10.08	20.73
	HUS1.81/10	16	1 13/16	9	3	_	30-16d	10-16d	4505	6405	4010	5200
									20.05	28.48	17.84	23.13
1¾ x 14						Min	28-16d	8-10dx1½	1960	5255	1810	4265
	HU14	14	1 13/16	1311/16	2½				8.72	23.38	8.05	18.97
						Max	36-16d	14-10dx1½	3430	5780	2695	5450
						Wax	00 100	11 100/172	15.26	25.71	11.99	24.24
						Min	10-16d	4-10d	1055	4270	980	3135
	HU48/	14	3%16	613/16	2½	IVIIII	10 100	7 100	4.69	18.99	4.36	13.95
3½ x 7¼	HUC48	14	0/16	0 716	2 /2	Max	14-16d	6-10d	1580	5780	1470	4225
J/2 X 1 /4						IVIAA	14-10u	0-100	7.03	25.71	6.54	18.79
	HGUS48	12	35%	71/16	4		36-16d	12-16d	6070	12980	4310	9215
	пиизи	12	378	1 716	4		30-10u	12-100	27.00	57.74	19.17	40.99
	11440	40	00/	02/	_		44.40.1	0.40-1	1440	4355	1340	3090
	U410	16	3%16	8%	2	_	14-16d	6-10d	6.41	19.37	5.96	13.75
			201	0457	_				3795	5690	3450	4570
	HUS410	14	3%16	815/16	2	_	8-16d	8-16d	16.88	25.31	15.35	20.33
									1580	5780	1470	4225
3½ x	HU410/					Min	14-16d	6-10d	7.03	25.71	6.54	18.79
91/4 - 91/2	HUC410	14	3%16	8%	21/2				2635	5780	2450	4690
						Max	18-16d	10-10d	11.72	25.71	10.90	20.86
									4745	9855	4310	7000
	HHUS410	14	3%	9	3	_	30-16d	10-16d	21.11	43.84	19.17	31.14
									6840	14645	4855	10400
	HGUS410	12	3%	91/16	4	_	46-16d	16-16d	30.43	65.14	21.60	46.26
	U410	16	3%16	8%	2	_	14-16d	6-10d	1440	4355	1340	3090
									6.41	19.37	5.96	13.75
	HUS412	14	3%16	10½	2	_	10-16d	10-16d	4745	7015	3650	4980
									21.11	31.20	16.24	22.15
						Min	16-16d	6-10d	1580	5780	1470	4225
3½ x	HU412/	14	39/16	105/16	2½				7.03	25.71	6.54	18.79
111/4 - 117/8	HUC412		37.10	.0710		Max	22-16d	10-10d	2635	5780	2450	4690
						IVIUA	<i>LL</i> 100	10 100	11.72	25.71	10.90	20.86
	HGUS410	12	35%	91/16	4	_	46-16d	16-16d	6840	14645	4855	10400
	11003410	12	3 /8	3 /16	4		40-10u	10-100	30.43	65.14	21.60	46.26
	HGUS412	12	35/8	107/16	4		56.164	20-16d	7640	14995	5425	10645
			33/0	111/16	4		56-16d	ZU-10(1	33.98	66.70	24.13	47.35

See footnotes on page 116.

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FACE MOUNT HANGERS – STRUCTURAL COMPOSITE LUMBER



			D	imension (in)	ns		Faster	iers	D F	Factored I	Resistance	P-F
Joist	Model			(111)					Uplift	Ir-L Normal	Uplift	P-F Normal
Size	No.	Ga				Min/			(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	$(K_D = 1.00)$
			W	Н	В	Max	Header	Joist	lbs	lbs	lbs	lbs
									kN	kN	kN	kN
					_				1440	4355	1340	3095
	U414	16	3%16	10	2	_	16-16d	6-10d	6.41	19.37	5.96	13.77
									2105	5780	1960	4690
04/ 44	HU416/		00/	105/	04/	Min	20-16d	8-10d	9.36	25.71	8.72	20.86
3½ x 14	HUC416	14	3%16	13%	2½	D.4	00.40.4	40.40.1	3160	5780	2695	5780
						Max	26-16d	12-10d	14.06	25.71	11.99	25.71
	HGUS414	12	35/8	127/16	4		66-16d	22-16d	10130	16400	7195	11645
	11003414	12	378	12716	4		00-100	22-10u	45.06	72.95	32.00	51.80
3½ x 16	HGUS414	12	35/8	127/16	4	_	66-16d	22-16d	10130	16400	7195	11645
372 X 10	11003414	12	J78	12/16	4		00-100	22-10u	45.06	72.95	32.00	51.80
3½ x 18	HGUS414	12	35/8	127/16	4	_	66-16d	22-16d	10130	16400	7195	11645
072 X 10	11000414	12	078	12/10			00 100	22 100	45.06	72.95	32.00	51.80
51/4 x 71/4	HGUS5.50/8	12	5½	73/16	4	_	36-16d	12-16d	6070	12980	4310	9215
6 717.77	11466616676		0,2	.,,,,	· ·		00.00	.2 .00	27.04	57.82	19.17	40.99
						Min	14-16d	6-16d	1840	5780	1710	4225
	HU610/	14	5½	75/8	2½				8.18	25.71	7.61	18.79
	HUC610					Max	18-16d	8-16d	2455	5780	2280	4690
51/4 x 91/4 - 91/2									10.92	25.71	10.14	20.86
	HHUS5.50/10	14	5½	9	3	_	30-16d	10-16d	4745	10545	4310	7485
									21.11	46.91	19.17	33.29
	HGUS5.50/10	12	5½	815/16	4	_	46-16d	16-16d	6840	14645 65.23	4855	10400
									30.47 1840	5780	21.60 1710	46.26 4225
	11110407					Min	16-16d	6-16d	8.18	25.71	7.61	18.79
E1/ v	HU612/ HUC612	14	5½	9%	21/2				2455	5780	2280	4690
5¼ x 11¼ - 11⅓	1100012					Max	22-16d	8-16d	10.92	25.71	10.14	20.86
									7640	14995	5425	10645
	HGUS5.50/12	12	5½	10½	4	_	56-16d	20-16d	34.03	66.79	24.13	47.35
									2455	5780	2280	4690
	HU614/					Min	18-16d	8-16d	10.92	25.71	10.14	20.86
	HUC614	14	5½	11%	21/2				3685	7025	3420	6185
5¼ x 14						Max	24-16d	12-16d	16.39	31.25	15.21	27.51
	1101105 50/14	40	F4./	101/			00.40.1	00.40.1	10130	16400	7195	11645
	HGUS5.50/14	12	5½	12½	4	_	66-16d	22-16d	45.12	73.05	32.00	51.80
						N Alian	00.404	0.404	2455	5780	2280	4690
	HU616/	1.1	E1/	1011/.	21/	Min	20-16d	8-16d	10.92	25.71	10.14	20.86
5¼ x 16	HUC616	14	5½	1211/16	2½	Max	26-16d	12-16d	3685	7025	3420	6185
J74 X 10						IVIAX	20-10u	12-100	16.39	31.25	15.21	27.51
	HGUS5.50/14	12	5½	12½	4	_	66-16d	22-16d	10130	16400	7195	11645
	110003.30/14	12	072	12/2	7		00 100	22 Tou	45.12	73.05	32.00	51.80
						Min	20-16d	8-16d	2455	5780	2280	4690
	HU616/	14	5½	1211/16	21/2		20 100	0.100	10.92	25.71	10.14	20.86
51/4x 18	HUC616		0,2	12 7.10		Max	26-16d	12-16d	3685	7025	3420	6185
									16.39	31.25	15.21	27.51
	HGUS5.50/14	12	5½	121/2	4	_	66-16d	22-16d	10130	16400	7195	11645
									45.12	73.05	32.00	51.80
7 x 71/4	HGUS7.25/8	12	71/4	77/32	4	_	36-16d	12-16d	6070	12980	4310	9215
									27.04	57.82	19.17	41.00
	1111440 07					Min	14-16d	6-16d	1840 8.18	5780 25.71	1710 7.61	4225 18.79
	HU410-2/ HUC410-2	14	71/8	91/8	21/2			+	2455	5780	2280	4690
	11001102					Max	18-16d	8-16d	10.92	25.71	10.14	20.86
7 x 91/4 - 91/2								+	4745	10770	4310	7650
	HHUS7.25/10	14	71/4	9	35/16	_	30-16d	10-16d	21.11	47.91	19.17	34.03
		<u> </u>	<u> </u>	_					6840	15760	4855	11190
	HGUS7.25/10	12	71/4	85/8	4	_	46-16d	16-16d	30.47	70.20	21.60	49.78
									1840	5780	1710	4225
	HU412-2/		71/	4417	017	Min	16-16d	6-16d	8.18	25.71	7.61	18.79
7 x	HUC412-2	14	71/8	1111//	21/2	N.A.	00.40.1	0.401	2455	5780	2280	4690
111/4 - 117/8						Max	22-16d	8-16d	10.92	25.71	10.14	20.86
	HOUGZ OF (40	10	71/	105/	4		EC 10-1	20.164	7640	16110	5425	11435
	HGUS7.25/12	12	71/4	10%	4		56-16d	20-16d	34.03	71.76	24.13	50.87

See footnotes on page 116.

Engineered Wood & Structural Composite Lumber Connectors

FACE MOUNT HANGERS – STRUCTURAL COMPOSITE LUMBER



			D	imensio	18		Fasten	0.00		Factored I	Resistance	
				(in)			rasien	ers	D.F	ir-L	S-I	P-F
Joist	Model	0-							Uplift	Normal	Uplift	Normal
Size	No.	Ga	w		В	Min/	Haadas	laint	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
			W	Н	В	Max	Header	Joist	lbs	lbs	lbs	lbs
									kN	kN	kN	kN
						Min	20-16d	8-16d	2455	5780	2280	4690
	HU414-2/	14	71/8	13%	21/2	IVIIII	20-10u	0-10u	10.92	25.71	10.14	20.86
7 x 14	HUC414-2	14	1 /8	1078	2 /2	Max	26-16d	12-16d	3685	7025	3420	6185
/ / 14						IVIAA	20-10u	12-10u	16.39	31.25	15.21	27.51
	HGUS7.25/14	12	71/4	125/8	4		66-16d	22-16d	10130	18200	7195	12920
	110037.23/14	12	1 74	1278	4		00-10u	22-10u	45.06	81.07	32.00	57.47
						Min	20-16d	8-16d	2455	5780	2280	4690
	HU414-2/	14	71/8	13%	2½	IVIIII	20-10u	0-10u	10.92	25.71	10.14	20.86
7 x 16	HUC414-2	14	1 /8	1078	2 /2	Max	26-16d	12-16d	3685	7025	3420	6185
7 X 10						IVIAX	20-10u	12-100	16.39	31.25	15.21	27.51
	HGUS7.25/14	12	71/4	127/16	4		66-16d	22-16d	10130	18200	7195	12920
	110037.23/14	12	1 74	12716	4		00-100	22-10u	45.06	81.07	32.00	57.47
						Min	20-16d	8-16d	2455	5780	2280	4690
	HU414-2/	14	71/8	13%	2½	IVIIII	20-10u	0-100	10.92	25.71	10.14	20.86
7 x 18	HUC414-2	14	1 78	1378	Z 72	Max	26-16d	12-16d	3685	7025	3420	6185
1 X 10						ividX	20-10u	12-100	16.39	31.25	15.21	27.51
	HGUS7.25/14	12	71/4	127/16	4		66-16d	22-16d	10130	18200	7195	12920
	110037.23/14	12	1 74	12716	4		00-100	22-10U	45.06	81.07	32.00	57.47

- 1. 10d common nails may be used instead of the specified 16d nails at 0.83 of the tabulated value.
- Uplift loads have been increased 15% for earthquake or wind loading with no further increase allowed. Reduce by 15% for standard term loading $(K_D = 1.00)$ such as in cantilever construction.
- 3. MIN nailing quantity and load values—fill all round holes; MAX nailing quantity and load values—fill all round and triangle holes.
- 4. Structural composite lumber is LVL, LSL and Parallam® PSL.
- 5. D-Fir.L factored resistances can be used for most LVL. Verify with manufacturer prior to selecting hanger.
- 6. **NAILS:** 16d = 0.162" dia. x 3½" long, 10d = 0.148" dia. x 3" long. See pages 22-23 for other nail sizes and information.

Parallam is registered trademark of iLevel® by Weyerhaeuser.

HUCQ Heavy-Duty Joist Hangers

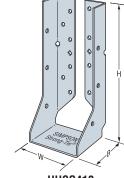
The HUCQ series are heavy-duty joist hangers that incorporate Simpson Strong-Tie® Strong-Drive® SDS Heavy-Duty Connector screws. Designed and tested for installation at the end of a beam or on a post, they provide a strong connection with fewer fasteners than nailed hangers.

MATERIAL: 14 gauge FINISH: Galvanized

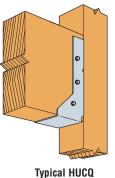
INSTALLATION: • Use all specified fasteners. See General Notes.

- Strong-Drive SDS Heavy-Duty Connector screws supplied.
- For use on solid sawn or engineered wood and structural composite lumber products.

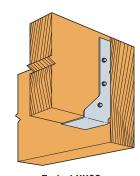
OPTIONS: HUCQ cannot be modified.







Installation on a Post



Typical HUCQ Installation on a Beam

	D	imension	ıs	Faste	nore		Factored F	Resistance	
		(in)		1 dott	511013	D.F	ir-L	S-I	P-F
Model						Uplift	Normal	Uplift	Normal
No.	w	н	В	Face	Joist	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
	VV	п	D	гасе	20121	lbs	lbs	lbs	lbs
						kN	kN	kN	kN
HUCQ1.81/9-SDS1.75	1 13/16	9	3	8-1/4"x13/4" SDS	4-1/4"x13/4" SDS	1565	4350	1450	3300
110001.01/8-3031.73	I '716	9	3	0-74 X 174 3D3	4-74 X 174 3D3	6.96	19.35	6.45	14.68
HUCQ1.81/11-SDS1.75	113/16	11	3	10-1/4"x13/4" SDS	4-1/4"x13/4" SDS	1565	5440	1450	3560
110001.01/11-3031.73	I '716	- 11	3	10-74 X 174 303	4-74 X 174 3D3	6.96	24.20	6.45	15.84
HUCQ410-SDS2.5	3%16	9	3	12-¼"x2½" SDS	6-¼"x2½" SDS	3210	7270	2900	6825
11000410-3032.3	3716	ס	3	12-74 X272 3D3	0-74 XZ72 3D3	14.28	32.34	12.90	30.36
HUCQ412-SDS2.5	3%16	11	3	14-¼"x2½" SDS	6-1/4"x21/2" SDS	3210	9090	2900	7645
11000412-3032.3	3716		3	14-74 8272 303	0-74 XZ72 3D3	14.28	40.43	12.90	34.01
HUCQ610-SDS2.5	5½	9	3	12-¼"x2½" SDS	6-¼"x2½" SDS	3210	7270	2900	6825
HUUU010-5D52.5	J /2	ס	3	12-74 XZ 72 3D3	0-74 XZ 72 3D3	14.28	32.34	12.90	30.36
HUCQ612-SDS2.5	5½	11	3	14-1/4"x21/2" SDS	6-1/4"x21/2" SDS	3210	9090	2900	7645
11000012-0032.3	3 /2	- 11	3	14-74 AZ /2 3D3	U-74 AZ 72 3D3	14.28	40.43	12.90	34.01

^{1.} Factored uplift resistances have been increased 15% for earthquake or wind loading with no further increase allowed. Reduce by 15% for standard term loading such as cantilever construction

LGU/MGU/HGU/HHGU High-Capacity Girder Hangers



The GU hangers are a high-capacity girder hangers designed for situations where the header and joist are flush at top. This part can be used for retrofit on the framing members after they are temporarily placed in position. It uses Simpson Strong-Tie® Strong-Drive® SDS Heavy-Duty Connector screws to make installation fast and easy, with no pre-drilling required.

MATERIAL: See table

FINISH: LGU, MGU—Galvanized;

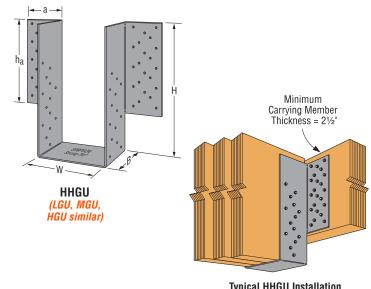
HGU, HHGU—Simpson Strong-Tie® gray paint

INSTALLATION: • Use all specified fasteners. See General Notes.

- Install with 1/4"x21/2" Strong-Drive SDS Heavy-Duty Connector screws, which are provided with the hangers. (Note: lag screws will not achieve the same loads.)
- · All multiple members must be fastened together to act as a single unit.
- Multiple member headers may require additional fasteners at the hanger locations. The quantity and location of the additional fasteners must be determined by the Designer.

OPTIONS: • Hot-dip galvanized available. Order as "X" version, specify HDG.

- Other seat widths available. Order as "X" version, specify width.
- See Hanger Options, pages 230-231, for one flange concealed option (all models except MGU3.63).
- LGU, MGU and HGU hangers may be skewed up to 45°.



Typical HHGU	Installation
--------------	--------------

	imensio	ns		Fact	eners		Factored F	Resistance					
					(in)			Tast	CHCIS	D.F	ir-L	S-I	P-F
	Model	Ga								Uplift	Normal	Uplift	Normal
	No.	ua	w	В	Min.	h		Header	Joist	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
			VV	В	Height (H)	ha	а	пеацеі	20121	lbs	lbs	lbs	lbs
										kN	kN	kN	kN
	LGU3.63-SDS2.5	10	35%	41/2	8	7½	31/4	16-¼"x2½" SDS	12-¼"x2½" SDS	7730	10170	5565	7320
	LG03.03-3D32.3	10	378	472	0	1 72	374	10-74 X272 3D3	12-74 X272 3D3	34.38	45.24	24.75	32.56
	MGU3.63-SDS2.5	10	35%	41/2	91/4	83/4	4	24-¼"x2½" SDS	16-¼"x2½" SDS	10100	13140	7270	9460
	WG03.03-3D32.3	10	378	472	374	074	4	24-74 X272 3D3	10-74 X272 3D3	44.93	58.45	32.34	42.08
<u></u>	HGU3.63-SDS2.5	7	35%	51/4	11	10%	43%	36-¼"x2½" SDS	24-¼"x 2½" SDS	14300	20320	10295	14630
۲	11003.03-3032.3	,	378	J /4	- 11	1078	478	30-74 XZ72 3D3	24-74 X 272 3D3	63.61	90.39	45.79	65.08
	MGU5.50-SDS2.5	10	5½	4½	91/4	83/4	4	24-¼"x2½" SDS	16-¼"x2½" SDS	10100	13140	7270	9460
	WG03.30-3D32.3	10	J /2	4 /2	3/4	074	4	Z4-74 XZ72 3D3	10-74 X272 3D3	44.93	58.45	32.34	42.08
	HGU5.50-SDS2.5	7	5½	51/4	11	10%	43/4	36-1/4" x 21/2" SDS	24-1⁄4"x 21⁄2" SDS	14300	20320	10295	14630
	11000.00 0002.0	,	072	074	11	1078	7/4	30 74 X 272 0D0	24 /4 X 2/2 ODO	63.61	90.39	45.79	65.08
	HHGU5.50-SDS2.5	3	5½	51/4	13	12%	43/4	44-¼"x2½" SDS	28-¼"x2½" SDS	21740	26665	15655	19195
	1111003.30-3032.3	3	372	J /4	10	12.78	474	44-74 8272 303	20-74 X272 3D3	96.70	118.61	69.64	85.38
	HGU7.25-SDS2.5	7	71/4	51/4	11	10%	43/4	36-¼"x2½" SDS	24-¼"x2½" SDS	14300	20320	10295	14630
	11007.23 0002.3	,	1 /4	374	11	1078	7/4	00 74 XZ72 0D0	24 74 AZ72 ODO	63.61	90.39	45.79	65.08
	HHGU7.25-SDS2.5	3	71/4	51/4	13	12%	43/4	44-¼"x2½" SDS	28-1/4"x21/2" SDS	21740	26665	15655	19195
	1111007.23 0002.3	0	1 /4	J / 4	10	12/0	7/4	44 74 AZ72 ODO	20 74 8272 000	96.70	118.61	69.64	85.38
	HGU9.00-SDS2.5	7	9	51/4	11	10%	43/4	36-1/4"x21/2" SDS	24-1/4"x21/2" SDS	14300	20320	10295	14630
	11000.00 0002.0		J	074	''	1078	774	00 74 NZ 72 0D0	24 /4 NZ/2 0D0	63.61	90.39	45.80	65.08
	HHGU9.00-SDS2.5	3	9	51/4	13	12%	43/4	44-1/4"x21/2" SDS	28-¼"x2½" SDS	21740	26665	15655	19195
	1111000.00 0002.0	0		074	10	12/0	774	11 /4 NZ/2 ODO	20 74 1272 000	96.71	118.62	69.64	85.39

- 1. Factored uplift resistances have been increased for earthquake and wind loading, with no further increase allowed.
- 2. Specify H dimension. The Designer should check the shear capacity of the carried member to make sure it matches the hanger's capacity. Maximum H = 30".
- 3. Header depth must exceed the ha dimension shown and is based on the size necessary to fit screw pattern. Use the next size up that meets the minimum depth requirement.



TOP FLANGE HANGERS ITS/LT/MIT/HIT Engineered Wood Products Hangers





This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

A dedicated range of Top Flange I-joist hangers meeting the unique needs of I-joists while offering superior performance and ease of installation.

The innovative ITS sets a new standard for engineered wood top flange hangers. The ITS installs faster and uses fewer nails than any other EWP top flange hanger. The Strong-Grip™ seat and Funnel Flange™ features allow standard joist installation without requiring joist nails resulting in the lowest installed cost. The Strong-Grip seat firmly secures I-joists with flange thicknesses from 11/4" to 11/2".

The LT series of top flange hangers is designed for use with wood 1-joists. Installation is fast and easy. The hanger's top flange simplifies placement and the side flanges laterally support the I-joist top flange eliminating the need for web stiffeners. Securing the carried I- joist is simple with only one or two screws required into the bottom flange through the seat of the hanger.

MIT/HIT - Patented Positive Angle Nailing (PAN)

PAN is specifically designed for I-joists when used with the MIT or HIT. With PAN, the nail hole material is not removed, but is formed to channel and confine the path of the nail at approximately 45°. PAN minimizes splitting of the flanges while permitting time-saving nailing from a better angle. See Top Flange tables on pages 126-136.

Refer to Joist Manufacturer's literature or appropriate Simpson Strong-Tie Connector Selection Guide for actual joist sizes.

MATERIAL: ITS, LT-18 gauge; MIT, HIT-16 gauge

FINISH: Galvanized INSTALLATION:

- Use all specified fasteners. Verify that the header can take the required fasteners specified in the table.
- See product specific installation drawings pages 118-119.
- ITS—no joist nailing required for standard I-joist installation without web stiffeners. When supporting I-joists with web stiffeners or rectangular SCL member 2-10dx1½" must be installed into optional triangle joist nail holes for standard installation values.
- ITS—optional triangle nail holes may be used for additional capacity. See load tables.
- MIT and LT—optional triangle nail holes may be used for increased uplift capacity. See Optional Nailing For Increased Uplift table.
- · HIT—closed PAN nail holes may be used for increased uplift capacity. See Optional Nailing For Increased Uplift table.
- For sloped joists up to 1/4:12 there is no reduction, between 1/4:12 and up to ½:12, tests show a 10% reduction in ultimate hanger strength. Local crushing of the bottom flange or excessive deflection may be limiting; check with joist manufacturer for specific limitations on bearing of this type.

FACTORED RESISTANCES:

• The ITS, LT, MIT and HIT hangers have locations for optional nails if additional uplift is needed. Optional uplift nailing requires the addition of properly-secured web stiffeners. See the load tables for minimum required fasteners and uplift capacities.

OPTIONS:

· Because these hangers are fully die-formed, they cannot be modified. However these models will normally accommodate a skew of up to 5°.

POSITIVE ANGLE NAILING

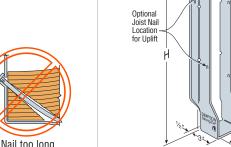


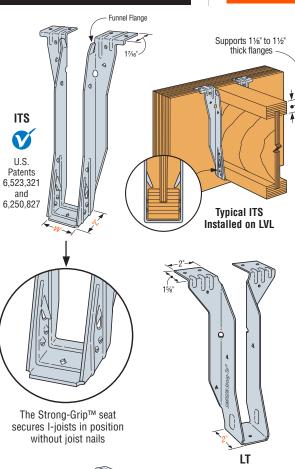


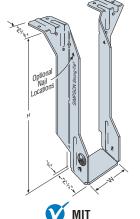


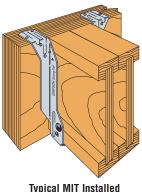




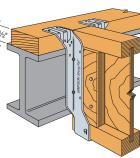








on a Double LVL



HIT Installation on a 3x Nailer mounted on a Steel Beam

TOP FLANGE HANGERS ITS/LT/MIT/HIT Engineered Wood Products Hangers



IT SERIES WITH VARIOUS HEADER APPLICATIONS

		Fastener	S		Fa	ctored	Resist	tance		
				Uplift		No	rmal (I	(_D = 1.	00)	
Model	Top	Face	Joist	$(K_D = 1.15)$	D.Fir-L	S-P-F	LVL ⁴	PSL	LSL	I-Joist⁵
	Top	Гасс	30131	lbs	lbs	lbs	lbs	lbs	lbs	lbs
				kN	kN	kN	kN	kN	kN	kN
	1-10dv11/	2-10dx1½		175	2115	1670	2050	1830	2385	1375
170.0	4-10ux 1 72	Z-10ux 1 72		0.78	9.41	7.43	9.12	8.14	10.61	6.12
ITS Series (Standard	4-10d	2-10d		175	2235	1690	2280	2005	2615	_
	4-10u	2-10u		0.78	9.94	7.52	10.14	8.92	11.63	_
	4-16d	2-16d		175	2375	1795	2610	2550	2795	_
	4-10u	2-10u		0.78	10.56	7.98	11.61	11.34	12.43	—
.=0.0	4-10d	4-10d	4-10dx1½	830	2870	1805	2545	2345	2770	_
	4-10u	4-10u	4-100X172	3.69	12.77	8.03	11.32	10.43	12.32	_
installation) ITS Series ⁸ (Optional Installation) LT Series	4-16d	4-16d	4-10dx1½	830	2870	1805	2610	2550	2795	_
motunation	4-10u	4-10u	4-10ux172	3.69	12.77	8.03	11.61	11.34	12.43	_
	4.10dv11/	0.10dv11/	1-#8x11/4 WS	105	1910	1480	2175	1980	2215	1695
	4-10ux 1 /2	Z-10UX 1 72	1-#0X174 W3	0.47	8.50	6.58	9.68	8.81	9.85	7.54
IT Corion	4-10d	2-10d	1-#8x1¼ WS	105	2625	1725	2560	2480	2620	_
LI Selles	4-10u	2-10u	1-#0X174 W3	0.47	11.68	7.67	11.39	11.03	11.65	_
(Optional Installation) LT Series	4-16d	2-16d	1-#8x1¼ WS	105	2760	1850	2560	2480	2620	_
	4-10u	2-10u	1-#0X174 VV3	0.47	12.28	8.23	11.39	11.03	11.65	_
	4.10dv11/	4-10dx1½	2-10dx1½	375	3145	1825	3330	2455	2630	1900
	4-10ux172	4-10ux 1 72	Z-100X172	1.67	13.99	8.12	14.81	10.92	11.70	8.45
MIT Series	4-10d	4-10d	2-10dx1½	375	3295	2420	3550	3025	2630	_
WILL SELIES	4-10u	4-10u	Z-100X172	1.67	14.66	10.77	15.79	13.46	11.70	_
	4-16d	4-16d	2-10dx1½	375	3490	2420	3550	3025	3465	_
	4-100	4-10u	Z-10UX172	1.67	15.52	10.77	15.79	13.46	15.41	_
HIT Series	4-16d	6-16d	2-10dx1½	450	4570	2705	3725	3220	3775	_
THE SELLES	4-10u	0-100	Z-10UX172	2.00	20.33	12.03	16.57	14.32	16.79	_

- 1. When I-joist is used as header, all nails must be 10dx11/2.
- Resistances may not be increased for short-term loading.
- Uplift resistances are based on D.Fir-L, and have been increased 15% for wind or earthquake loading with no further increase allowed. Divide by 1.15 for normal loading criteria like cantilever construction.
- For S-P-F use 0.71 x resistance.

 4. Applies to LVL headers made primarily from Douglas Fir or Southern Pine. For LVL made primarily from Spruce Pine Fir or similar less dense veneers, use the values found in the S-P-F column.
- 5. For I-joist flanges less than 1½" thick multiply table values by a factor of 0.85.
 6. Minimum solid header thickness to achieve LT table loads is 1¾".
- Structural composite lumber is LVL, LSL and Parallam® PSL
- ITS optional installation requires web stiffeners installed per I-joist manufacturers recommendations.
- For 16 and 18 gauge, 31/2" wide I-joist hangers, web stiffeners are required when the factored reaction is greater than 2330 lbs. (10.36 kN).

Parallam is registered trademark of iLevel® by Weyerhaeuser.

NAILS: 16d = 0.162" dia. x 3½" long, 16dx2½ = 0.162" dia. x 2½" long, 10d = 0.148" dia. x 3" long, 10dx1½ = 0.148" dia. x 1½" long. See pages 22-23 for other nail sizes and information.

OPTIONAL NAILING FOR INCREASED UPLIFT

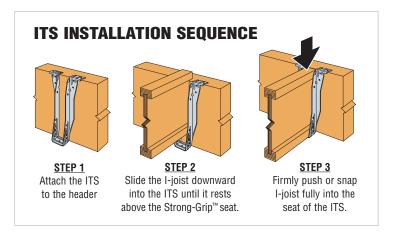
		Fasteners			d Uplift
				Resistance	$(K_D = 1.15)$
Model	Top	Face	Joist	D.Fir-L	S-P-F
	тор	1 466	30131	lbs	lbs
				kN	kN
	4-10dx1½	4-10dx1½	2-10dx11/2	380	380
LT Series	4-10ux172	4-10UX172	Z-10UX172	1.69	1.69
	4-10d	4-10d	2-10dx1½	380	380
	4-10u	4-10u	Z-10UX 1 72	1.69	1.69
	4-16d	4-16d	2-10dx11/2	380	380
	4-10u	4-10u	Z-10UX 172	1.69	1.69
	4-10dx1½	4-10dx1½	4-10dx1½	895	705
	4-10ux172	4-10ux172	4-10ux172	3.98	3.14
MIT	4-10d	4-10d	4-10dx1½	895	705
Series	4-10u	4-10u	4-10ux 1 72	3.98	3.14
	4-16d	4-16d	4-10dx1½	895	705
	4-100	4-100	4-10UX172	3.98	3.14
HIT Series	1-16d	6-16d	4-10dx1½	895	705
	4-16d	0-10u	4-10UX172	3.98	3.14
	4-16d	6-16d	6-10dx1/2	1345	1175
	4-10U	0-160	OFIUIX IV2	5 98	5 23

NAILER TABLE

This table indicates the maximum factored normal resistances for ITS/LT/MIT/HIT hangers used on wood nailers. The header nail type must be substituted for those listed in other tables.

				tored Nor ance (K _D	
Model	Nailer	Header Nailing	D.Fir-L	S-P-F	LSL
		Nailling	lbs	lbs	lbs
			kN	kN	kN
	0	0.40441/	1855	1855	_
ITS Series	2x	6-10dx1½	8.25	8.25	_
(Standard Installation)	2-2x	6-10d	1855	1855	_
,	2-2X	0-10u	8.25	8.25	_
	2-2x	0 104	2560	2240	_
ITS Series	2-2X	8-10d	11.39	9.96	_
(Optional Installation)	4	0.404	2770	_	_
motumation	4x	8-16d	12.32	_	_
	2x	6-10dx1½	1770	1620	1995
	2X	6-100X1½	7.87	7.21	8.87
LT Series	2-2x	6-10d	2310	1995	_
Li Series	Z-ZX	0-10u	10.28	8.87	_
	41/	6 164	2665	_	_
	4x	6-16d	11.85	_	_
	0.4	6-10dx1½	2140	2055	2630
	2x	0-100X172	9.52	9.14	_
	0.00	0 104	2365	2055	_
MIT Series	2-2x	8-10d	10.52	9.14	_
IVIII Series	3x	8-16dx2½	2720	2430	_
	ЭX	0-10UXZ 1/2	12.10	10.81	_
	4.4	8-16d	3090	_	_
	4x	0-10U	13.75	_	_
	2-2x	10-10d	3815	_	_
	Z-ZX	10-100	16.97		_
HIT Series	3x	10-16dx2½	4645	_	_
HIT Selles	ЗX	10-10ux2½	20.66		_
	4x	10-16d	4670		_
	48	10-100	20.77	_	_

- 1. Maximum factored uplift resistance (K_D =1.15) for nailer applications is the lesser of the value shown in "Various Header Applications" table or 310 lbs. (1.38kN)
- 2. For 16 and 18 gauge, 31/2" wide I-joist hangers, web stiffeners are required when the factored reaction is greater than 2330 lbs. (10.36 kN).



^{1.} Factored resistances have been increased 15% for wind or earthquake loading with no further increase allowed. Reduce according to the code for formal loading criteria such as in cartilevel construction.

2. Web stiff eners are required on I-loist for additional nailing.

W/WP/WPU/WM/WMU/HW/HWU 1-Joist & Structural Composite Lumber Hangers

The W, WP, WPU, HWU and HW series purlin hangers offer the greatest design flexibility and versatility. WMs are designed for use on standard 8" grouted masonry block wall construction.

MATERIAL: See tables on pages 126-141; W, WI—12 ga. top flange and stirrup;

WM, WMI, WMU—12 ga. top flange and stirrup; WPU, WP—7 ga. top flange, 12 ga. stirrup; HW, HWI—3 ga. top flange, 11 ga. stirrup;

HWU—3 ga. top flange, 10 ga, stirrup.

FINISH: Simpson Strong-Tie® gray paint; hot-dipped galvanized available: specify HDG.

FACTORED RESISTANCES: For hanger heights exceeding the joist height, the factored resistance is 0.50 of the tabulated resistance.

INSTALLATION: • Use all specified fasteners. WM—two 16d duplex nails must be installed into the top flange and embedded into the grouted wall for mid-wall applications. Verify that the header can take the required fasteners specified in the table.

- Hangers may be welded to steel headers with 1/8" for W, WI, 3/16" for WP, WPI, and 1/4" for HW, HWI, by 1½" fillet welds located at each end of the top flange. Weld-on applications produce maximum factored resistance listed. Uplift resistances do not apply to this application.
- · Hangers can support multi-ply carried members; the individual members must be secured together to work as a single unit before installation into the hanger.
- Hangers can support joists sloped up to 1/4:12 using table values. For joists sloping between 1/4:12 - 3/8:12 use 85% of table value.
- Embed WM into block with a minimum of one course above and one course below the top flange with one 15M vertical rebar minimum 24" long in each cell. Minimum grout strength is 15 MPa.
- Web stiffeners are required for standard joist nailing configuration with this hanger.

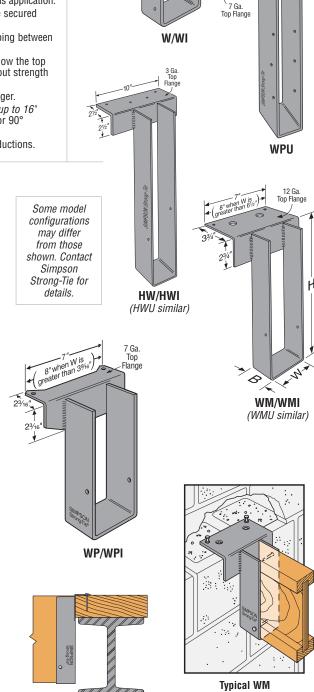
OPTIONS: • Specify alternate nailing pattern when web stiffeners are not being used (up to 16" in depth). Add X ANP after model number for nailing into the flange, available for 90° applications only. Uplift resistances do not apply to this application.

See Hanger Options, page 231 for hanger modifications and associated load reductions.

NAILER TABLE

The table indicates the maximum factored normal resistances for W, WP and HW hangers used on wood nailers. Nailers are wood members attached to the top of a steel I-beam, concrete or masonry wall. This table also applies to sloped-seat hangers.

		Top	Fa	ctored Resistar (K _D = 1.00)	ıce
Model	Nailer	Flange	D.Fir-L	S-P-F	LSL
		Nailing	lbs	lbs	lbs
			kN	kN	kN
	0	0.40-1-41/	2470	2470	_
	2x	2-10dx1½	11.00	11.00	_
	2-2x	2-10d	2730	2730	_
W/WI	Z-ZX	2-10u	12.14	10.61	_
VV/ VVI	3x	2-16dx2½	2895	2855	_
	J 3X	Z-10UXZ1/2	12.88	12.70	_
	4x	2-10d	3025	2855	_
	4X	2-10u	13.46	12.70	_
	2x	2-10dx1½	3665	3630	4900
		Z-10UX172	16.30	16.15	21.82
	2-2x 3x 4x	2-10d	4475	3760	_
WD/WDI	Z-2X	2-10u	19.91	16.75	_
VVF/VVFI	2 v	2-16dx2½	4110	3760	_
	3x 2-10	Z-10UXZ72	18.28	16.75	_
3x 4x	14	2-10d	4475	3760	_
	4x	2-10u	19.91	16.75	_
	2-2v	7-10d	4475	3760	_
	Z-ZX	7-10u	19.91	16.75	_
WPU/	3x	7-16dx2½	4110	3760	_
WNPU	JX.	7-10UXZ72	18.28	16.75	_
	4x	7-10d	4475	3760	_
	47	7-10u	19.91	16.75	_
	2-2x	4-10d	7600	_	_
	Z-ZX	4-10u	33.81	_	_
HW/	3x	4-16dx2½	7600	_	
HWI		4-10UXZ /2	33.81	_	_
	4x	4-16d	7670	_	_
	7.7	7 100	34.16	_	_
	2-2x	8-16dx21/2	7880	_	_
		0 100AZ/2	35.05	_	_
HWU	3x	8-16dx2½	7880	_	
11000		0 100AZ/2	35.05	_	_
	4x	8-16d	7880 35.05	/ TE	HEC



12 Ga.

Top

Flange

RICONTRUSS.CA

Correct Nailer Attachment

Mid-Wall Installation

with Alternate Nailing Pattern (ANP)

W/WP/WPU/WM/WMU/HW/HWU I-Joist & Structural Composite Lumber Hangers



Engineered Wood & Structural Composite Lumber Connectors

W SERIES WITH VARIOUS HEADER APPLICATIONS

	Top Face Joist												
						Uplift1			Normal (K _D = 1.00)			
Model	Width	Depth	Ton	Face	laiat		D.Fir-L	S-P-F	LVL	PSL	LSL	Masonry	
	(in)	(in)	10p	race	JOIST	lbs	lbs	lbs	lbs	lbs	lbs	lbs	
						kN	kN	kN	kN	kN	kN	kN	
	41/ 4- 4	01/ += 00	0.404.41/		0.40441/	_	2455	2375	2675	2850	_	_	
	1½ to 4	3½ to 30	2-10dx1½	_	2-10dx1½	_	10.92	10.56	11.90	12.68	_	_	
W/WI	1½ to 4	3½ to 30	2-10d		2-10dx1½	_	2920	2375	3425	3305	1	_	
VV/ VVI	1 /2 10 4	372 10 30	2-10u		Z-100X172	_	12.99	10.56	15.24	14.70		_	
	1½ to 4	3½ to 30	2-16d	_	2-10dx1½	_	2955	2375	3820	3190		_	
	172 10 4	072 10 00	2 100		2 100X172	_	13.15	10.56	16.99	14.19	_	_	
	1½ to 7½	3½ to 30	2-16d DPLX	_	2-10dx1½			MID-WA	LL INSTAI	LATION		6060	
WM/	172 10 172	0721000	2 100 21 27		2 100/172	_						26.96	
WMI	1½ to 7½	3½ to 30	2-1/4x13/4 Titen	_	2-10dx1½			TOP OF W	ALL INST	ALLATION		5300	
			-			_						23.58	
	1½ to 7½	9 to 28	2-16d DPLX	4-1/4x13/4 Titen	6-10dx1½	860		MID-WA	LL INSTAI	LATION		6060	
WMU						3.83						26.96	
	1½ to 7½	9 to 28	2-1/4x13/4 Titen	4-1/4x13/4 Titen	6-10dx1½	745		TOP OF W	ALL INST	ALLATION		5300	
						3.31	4005	0045	4005	4700		23.58	
	1½ to 7½	3½ to 30	3-10dx1½	_	2-10dx1½	_	4095	3345	4695	4720		_	
						_	18.22	14.88	20.89 3665	21.00	<u> </u>		
WPI	1½ to 7½	3½ to 30	3-10d	_	2-10dx1½	_	4095 18.22	3550 15.79	16.30	4720 21.00	5980 26.60		
						_	4430	3855	5950	5430	5980	_	
	1½ to 7½	3½ to 30	3-16d	_	2-10dx1½		19.71	17.15	26.47	24.15	26.60		
						1665	6390	6390	6825	7085	5980		
	1¾ to 5½	7¼ to 18	3-16d	4-16d	6-10dx1½	7.41	28.43	28.43	30.36	31.52	26.60	_	
WPU						595	6390	6390	6825	7085	5980	_	
	1¾ to 5½	18½ to 28	3-16d	4-16d	6-10dx1½	2.65	28.43	28.43	30.36	31.52	26.60	_	
						_	6900	5285	4695	5810	_	_	
HW/	1½ to 7½	3½ to 32	4-10d	_	2-10dx1½	_	30.69	23.51	20.89	25.85	_	_	
HWI						_	6900	5285	7695	5810	6870	_	
	1½ to 7½	3½ to 32	4-16d	_	2-10dx1½	_	30.69	23.51	34.23	25.85	30.56	_	
	4271 017	0.1.40	4.40.1	4.40.1	0.401.41/	1775	10170	8875	10170	8325	8925	_	
	1¾ to 3½	9 to 18	4-16d	4-16d	6-10dx1½	7.90	45.24	39.48	45.24	37.03	39.70	_	
	13/ to 21/	101/ to 00	4 10 4	4 104	6 10dv11/	1490	10170	8875	10170	8325	8925	_	
	1¾ to 3½	18½ to 28	4-16d	4-16d	6-10dx1½	6.63	45.24	39.48	45.24	37.03	39.70	_	
	1¾ to 3½	28½ to 32	4-16d	4-16d	8-10dx1½	1520	10170	8875	10170	8325	8925	_	
HWU	174 10 372	20/2 10 32	4-100	4-100	0-10UX172	6.76	45.24	39.48	45.24	37.03	39.70	_	
11VV U	4½ to 7½	9 to 18	4-16d	4-16d	6-10dx1½	1775	8250	8250	8250	8250	8250	_	
	T/2 LU 1 /8	3 10 10	7 100	7 100	U IUUXI72	7.90	36.70	36.70	36.70	36.70	36.70	_	
	4½ to 7½	18½ to 28	4-16d	4-16d	6-10dx1½	1490	8250	8250	8250	8250	8250	_	
	1/2 10 1 /0	10/2 10 20	7 100	т той	J TOURT /2	6.63	36.70	36.70	36.70	36.70	36.70	_	
	4½ to 7½	28½ to 32	4-16d	4-16d	8-10dx1½	1520	8250	8250	8250	8250	8250	_	
	1/2 10 1/0	20,21002	. 100	. 100	0 10ux172	6.76	36.70	36.70	36.70	36.70	36.70	_	

^{1.} Factored uplift resistances shown are for D.Fir-L. Multiply tablulated resistances x 0.71 for either SPF joist or header.

Factored resistances shown are for header connection only. The Designer must ensure the joist is capable of generating the factored resistances shown.

^{3.} Structural composite lumber is LVL, LSL and Parallam® PSL.

^{4.} WP/WPI quantity of nail holes in top flange varies.

Applies to LVL headers made primarily from Douglas Fir or Southern Pine. For LVL made primarily from Spruce Pine Fir or similar less dense veneers, use the values found in the S-P-F column.

^{6.} Titen® 1/4 x 13/4 installed on top of wall after grout has cured.

^{7.} **NAILS:** 16d and 16d DPLX = 0.162" dia. x $3\frac{3}{2}$ " long, 10d = 0.148" dia. x 3" long, $10dx1\frac{1}{2} = 0.148$ " dia. x $1\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.

BA/LBV/B/HB I-Joist & Structural Composite Lumber Hangers

SINEERED

Engineered Wood & Structural Composite Lumber Connectors

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

The BA hanger is a cost effective hanger targeted at high capacity I-joists and common Structural Composite Lumber applications. A min/max joist nail option gives dual use of this hanger. Minimum values featuring Positive Angle Nailing are targeted at I-joist without web stiffeners requirement and the maximum nailing generates higher capacities to support structural composite lumber. The unique two level embossment provides added stiffness to the top flange.

The LBV, B and HB hangers offer wide versatility for I-joists and structural composite lumber. An enhanced load capacity widens the range of applications for these hangers. The LBV features Positive Angle Nailing and does not require the use of web stiffeners for standard non modified I-joist installations.

MATERIAL: See tables, pages 126-141.

FINISH: LBV, B, BA and HB—Galvanized; all saddle hangers and all welded sloped and special hangers—Simpson Strong-Tie® gray paint. LBV, B, BA and HB may be ordered hot-dip galvanized; specify HDG.

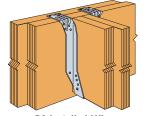
INSTALLATION: • Use specified fasteners. See General Notes and nailer table.

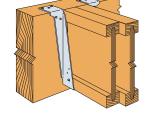
- LBV, B, BA and HB may be used for weld-on applications. Minimum weld size is 1/8"x2" fillet weld to each side of each top flange tab. Distribute the weld equally on both top flanges. Welding cancels the top and face nailing requirements. Consult the code for special considerations when welding galvanized steel. The area should be well-ventilated, see pages 20-21 for additional weld information. Weld on applications produce the maximum factored reisistance listed. Uplift values do not apply to welded applications.
- LBV hangers do not require the use of web stiffeners for non-sloped or non-skewed applications.
- . B and HB hangers require the use of web stiffeners.
- BA Min nailing does not require the use of web stiffeners. BA Max nailing does require web stiffeners.
- Ledgers must be evaluated for each application separately. Check TF dimension, nail length and nail location on ledger.

OPTIONS: • LBV. B and HB

 See Hanger Options, page 231 for hanger modifications and associated reductions in resistance.

LBV HB BA (B Similar) Patent Pending



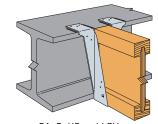


BA Installed LVL to LVL Max Nailing

Typical Double LBV Hanger Installation



LBV features Positive Angle Nailing, no web stiffeners are required



BA, B, HB and LBV are acceptable for weld-on applications (LBV shown). See Installation Information.

B SERIES WITH VARIOUS HEADER APPLICATIONS

		Fasteners				Factore	d Resist	ance		
Model				Uplift1		N	lormal (k	$C_{\rm D} = 1.00$	1)	
Model Series	Тор	Face	Joist	$(K_D = 1.15)$	D.Fir-L	S-P-F	LVL	PSL	LSL	I-Joist
001100	ioh	гасе	30121	lbs	lbs	lbs	lbs	lbs	lbs	lbs
				kN	kN	kN	kN	kN	kN	kN
	6-10dx1½	4-10dx1½	2-10dx1½	435	3165	2340	3760	3885	3295	2200
	0-10ux172	4-10ux172	Z-100X172	1.94	14.08	10.41	16.73	17.28	14.66	9.79
LBV	6-10d	4-10d	2-10dx1½	435	3890	2805	3760	3885	4330	_
LDV	0-100	4-100	Z-100X172	1.94	17.33	12.48	16.73	17.28	19.26	_
	6-16d	4-16d	2-10dx1½	435	3905	3125	3905	4410	4630	_
	0-100	4-10u	Z-10UX172	1.94	17.37	13.90	17.37	19.62	20.60	_
	6-10dv11/	10-10dx1½	2-10dv11/	_	_	_	_	_	_	2420
	0-10ux 1 72	10-10ux172	Z-10UX172	_	_		_	_		10.77
BA	6-10d	10-10d	2-10dx1½	435	4470	3975	4695	5385	5665	_
(Min)		10-100	Z-10UX172	1.94	19.88	17.68	20.89	23.95	25.20	_
	6-16d	10-16d	2-10dx1½	435	4990	4370	5835	5385	5820	_
	0-10u	10-10u	Z-100X172	1.94	22.20	19.44	25.96	23.95	25.89	_
	6-10d	10-10d	8-10dx1½	1960	5265	4035	5825	5945	5980	_
BA	0-10u	10-100	0-10UX 1 72	8.72	23.42	17.95	25.91	26.45	26.60	_
(Max)	6-16d	10-16d	8-10dx1½	1960	5940	4370	6490	7075	6185	_
	0-100	10-100	0-10ux 1 72	8.72	26.42	19.44	28.87	31.47	27.51	_
	6-10d	8-10d	6-10dx1½	1650	5265	3590	5825	5230	5965	_
В	0-100	0-100	0-10ux 1 72	7.34	23.42	15.97	25.91	23.27	26.53	_
В	6-16d	8-16d	6-16dx2½	1650	5940	3910	6490	5230	6185	_
	6-16d	0-10u	U-10UXZ /2	7.34	26.42	17.39	28.87	23.27	27.51	_
HB ⁸	6-16d	16-16d 🖥	-10-164 -	3555	9335	5945	9525	9240	10475	_
пр	0-10u	10-100	10-16d	15.81	41.53	26.45	42.37	41.10	46.60	

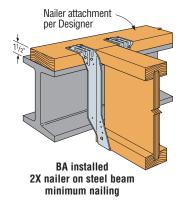
- 1. Factored uplift resistances shown are for D.Fir-L. Multiply tabulated loads x 0.71 for either SPF joist or header.
- 2. Factored resistances shown are for header connection only. The Designer must ensure the joist is capable of generating the factored resistances shown.
- 3. Applies to LVL headers made primarily from Douglas Fir or Southern Pine. For LVL made primarily from Spruce-Pine-Fir or similar less dense veneers, use the values found in the SPF column.
- 4. I-joist values shown refer to I-joists made with SPF or LVI flanges
- 5. I-joists with flanges less than 15/16" thick used in combination with hangers thinner than 14 gauge may deflect an additional 1/32 inch beyond the standard 1/8" limit.
- 6. For flanges with thicknesses from 15/16" to 13/8", use 0.85 of the I-joist header value. For flanges with thicknesses from 11/8" to 11/4", use 0.75 of the I-joist header value.
- 7. For LBV optional uplift, fill all triangle holes with 10dx11/2" nails. Uplift resistances are 1465 lbs (6.52 kN) D.Fir-L and 1040 lbs (4.63 kN) S-P-F.
- 8. Values shown are for a minimum joist width of 21/2".
- 9. **NAILS:** 16d = 0.162" dia. x $3\frac{1}{2}$ " long, $16dx2\frac{1}{2} =$ 0.162" dia. x 21/2" long, 10d = 0.148" dia. x 3" long, 0.148" dia.x 11/2" long. See pages 22-23 for nail sizes and information.

BA/LBV/B/HB I-Joist & Structural Composite Lumber Hangers

NAILER TABLE

This shows the maximum factored resistances for BA, LBV, B, and HB hangers used on wood nailers. Nailers are wood members attached to the top of a steel I-beam, concrete or masonry wall. This table also applies to sloped-seat hangers.

Model		Header		nal Resistance 1.00)
No.	Nailer	Fasteners	D.Fir-L	S-P-F
NO.		Tastellers	lbs	lbs
			kN	kN
	2x	10-10dx1½	2835	2340
	2X	10-100X172	12.61	10.41
	0.00	10 104	2835	2340
L DV	2-2x	10-10d	12.61	10.41
LBV	0	40.40401/	3135	_
	3x	10-16dx2½	13.95	_
	4	10 101	3135	_
	4x	10-16d	17.44	_
	0	40.40441/	3220	2870
	ZX	10-10dx1½	14.32	12.77
	0.0	14 10 1	3915	3660
DA	2x 2-2x 3x	14-10d	17.41	16.28
BA		14-16dx2½	4055	_
) 3X	14-10UXZ72	18.04	_
	4x	14-16d	4055	_
	4X	14-10u	18.04	_
	2x	10-10dx1½	2835	2340
	2.X	10-10ux 1 72	12.63	10.42
	2-2x	14-10d	3915	3660
В	Z-ZX	14-100	17.41	16.28
ט	3x	14-16dx2½	4055	_
	X	14-10UXZ 72	18.04	_
	4x	14-16d	4055	_
	47	14-10u	18.04	_
НВ	4x	22-16d	9015	_
טוו	47	22-10u	40.15	_



- 1. Maximum factored uplift resistance ($K_D = 1.15$) is the lesser of the value shown in the table on page 122 or 385 lbs. (1.71kN).
- 2. **NAILS:** $16d = 0.162^{\circ}$ dia. x $3\frac{1}{2}$ long, $16dx2\frac{1}{2} =$ 0.162" dia. x 21/2" long, 10d = 0.148" dia. x 3" long, $10dx1\frac{1}{2} = 0.148$ " dia. x $1\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.

GLTV/HGLTV Heavy Duty Hangers

GLTV and HGLTV hangers are designed for use with structural composite lumber headers, and may take heavy loads. The top flange nails are sized and specifically located to prevent degradation of the header due to splitting of laminations.

For heavy loads with a face-mount application, see the HGUS series.

MATERIAL: Top flange—3 gauge; Stirrups—7 gauge

FINISH: Simpson Strong-Tie® gray paint

FACTORED RESISTANCES: • For hanger heights exceeding the joist height, the factored resistance is 0.50 of the tabulated resistance.

INSTALLATION: • Use all specified fasteners. Verify that the header can take the required fasteners specified in the table.

- This series may be used for weld-on applications. Minimum required weld is a 3%6" x 21%2" fillet weld at each end of the top flange for GLTV, and a 1%2" x 21%2" fillet weld at each end of the top flange for HGLTV. Weld-on applications produce maximum factored resistances listed. Uplift resistances do not apply to this application.
- · Web stiffeners are required with I-joists using this hanger style.
- Nailers and ledgers must be a minimum of 4x lumber to guarantee the resistances given in the tables. Thinner lumber or laminated veneer lumber used as a nailer must be evaluated by the Building Designer. The HGLTV series cannot be used with a nailer.

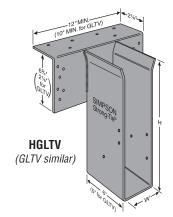
OPTIONS: • Hot-dipped galvanized: specify HDG.

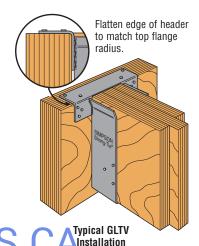
• See Hanger Options, page 231.

		Fasteners	3			Factored R	esistance		
80-4-1				Uplift		Nor	mal (K _D = 1	.00)	
Model No.	Ton	Face	Joist	(K _D = 1.15)	D.Fir-L	S-P-F	LVL ⁴	PSL	LSL
No.	Top	гасе	30121	lbs	lbs	lbs	lbs	lbs	lbs
				kN	kN	kN	kN	kN	kN
GLTV series	4 164	6-16d	6-16d	2145	10455	7470	10890	10745	8590
GLIV Selles	4-16d	0-100	0-100	9.54	46.51	33.23	48.44	47.8	38.21
HGLTV series	6-16d	12-16d	6-16d	2145	13070	9830	15365	11325	13795
		12-100	0-100	9.54	58.14	43.73	68.35	50.38	61.36

- 1. Uplift resistances have been increased 15% for short-term loading with no further increase allowed. Reduce resistance when other load durations govern.
- 2. Uplift loads only apply when "H" is 28" or les
- 3. S-P-F factored uplift resistance is 1520
- 4. Applies to LVL headers made primarily
- or Southern Pine. For LVL made primarily from Spruce Pine Fir or similar less dense veneers, use the values found in the S-P-F column.







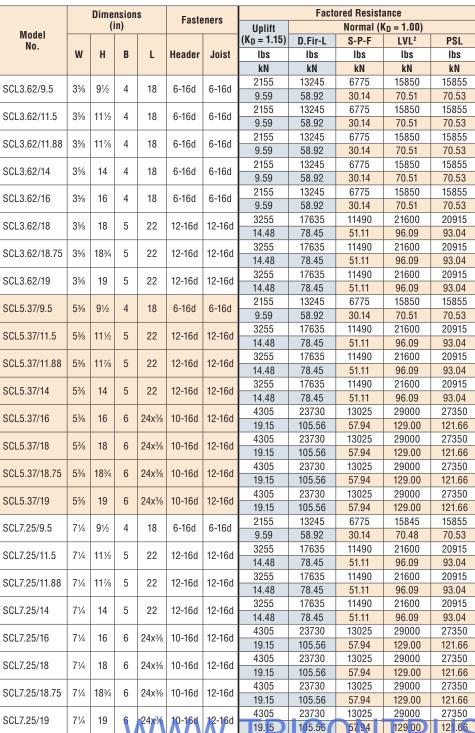
SCL High Capacity Top Flange Hangers

The SCL series of top flange hangers are high load capacity connectors designed for use with Structural Composite Lumber. The large top flange distributes the load to the carrying member and the fasteners are located specifically for structural composite lumber applications.

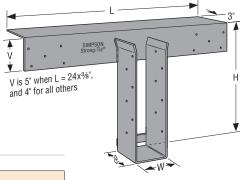
MATERIAL: Stirrups—3 gauge; Top flange—1/4" or 3/6" hot rolled angle, see table FINISH: Simpson Strong-Tie® gray paint

INSTALLATION: • Use all specified fasteners. See General notes.

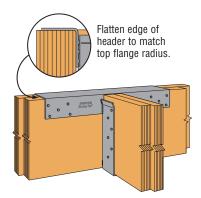
- All multiple members must be fastened together to act as one single unit.
- This series may be used for weld on application. Weld top flange using ¼" x 4" long fillet welds spaced at 7" on center with 2" return around corners.
- These hangers cannot be used with a nailer.







SCL



Typical SCL Installation

- Factored uplift resistances have been increased 15% for short term loading with no further increase allowed. Reduce when other load durations govern.
- Factored uplift resistances shown are for D.Fir-L. Multiply tabulated loads x 0.71 for either SPF joist or header.
- Applies to LVL headers made primarily from D.Fir-L, assuming φF_{CP} = 1092 psi and a specific gravity of 0.50. See LVL manufacturer specifications.
- 4. NAILS: 16d = 0.162" dia. x 3½" long. See pages 22-23 for other nail sizes and information.

EGQ High Capacity Hanger

WEINEERED



This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

The EGQ is a high capacity connector for use with Structural Composite Lumber beams. Utilizing the Simpson Strong-Tie® Strong Drive® SDS Heavy-Duty Connector screws makes installation fast and easy.

EGQ hangers are precisely fabricated to individual order requirements. The H dimension required must be specified.

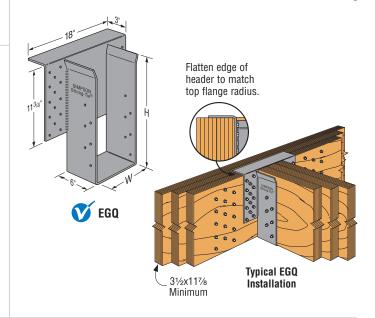
MATERIAL: Top flange—3 gauge; Stirrups—7 gauge

FINISH: Simpson Strong-Tie® gray paint

INSTALLATION: • Use all specified fasteners. See General Notes.

- Install with ¼"x3" Strong-Drive SDS Heavy-Duty Connector screws, which are provided with the EGQ. (Lag screws will not achieve the same load.)
- All multiple members must be fastened together to act as a single unit.
- Multiple member headers may require additional fasteners at hanger locations. Quantity and location to be determined by Designer. See SDS section for additional information and SDS screws applications.
- Minimum header depth shall be 11%".

OPTIONS: • Can be skewed or sloped. See Hanger Options page 231.



			_				Fa	ctored Resistan	ce	
1-1-4		Dimer (i		Fasto	eners	Uplift	D.Fir-L	PSL	LVL ¹	S-P-F LVL
Joist or Purlin	Model	(,			Орин	Normal	Normal	Normal	Normal
Size (in)	No.					$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.00)$	$(K_D = 1.00)$	$(K_D = 1.00)$
(111)		W	Min H	Header	Joist	lbs	lbs	lbs	lbs	lbs
						kN	kN	kN	kN	kN
3½	EGQ3.62-SDS3	35%	117/	28-1/4"x3" SDS	12-1/4"x3" SDS	9040	24915	25450	28410	19995
372	EGQ3.02-3D33	378	11%	20-74 X3 3D3	12-74 X3 3D3	40.21	110.83	113.21	126.38	88.95
51/4	ECOE EO CDC2	E1/	117/8	28-1/4"x3" SDS	12-1/4"x3" SDS	9040	27305	28030	30425	23930
374	EGQ5.50-SDS3 5½	372	1178	20-74 X3 3D3	12-74 X3 3D3	40.21	121.46	124.69	135.34	106.45
7	7 EGQ7.25-SDS3	71/4	117/8	28-1/4"x3" SDS	12-1/4"x3" SDS	9040	27305	30605	32435	23930
/		1 74	1178	20-74 X3 3D3	12-74 83 303	40.21	121.46	136.14	144.28	106.45

- Applies to LVL made primarily from Douglas Fir or Southern Pine. For LVL made primarily from other species, contact the LVL manufacturer for suitability.
- 2. "Min H" is the minimum joist height dimension that may be specified.
- 3. Use S-P-F LVL values for S-P-F glulam.
- 4. Multiply tabulated uplift values x 0.72 for S-P-F LVL

CSC Ceiling Support Clip / FSS Furring Stabilizer Strap

Provides 1" separation between the furring channel and joist to allow for the use of Thermafiber® insulation and the attachment of the furring channel to all joists. Provides an efficient sound barrier, and a one hour U.L. listed fire rating.

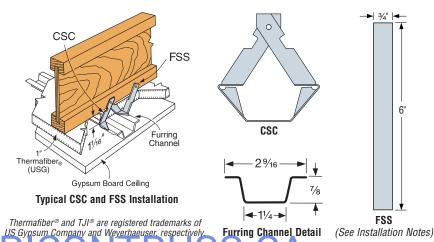
Field-form the FSS with the CSC to prevent furring channel rotation during installation. Furring channel must match dimensions shown to fit the CSC properly. U.L. listed. See Underwriters Laboratories, Inc., Design No. L530 for USG and TJI® I-joists.

MATERIAL: 24 gauge (minimum)

FINISH: Galvanized

INSTALLATION: • For CSC use 1-8dx1½ nail.

 For FSS use #8 self-tapping steel screw (not provided) into channel, twist 90°, bend upward and fasten to the side of joist bottom flange with screw or nail.



Engineered Wood & Structural Composite Lumber Connectors

						nsions	;		Fastener	\$			Fac	tored R				
Joist	Model	Web			(i	n)	ı				Uplift					= 1.00	<u> </u>	1
Size	No.	Stiff	Ga			_		Head	er		(K _D =1.15)			LVL	PSL	LSL		Masonry
		Reqd		W	Н	В	TF	Тор	Face	Joist	lbs kN	lbs kN	lbs kN	lbs kN	lbs kN	lbs kN	lbs kN	lbs kN
	LBV1.56/9.25	_	14	1%16	91/4	3	2½	6-16d	4-16d	2-10dx1½	435 1.94	3905 17.37	3125	3905 17.37	4410 19.62	4630	2200 9.79	_
1½ x 9¼	WP29.25	✓	12	1%16	91/4	4	23/16	2-16d	_	2-10dx1½		4430	3855	5950	5430	5980	_	_
	WM29.25	√	12	1%6	91/4	41/2	33/4	2-16d DPLX		2-10dx1½	_	19.71	17.15	26.47	24.15	26.60	_	6060
		V	12	1716			374				— 105	- 2625	- 1725		- 2480	- 2620	1695	26.96
	LT159	_	18	1%6	9½	2	1%	4-10d	2-10d	1-#8x1¼WS	0.47	11.68	7.67	11.39	11.03	11.65	7.54	_
	MIT29.5	_	16	1%6	9½	2½	25/16	4-16d	4-16d	2-10dx1½	375 1.67	3490 15.52	2420 10.77	3550 15.79	3025 13.46	3465 15.41	8.45	_
1½ x 9½	LBV1.56/9.5	_	14	1%6	9½	3	21/2	6-16d	4-16d	2-10dx1½	435 1.94	3905 17.37	3125 13.90	3905 17.37	4410 19.62	4630	2200 9.79	_
	WP29.5	✓	12	1%16	9½	4	23/16	2-16d	_	2-10dx1½	_	4430 19.71	3855 17.15	5950 26.47	5430 24.15	5980 26.60	_	_
	WM29.5	✓	12	1%6	9½	41/2	33/4	2-16d DPLX		2-10dx1½		19.71 —	— —					6060
											— 105	<u>-</u> 2625	- 1725			- 2620	1695	26.96
	LT151188	_	18	1%16	11%	2	1%	4-10d	2-10d	1-#8x1¼WS	0.47 375	11.68 3490	7.67 2420	11.39 3550	11.03 3025	11.65 3465	7.54 1900	_
	MIT211.88	_	16	1%6	11%	2½	25/16	4-16d	4-16d	2-10dx1½	1.67	15.52	10.77	15.79	13.46	15.41	8.45	_
1½ x 11%	LBV1.56/11.88	_	14	1%6	11%	3	2½	6-16d	4-16d	2-10dx1½	435 1.94	3905 17.37	3125 13.90	3905 17.37	4410 19.62	4630 20.60	9.79	_
	WP211.88	✓	12	1%16	11%	4	23/16	2-16d	_	2-10dx1½	_	4430 19.71	3855 17.15	5950 26.47	5430 24.15	5980 26.60	_	_
,	WM211.88	✓	12	1%6	11%	41/2	3¾	2-16d DPLX	_	2-10dx1½		_	_	_	_	_	_	6060
1½ x 14	LBV1.56/14	_	14	1%16	14	3	2½	6-16d	4-16d	2-10dx1½	435	3905	3125	3905	4410	4630	2200	26.96
											1.94 435	17.37 3905	13.90 3125	17.37 3905	19.62 4410	20.60 4630	9.79	_
1½ x 16	LBV1.56/16	_	14	1%6	16	3	2½	6-16d	4-16d	2-10dx1½	1.94 435	17.37 4990	13.90 4370	17.37 5835	19.62 5385	20.60 5820	9.79 2420	_
	BA1.81/7.25 (Min)	_	14	1 ¹³ ⁄ ₁₆	71/4	3	2½	6-16d	10-16d	2-10d x 1½	1.94	22.20	19.44	25.96	23.95	25.89	10.77	_
. 42/ . 71/	BA1.81/7.25 (Max)	✓	14	1 ¹³ ⁄ ₁₆	71/4	3	2½	6-16d	10-16d	8-10d x 1½	1960 8.72	5940 26.42	4370 19.44	6490 28.87	7075 31.47	6185 27.51	_	_
1¾ x 7¼	LBV1.81/7.25	_	14	1%16	71/4	3	2½	6-16d	4-16d	2-10dx1½	435 1.94	3905 17.37	3125 13.90	3905 17.37	4410 19.62	4630 20.60	2200 9.79	_
	WP1.81/7.25	✓	12	1%16	71/4	3½	23/16	2-16d	_	2-10dx1½	_	4430	3855	5950	5430	5980	_	_
											475	19.71		26.47	24.15			_
	ITS1.81/9.5	-	18	1%	97/16	2	1 ½16	4-10d	2-10d	_	175 0.78	2235 9.94	1690 7.52	2280 10.14	2005 8.92	2615 11.63	1375 6.12	_
	LT179	_	18	1 13/16	9½	2	1%	4-10d	2-10d	1-#8x11/4WS	105	2625	1725	2560	2480	2620	1695	_
	MIT9.5		16	1 13/16	9½	2½	25/16	4-16d	4-16d	2-10dx1½	0.47 375	11.68 3490	7.67 2420	11.39 3550	3025	3465	7.54 1900	_
											1.67 435	15.52 4990	10.77 4370	15.79 5835	13.46 5385	15.41 5820	8.45 2420	_
1¾ x 9½	BA1.81/9.5 (Min)	_	14	1 ¹³ / ₁₆	9½	3	2½	6-16d	10-16d	2-10d x 1½	1.94 1960	22.20 5940	19.44 4370	25.96 6490	23.95 7075		10.77	_ _
	BA1.81/9.5 (Max)	√	14	1 ¹³ ⁄ ₁₆	9½	3	2½	6-16d	10-16d	8-10d x 1½	8.72	26.42	19.44	28.87	31.47	27.51	_	_
	LBV1.81/9.5	_	14	1 13/16	9½	3	2½	6-16d	4-16d	2-10dx1½	435 1.94	3905 17.37	3125 13.90	3905 17.37	4410 19.62	4630 20.60	9.79	_ _
	WP9	✓	12	1 13/16	9½	3½	2¾16	2-16d	_	2-10dx1½	_	4430 19.71	3855 17.15	5950 26.47	5430 24.15		_	_
	WM9	✓	12	1 13/16	9½	41/2	3¾	2-16d DPLX		2-10dx1½	_	_	_	_	_	_	_	6060
											_	_		_	_	_	_	26.96

^{1.} When I-joist is used as a header, all header fasteners must be 10dx1½. See footnotes on pages 119 and 122 for reduction values when flange material is less than 1½" thick.

^{2.} See pages 118-125 for specific notes on individual model types.



						Nimo-	nsions							Fan	tored D	lesista	nce		8
			Web				nsions n)	i		Fastener	S	Uplift		гас			= 1.00)	
	Joist	Model	Stiff	Ga			<u>, </u>		Head	er		(K _D =1.15)	D.Fir-L	S-P-F		PSL	LSL		Masonry
	Size	No.	Reqd		W	Н	В	TF	Тор	Face	Joist	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs
-							_					kN 175	kN 2235	kN 1690	kN 2280	kN 2005	kN 2615	kN 1375	kN —
		ITS1.81/11.88	_	18	1%	11 ¹³ ⁄ ₁₆	2	17/16	4-10d	2-10d	_	0.78	9.94	7.52	10.14	8.92	11.63	6.12	_
		LT171188	_	18	1 13/16	11%	2	1%	4-10d	2-10d	1-#8x11/4WS	105 0.47	2625 11.68	1725 7.67	2560 11.39	2480	2620 11.65	1695 7.54	_
		MIT11 00		16	1 137	447/	01/	25/16	4 104	4 104	0.404541/	375	3490	2420	3550	3025	3465	1900	_
		MIT11.88		16	113/16	11%	2½	Z%16	4-16d	4-16d	2-10dx1½	1.67	15.52	10.77	15.79	13.46	15.41	8.45	_
	10/ 117/	BA1.81/11.88 (Min)	_	14	1 13/16	11%	3	2½	6-16d	10-16d	2-10dx1½	435 1.94	4990	4370 19.44	5835 25.96	5385	5820 25.89	2420 10.77	_
	1¾ x 11⅓	BA1.81/11.88 (Max)	√	14	1 13/16	11%	3	2½	6-16d	10-16d	8-10dx1½	1960	5940	4370	6490		6185	_	_
		, ,		4.4	410/	447/		01/	0.40.1	4.40-1	0.40.4.44/	8.72 435	26.42 3905	19.44 3125	28.87 3905	31.47 4410	27.51 4630	2200	_
		LBV1.81/11.88	_	14	1 13/16	11%	3	2½	6-16d	4-16d	2-10dx1½	1.94	17.37	13.90	17.37	19.62	20.60	9.79	_
		WP11	✓	12	1 13/16	11%	3½	23/16	2-16d	_	2-10dx1½		4430 19.71	3855 17.15	5950 26.47	5430 24.15	5980 26.60		_
		WM11	✓	12	113/16	11%	4½	3¾	2-16d DPLX	_	2-10dx1½		_	_	_	_	_		6060
			•								2 100/172	<u> </u>	2235	1690	<u>-</u> 2280	2005		1375	26.96
		ITS1.81/14	_	18	1%	1315/16	2	17/16	4-10d	2-10d	_	0.78	9.94	7.52	10.14	8.92	11.63	6.12	_
		LT1714	_	18	113/16	14	2	1%	4-10d	2-10d	1-#8x11/4 WS	105 0.47	2625 11.68	1725 7.67	2560 11.39	2480	2620 11.65	1695 7.54	_
		MIT1.81/14		16	1 13/16	14	2½	25/16	4-16d	4-16d	2-10dx1½	375	3490	2420	3550	3025	3465	1900	_
		IVII I 1.0 1/ 14		10	I '916	14	2 72	Z%16	4-10u	4-10u	Z-10UX 1 72	1.67	15.52	10.77	15.79	13.46	15.41	8.45	_
1	407 44	BA1.81/14 (Min)	_	14	1 13/16	14	3	2½	6-16d	10-16d	2-10dx1½	435 1.94	4990 22.20	4370 19.44	5835 25.96	5385 23.95	5820 25.89	2420 10.77	_
1	1¾ x 14	BA1.81/14 (Max)	✓	14	113/16	14	3	2½	6-16d	10-16d	8-10dx1½	1960	5940	4370	6490	7075	6185	-	-
		, ,										8.72 435	26.42 3905	19.44 3125	28.87 3905	31.47 4410	27.51 4630	2200	-
		LBV1.81/14	_	14	1 13/16	14	3	2½	6-16d	4-16d	2-10dx1½	1.94	17.37	13.90	17.37	19.62	20.60	9.79	_
		WP14	✓	12	1 13/16	14	3½	23/16	2-16d	_	2-10dx1½		4430 19.71	3855 17.15	5950 26.47	5430 24.15	5980 26.60		
		WM14	✓	12	1 13/16	14	4½	33/4	2-16d DPLX		2-10dx1½	_		-				_	6060
L		VV IVI 14	V	12	I 716	14	4/2	374	2-100 DF LX		Z-100X172	— 175	<u> </u>	1690	- 2280	2005		1375	26.96
		ITS1.81/16	_	18	1%	15 ¹⁵ /16	2	17/16	4-10d	2-10d	_	0.78	9.94	7.52	10.14	8.92	11.63	6.12	_
		LT1716	_	18	113/16	16	2	1%	4-10d	2-10d	1-#8x1¼ WS	105	2625	1725	2560	2480	2620	1695	_
		MIT4 04/40		10	4 12/	10	01/	05/	4 404	1 101	0.40441/	0.47 375	11.68 3490	7.67	11.39 3550	11.03 3025	11.65 3465	7.54	_ _
		MIT1.81/16	_	16	1 13/16	16	2½	25/16	4-16d	4-16d	2-10dx1½	1.67	15.52	10.77	15.79	13.46	15.41	8.45	_
	1¾ x 16	LBV1.81/16	_	14	1 13/16	16	3	2½	6-16d	4-16d	2-10dx1½	435 1.94	3905 17.37	3125 13.90	3905 17.37	4410 19.62	4630 20.60	9.79	_
		B1.81/16	✓	12	1 13/16	16	3	2½	6-10d	8-10d	6-10dx1½	1650	5265	3590	5825	5230	5965	_	_
												7.34 —	23.42 4430	15.97 3855		23.27 5430			_
		WP16	√	12	1 13/16	16	3½	23/16	2-16d	_	2-10dx1½	_	19.71			24.15		_	_
		WM16	✓	12	113/16	16	4½	3¾	2-16d DPLX	_	2-10dx1½		_	_	_	_	_		6060 26.96
ŀ		ITS2.06/9.5	_	18	21/8	97/16	2	17/16	4-10d	2-10d	_	175	2235	1690	2280	2005	2615	1375	
	2 x 9½	1102.00/3.0		10		3716		1710	7 100	2 100		0.78 435	9.94 3905	7.52 3125	10.14	8.92 4410		6.12 2200	_ _
		LBV2.06/9.5	_	14	21/16	9½	2½	2½	6-16d	4-16d	2-10dx1½	1.94	17.37		17.37	19.62	20.60	9.79	_
		ITS2.06/11.88	_	18	21/8	11%	2	17/16	4-10d	2-10d	_	175	2235	1690		2005		1375	_
	2 x 11 ⁷ / ₈	1 00 00 44 00		4.4	01/	447/	01/	01/	0.40.1	4.40.1	0.40-1-41/	0.78 435	9.94 3905	7.52 3125	10.14 3905	8.92 4410		6.12 2200	_
L		LBV2.06/11.88	_	14	21/16	11%	2½	2½	6-16d	4-16d	2-10dx1½	1.94	17.37			19.62		9.79	_
	0.4	ITS2.06/14	_	18	21/8	13 ¹⁵ /16	2	17/16	4-10d	2-10d	_	175 0.78	2235 9.94	1690 7.52	10.14	2005 8.92	2615 11.63	1375 6.12	_
	2 x 14	LBV2.06/14	_	14	21/16	14	2½	2½	6-16d	4-16d	2-10dx1½	435	3905	3125	3905	4410	4630	2200	_
-												1.94 175	17.37 2235			19.62 2005		9.79	_
	2 x16	ITS2.06/16	_	18	21/8	15 ¹⁵ /16	2	17/16	4-10d	2-10d	_	0.78	9.94	7.52	10.14	8.92	11.63	6.12	_
	۷ ۱۱۷	LBV2.06/16	_	14	21/16	16	2½	2½	6-16d	4-16d	2-10dx1½	435 1.94	3905 17.37	3125		4410 19.62		2200 9.79	_
		ITS2.06/9.5	_	18	21/8	97/16	2	1 ½16	4-10d	2-10d	_	175	2235	1690	2280	2005	2615	1375	_
	21/16 x 91/2	1102.00/3.0		10	2/8	3716	2	1 716	4-10U	2-10u		0.78	9.94	7.52	10.14		11.63	6.12	_
		LBV2.1/9.5	_	14	21/8	9½	2½	2½	6-16d	4-16d	2-10dx1½	435 1.94	3905 17.37			19.62		9.79	_

^{1.} When I-joist is used as a header, all header fasteners must be 10dx1½ See foonotes on pages 2. See pages 118-125 for specific notes on individual model types. 119 and 122 for reduction values when f ange material is less than $1 \frac{1}{2}$ " thick.



						Dimer	nsinns							Fac	tored R	Resista	nce		
			Web					'		Fastener	S	Uplift		- 1 40)	
TISCORTILOR TISCORTICO TISCORTILOR TISCORTILOR TISCORTILOR TISCORTILOR T			Stiff						Head	er			D.Fir-L	S-P-F	LVL	PSL	LSL	I-Joist	Masonry
	3126	NU.	Reqd		W	Н	В	TF	Ton	Face	Joist	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs
									TOP	гасс									kN
2% x 110 LBV2_1111 88		ITS2 06/11 88	_	18	21/8	11 13/16	2	17/16	4-10d	2-10d	_								_
LIVELY 111	2½6 x 11%				-/-	,	_	.,,,,											_
Time			_	14	21/8	11%	2½	21/2	6-16d	4-16d	2-10dx1½								
24% x 14 LBV2.1714													_						
		ITS2.06/14	—	18	21/8	13¹5⁄₁6	2	1 ½16	4-10d	2-10d	_		_						
Lay 1.1 Lay 1.1 Lay 1.1	21/16 x 14																		
2% x 16 LBV2.1716		LBV2.1/14	-	14	21/8	14	2½	2½	6-16d	4-16d	2-10dx1½								_
TS2.37/18	01/ 1/16	L DV0 1/16		1.1	01/	16	01/	01/	6 164	4 104	0.104511/	435	3905	3125	3905	4410	4630	2200	_
	2716 X 10	LBV2.1/10		14	278	10	272	Z 7/2	0-100	4-160	Z-100X172	1.94	17.37	13.90	17.37	19.62	20.60	9.79	_
T239		ITS2 37/0 5		18	27/40	Q7/4c	2	17/40	4-10d	2-10d		175	2235		2280	2005	2615		_
Lisy Color Fig.		1102.01/ 0.0		10	2/10	3716		1710	4 10u	2 100									_
LBV2.379.5	25/16 x 91/2	LT239	_	18	23/8	9½	2	15/8	4-10d	2-10d	1-#8x11/4 WS								
BBV237918																			
		LBV2.37/9.5	—	14	25/16	9½	21/2	21/2	6-16d	4-16d	2-10dx1½		_						
Transition Tra		ITS2.37.11.88	-	18	27/16	11 13/16	2	17/16	4-10d	2-10d	_								
29% x 11% WM3511.88		1.7004400		40	05/	447/	_	45/	4.40.1	0.40-1	4 //0 41/ 14/0	105	_						_
29% x 11%		L1231188	-	18	2%16	111//8	2	1%	4-100	2-100	1-#8X1½ WS	0.47	11.68	7.67	11.39	11.03	11.65	7.54	_
2% x 11% LBV2.37/11.88		MIT3511 88		16	25/46	117%	21%	25/40	4-16d	4-16d	2-10dv11/6	375	3490	2420	3550	3025	3465	1900	_
LBV2.37/11.88	25/16 x 117/8			10	2/10	1170	2/2	2/10	4 10u	4 10u	2 100X172		_						_
W3511.88	Z710 X 1170		_	14	25/16	11%	2½	21/2	6-16d	4-16d	2-10dx1½		_						_
W3511.88																			_
WM3511.88		W3511.88	✓	12	25/16	11%	2½	21/2	2-16d	_	2-10dx1½								
WM3511.88 V 12 2% 11% 3 34 2-16d DPLX - 2-10dx1½ - - - - - - - - 26.96																			
182.3/14		WM3511.88	✓	12	25/16	11%	3	3¾	2-16d DPLX	_	2-10dx1½	_	_	_	_	_	_	_	
LT2314		ITC2 27/1/		10	27/	1215/	2	17/	4-10d	2-104		175	2235	1690	2280	2005	2615	1375	_
Name		1102.07/14		10	2/16	10 716		1716	4-10u	Z-10u			_						_
2% x 18 MIT3514 — 16 2% 14 2½ 2% 4-16d 4-16d 2-10dx1½ 1.67 15.52 10.77 15.79 13.46 15.41 8.45 — WB3514 — 12 2% 14 3 3% 2-16d DPLX — 2-10dx1½ — 4.430 3855 5950 5430 5980 — 6.660 — 6.660		LT2314	_	18	23/8	14	2	1%	4-10d	2-10d	1-#8x11/4WS		_						
2% x 14 LBV2.37/14																			
LBV2.37/14		MIT3514	—	16	25/16	14	2½	25/16	4-16d	4-16d	2-10dx1½								
LBV2.3/14	25/16 x 14																		
WM3514		LBV2.37/14	-	14	25/16	14	2½	2½	6-16d	4-16d	2-10dx1½								_
WM3514		WD2514	/	10	25/	1/	21/	93/	0.164		0.10dv11/	_	4430	3855	5950	5430	5980	_	_
Trsc.37/16		WF3514	V	12	Z916	14	2 72	2916	Z-10u		Z-100X172	_	19.71	17.15	26.47	24.15	26.60	_	_
Trs2.37/16		WM3514	/	12	25/16	14	3	3¾	2-16d DPLX	_	2-10dx11/2		_	_	_	_			
Tisk														1000		-			26.96
LT2316		ITS2.37/16	—	18	27/16	15 ¹⁵ ⁄ ₁₆	2	1 ½16	4-10d	2-10d	_								
2%6 x 16 2%6 16 2½ 2%6 4-16d 4-16d 2-10dx1½ 2%6 x 16 2%6 16 2½ 2%6 4-16d 4-16d 2-10dx1½ 2%6 x 16 2%6 16 2½ 2%6 4-16d 4-16d 2-10dx1½ 2%6 x 16 2%7 x 1														_					
2½6 x 16 MIT3516 — 16 2½6 16 2½ 2½6 4-16d 4-16d 2-10dx1½ LBV2.37/16 — 14 2½6 16 2½ 2½ 6-16d 4-16d 2-10dx1½ WP3516 — 12 2½6 16 3 3¾ 2-16d DPLX — 14 2½6 16 3 3¾ 2-16d DPLX — 14 2½6 16 3 3¾ 2-16d DPLX — 14 2½6 16 3 3¾ 2-16d DPLX — 14 2½6 16 3 3¾ 2-16d DPLX — 15.52 10.77 15.79 13.46 15.41 8.45 — WM3518 — 16 2½ 2½6 6-16d 4-16d 2-10dx1½ — 4430 3855 5950 5430 5980 — — —		LT2316	_	18	2%	16	2	1%	4-10d	2-10d	1-#8x1¼WS								
2%6 x 16 LBV2.37/16		MITOE16		10	05/	16	01/	05/	4.104	4 104	0.104511/	375							_
LBV2.37/16	25/ ₆ x 16	IVIII 33 IO		10	Z%16	10	272	Z%16	4-100	4-160	Z-100X172	1.67							_
WP3516	2710 X 10	LBV2.37/16	_	14	25/16	16	21/2	21/2	6-16d	4-16d	2-10dx11/2								_
WM3516																			
WM3516		WP3516	✓	12	25/16	16	21/2	23/16	2-16d	_	2-10dx1½								
WM3516																			
MI 13518		WM3516	✓	12	2%16	16	3	3¾	2-16d DPLX	_	2-10dx1½								
25% x 18 LBV2.37/18 - 14 25% 18 2½ 2½ 6-16d 4-16d 2-10dx1½ 435 3905 3125 3905 4410 4630 2200 — LBV2.37/18 WP3518 V 12 25% 18 2½ 2½ 6-16d — 2-10dx1½ — 4430 3855 5950 5430 5980 — — WWM3518 V 12 25% 18 3 33% 2-16d DPLX — 2-10dx1½ — — — — — — 6060		MIT3518		16	25/40	18	21/6	25/40	4-10d	4-16d	2-10dv1½							1900	_
25/6 x 18 WP3518				10	2/16	10	L /2	L / 16	7 100	7 10u	L 100x172								_
2%s x 18 WP3518 V 12 2%s 18 2½ 2%s 2-16d — 2-10dx1½ — 4430 3855 5950 5430 5980 — — WM3518 V 12 2%s 18 3 3% 2-16d DPLX — 2-10dx1½ — — — — — — 6060		LBV2.37/18	_	14	25/16	18	21/2	21/2	6-16d	4-16d	2-10dx1½								
WP3518	25/16 x 18																		
WM3518		WP3518	V	12	25/16	18	21/2	23/16	2-16d	_	2-10dx1½								
WW3518 12 2%: 18 3 3% 2-16d DPLX - 2-10dV1%		\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	/	10	05/	10	0	027	0.404.001.11		0.40-1-41/								
		VVIVI3518	V	12	2%16	18	3	3%	2-160 DPLX		2-100x1½	_	_	_	_	_	_	_	

^{1.} When I-joist is used as a header, all header fasteners must be 10dx1½. See foonotes on pages 119 and 122 for reduction values when flange material is less than 1½" thick.

^{2.} See pages 118-125 for specific notes on individual model types.



					Dimer		3		Fastener	s			Fac	tored R				
Joist	Model	Web	0-		(i	n)					Uplift	D. Et I	0.0.5			= 1.00		D#
Size	No.	Stiff Reqd	Ga	w	Н	В	TF	Head	er	Joist	(K _D =1.15) lbs	D.FIT-L lbs	S-P-F	LVL	PSL lbs	LSL lbs	I-Joist lbs	Masonry lbs
		·		"				Тор	Face	00101	kN	kN	kN	kN	kN	kN	kN	kN
	MIT3520		16	25/16	20	2½	25/16	4-16d	4-16d	2-10dx1½	375	3490	2420	3550	3025	3465	1900	_
											1.67 435	15.52 3905	10.77 3125	15.79 3905	13.46 4410	15.41 4630	8.45 2200	<u> </u>
25/16 x 20	LBV2.37/20	√	14	25/16	20	2½	2½	6-16d	4-16d	2-10dx1½	1.94	17.37	13.90	17.37	19.62	20.60	9.79	_
2/16 X 2U	WP3520	✓	12	25/16	20	2½	23/16	2-16d	_	2-10dx1½		4430	3855	5950	5430	5980	_	_
ı											-	19.71	17.15	26.47	24.15	26.60	_	6060
	WM3520	√	12	25/16	20	3	3¾	2-16d DPLX		2-10dx1½	_	_	_	_	_	_	_	26.96
	ITS2.56/9.5	_	18	25/8	97/16	2	17/16	4-10d	2-10d	_	175 0.78	2235 9.94	1690 7.52	2280 10.14	2005 8.92	2615	1375 6.12	_
	17050		10	09/	01/	2	15/	4 104	0.404	1-#8x11/4WS	105	2625	1725	2560	2480	2620	1695	_
	LT259	_	18	2%16	9½		1%	4-10d	2-10d	1-#0X174 W S	0.47	11.68	7.67	11.39	11.03	11.65	7.54	_
2½ - 21/2	BA2.56/9.5 (Min)	_	14	29/16	9½	3	2½	6-16d	10-16d	2-10dx1½	435 1.94	4990 22.20	4370 19.44	5835 25.96	5385 23.95	5820 25.89	2420 10.77	_
x 9½	BA2.56/9.5 (Max)	✓	14	29/16	9½	3	2½	6-16d	10-16d	8-10dx1½	1960	5940	4370	6490	7075	6185	_	_
	B/(2.00/0.0 (Wax)	ľ	1-7	2710	3 72		L /2	0 100	10 100	0 100X172	8.72 435	26.42 3905	19.44 3125	28.87 3905	31.47 4410	27.51 4630	2200	_
	LBV2.56/9.5	_	14	2%16	9½	2½	2½	6-16d	4-16d	2-10dx1½	1.94	17.37	13.90	17.37	19.62	20.60	9.79	_
	WI39.5	✓	12	2%16	9½	2	2½	2-16d	_	2-10dx1½		2955	2375	3820	3190	_	_	
							-/-			2 100/172	— 175	13.15 2235	10.56 1690	16.99 2280	14.19	2615	1375	_
	ITS2.56/11.25	_	18	25/8	113/16	2	17/16	4-10d	2-10d	_	0.78	9.94	7.52	10.14	8.92	11.63	6.12	_
2½ x 11¼	LBV2.56/11.25	_	14	2%16	1111/4	2½	2½	6-16d	4-16d	2-10dx1½	435	3905	3125	3905	4410	4630	2200	_
						_					1.94	17.37 2955	13.90 2375	17.37 3820	19.62 3190	20.60	9.79	_
	WI311.25	√	12	2%16	1111/4	2	2½	2-16d	_	2-10dx1½	_	13.15	10.56	16.99	14.19	_	_	_
	ITS2.56/11.88	_	18	25/8	11 13/16	2	17/16	4-10d	2-10d	_	175 0.78	2235 9.94	1690 7.52	2280 10.14	2005 8.92	2615	1375 6.12	_
	LT051100		10	00/	447/		45/	4 404	0.404	4 //041/ MC	105	2625	1725	2560	2480	2620	1695	<u> </u>
	LT251188	_	18	2%16	11%	2	1%	4-10d	2-10d	1-#8x11/4WS	0.47	11.68	7.67	11.39	11.03	11.65	7.54	_
	MIT311.88	_	16	2%16	11%	2½	25/16	4-16d	4-16d	2-10dx1½	375 1.67	3490 15.52	2420	3550 15.79	3025 13.46	3465 15.41	1900 8.45	_
2½ - 21/16	BA2.56/11.88 (Min)		14	29/16	111//8	3	2½	6 164	10-16d	2-10dx1½	435	4990	4370	5835	5385	5820	2420	_
x 11%	BA2.30/11.00 (WIIII)		14	Z%16	1178	3	2 72	6-16d	10-100	Z-100X172	1.94	22.20	19.44				10.77	_
	BA2.56/11.88 (Max)	✓	14	29/16	111//8	3	2½	6-16d	10-16d	8-10dx1½	1960 8.72	5940 26.42	4370 19.44	6490 28.87	7075 31.47	6185 27.51		_
	LBV2.56/11.88		14	2%16	11%	2½	2½	6-16d	4-16d	2-10dx1½	435	3905	3125	3905	4410	4630	2200	_
	LBV2.00/11.00		17		1176	L /2	L /2	0 100	7 100	L TOUXT72	1.94	17.37 4430	13.90 3855	17.37 5950	19.62 5430	20.60 5980	9.79	_
	WPI311.88	√	12	2%16	1111//8	2½	23/16	2-16d	_	2-10dx1½		19.71	17.15	26.47	24.15	26.60	_	_
	ITS2.56/14	_	18	25/8	13¹5⁄₁6	2	17/16	4-10d	2-10d	_	175	2235	1690	2280	2005	2615	1375	_
											0.78 105	9.94 2625	7.52 1725	10.14 2560	8.92 2480		6.12 1695	<u> </u>
	LT2514	_	18	2%16	14	2	1%	4-10d	2-10d	1-#8x11/4WS	0.47	11.68	7.67	11.39	11.03	11.65	7.54	_
	MIT314	_	16	2%16	14	2½	25/16	4-16d	4-16d	2-10dx1½	375 1.67	3490 15.52	2420	3550	3025 13.46	3465 15.41	1900 8.45	_
ı	DAO 50/14 (Mim)		4.4	00/	4.4		01/	0.404	10 101	0.404.41/	435	4990	4370		5385		2420	_
21/2 - 29/16	BA2.56/14 (Min)		14	2%16	14	3	2½	6-16d	10-16d	2-10dx1½	1.94	22.20	19.44	25.96	23.95	25.89	10.77	_
x 14	BA2.56/14 (Max)	✓	14	2%16	14	3	2½	6-16d	10-16d	8-10dx1½	1960 8.72	5940 26.42	4370 19 44	6490 28.87	70/5 31 47	6185 27.51	_	_
	LBV2.56/14		14	2%16	14	2½	2½	6-16d	4-16d	2-10dx1½	435	3905	3125	3905	4410	4630	2200	_
	LBV2.30/14		14	2/16	14	2/2	2 /2	0-100	4-10u	Z-100X172	1.94	17.37 4430	13.90 3855	17.37 5950	19.62 5430	20.60 5980	9.79	_
	WPI314	✓	12	2%16	14	2½	23/16	2-16d	_	2-10dx1½		19.71	17.15	26.47	24.15		_	_
	WMI314	√	12	2%16	14	3	3¾	2-16d DPLX	_	2-10dx1½		_	_	_		_	_	6060
		•									— 175	<u> </u>	1690		2005	- 2615	1375	26.96
	ITS2.56/16		18	2%	15 ¹⁵ /16	2	17/16	4-10d	2-10d	_	0.78	9.94	7.52	10.14	8.92	11.63	6.12	_
	LT2516	_	18	29/16	16	2	1%	4-10d	2-10d	1-#8x11/4WS	105	2625	1725		2480		1695	_
2½ - 21/2											0.47 375	11.68 3490	7.67 2420	3550	11.03 3025		7.54 1900	_
x 16	MIT316		16	29/16	16	2½	25/16	4-16d	4-16d	2-10dx1½	1.67	15.52	10.77	15.79	13.46	15.41	8.45	_
	BA2.56/16 (Min)	_	14	2%16	16	3	2½	6-16d	10-16d	2-10dx1½	435 1.94	4990 22.20	4370	5835		5820 25.89	2420 10.77	_
	DAO 56/16 (Marx)	./	1.4	09/	10	0	01/	6 10 1	10.104	0.104541/	1960	5940	4370	6490	7075	6185	10.77	_
	BA2.56/16 (Max)	√	14	2%16	16	3	2½	6-16d	10-16d	8-10dx1½	8.72	26.42				27.51	_	_

^{1.} When I-joist is used as a header, all header fasteners must be $10dy 11/\sqrt{See}$ 2. See pages 118-125 for specific notes of individual model-types. eduction values when flange material is less than 1½" thick. foonotes on pag

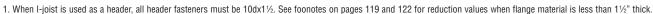


			ı					I					F					®
		Web			Dimer ii)	ısions n)			Fastener	s	Uplift		Fac	tored R Norn		nce = 1.00)	١	
Joist	Model	Stiff	Ga		(,		Head	er		(KD=1.15)	D.Fir-L	S-P-F	LVL	PSL	LSL		Masonry
Size	No.	Reqd		w	Н	В	TF	Тор	Face	Joist	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs
								ioh	Гасс		kN	kN	kN	kN	kN	kN	kN	kN
	LBV2.56/16	_	14	2%16	16	21/2	2½	6-16d	4-16d	2-10dx1½	435	3905	3125	3905	4410	4630	2200	_
2½ - 2%											1.94	17.37 4430	13.90 3855	17.37 5950	19.62 5430	20.60 5980	9.79	_
	WPI316	√	12	2%16	16	21/2	23/16	2-16d	_	2-10dx1½	_	19.71	17.15	26.47	24.15	26.60		_
(cont.)	WMI316	✓	12	2%16	16	3	3¾	2-16d DPLX	_	2-10dx1½	_	_	_	_	_	_	_	6060
	VVIVIIO	•	12	2716	10	3	J 74	2-100 DI LX		Z-100X172		_	_	_	_	_	_	26.96
	MIT318	_	16	2%16	18	21/2	25/16	4-16d	4-16d	2-10dx1½	375 1.67	3490 15.52	2420	3550 15.79	3025 13.46	3465 15.41	1900	_
-											375	4570	2705	3725	3220	3775	8.45	_
	HIT318	-	16	2%16	18	3	21/8	4-16d	6-16d	2-10dx1½	1.67	20.33	12.03	16.57	14.32	16.79	_	_
2½ x 18	LBV2.56/18	_	14	2%16	18	2½	2½	6-16d	4-16d	2-10dx1½	435	3905	3125	3905	4410	4630	2200	_
272 X 10	LD V Z.00/ 10		1-7	2710		2/2	L /2	0 100	4 100	2 100X172	1.94	17.37	13.90	17.37	19.62	20.60	9.79	_
	WPI318	✓	12	2%16	18	21/2	23/16	2-16d	_	2-10dx1½	_	4430 19.71	3855 17.15	5950 26.47	5430 24.15	5980 26.60	_	_
						_					_		—				_	6060
	WMI318	✓	12	2%16	18	3	3¾	2-16d DPLX		2-10dx1½	_	_	_	_	_	_	_	26.96
	MIT320	✓	16	2%16	20	2½	25/16	4-16d	4-16d	2-10dx1½	375	3490	2420	3550	3025	3465	1900	_
		ļ ·									1.67 450	15.52 4570	10.77 2705	15.79 3725	13.46 3220	15.41 3775	8.45	_
	HIT320	-	16	2%16	20	3	21/8	4-16d	6-16d	2-10dx1½	2.00	20.33	12.03	16.57	14.32	16.79		_
01/ 00	1 DV0 CC/00		4.4	00/	00	01/	01/	0.404	4 404	0.40441/	435	3905	3125	3905	4410	4630	2200	_
2½ x 20	LBV2.56/20		14	2%16	20	2½	2½	6-16d	4-16d	2-10dx1½	1.94	17.37	13.90	17.37	19.62	20.60	9.79	_
	WPI320	✓	12	2%16	20	2½	23/16	2-16d	_	2-10dx1½		4430	3855	5950	5430	5980		_
_											_	19.71	17.15	26.47	24.15	26.60		6060
	WMI320	✓	12	2%16	20	3	3¾	2-16d DPLX	_	2-10dx1½								26.96
	LUTOOO		10	09/	00	0	07/	4 404	0.404	0.40441/	450	4570	2705	3725	3220	3775	_	_
	HIT322		16	2%16	22	3	2%	4-16d	6-16d	2-10dx1½	2.00	20.33	12.03	16.57	14.32	16.79	_	_
	LBV2.56/22	_	14	29/16	22	21/2	21/2	6-16d	4-16d	2-10dx1½	435	3905	3125	3905	4410	4630	2200	_
2½ x 22											1.94	17.37 4430	13.90 3855	17.37 5950	19.62 5430	20.60 5980	9.79	_
	WPI322	✓	12	2%16	22	21/2	23/16	2-16d	_	2-10dx1½	_	19.71	17.15	26.47	24.15	26.60	_	_
	HWI322	/	11	2%16	22	4	2½	4-16d	_	4-10dx1½		6900	5285	7695	5810	6870	_	
	11001022	٧	11	2/16	22	4	2 /2	4-10u		4-10ux172		30.69	23.51	34.23	25.85	30.56	_	_
	HIT324	_	16	2%16	24	3	2 1/8	4-16d	6-16d	2-10dx1½	450 2.00	4570 20.33	2705 12.03	3725 16.57	3220 14.32	3775 16.79	_	_
											435	3905	3125	3905	4410	4630	2200	_
2 ½ x 24	LBV2.56/24	_	14	2%16	24	21/2	2½	6-16d	4-16d	2-10dx1½	1.94	17.37	13.90	17.37	19.62	20.60	9.79	_
	WPI324	✓	12	2%16	24	2½	23/16	2-16d	_	2-10dx1½		4430	3855		5430			
		Ť		27.0		-/-	2710	2 .00		2 100////2	450	19.71		26.47		26.60		_
	HIT326	-	16	2%16	26	3	21/8	4-16d	6-16d	2-10dx1½	450 2.00	4570 20.33	2705 12 03		3220 14.32	3775 16.79	_	_
01/ 00	1 DV0 50/00		4.4	00/	00	01/	01/	0.404	4.40.4	0.40.141/	435	3905	3125	3905	4410	4630	2200	_
2½ x 26	LBV2.56/26		14	2%16	26	2½	2½	6-16d	4-16d	2-10dx1½	1.94	17.37	13.90				9.79	_
	WPI326	1	12	2%16	26	2½	23/16	2-16d	_	2-10dx1½		4430	3855			5980		_
											<u> </u>	19.71 3905	3125	26.47 3905	24.15 4410	26.60 4630	2200	_
	LBV2.56/28	_	14	2%16	28	21/2	2½	6-16d	4-16d	2-10dx1½	1.94	17.37	13.90		19.62	20.60	9.79	_
2½ x 28	WDI220	√	10	29/	20	21/	03/	0.164		2.10dv11/	_	4430	3855	5950	5430	5980	_	_
	WPI328	v	12	2%16	28	2½	2¾16	2-16d	_	2-10dx1½	_	19.71		26.47	24.15	26.60	_	_
	LBV2.56/30	_	14	2%16	30	21/2	2½	6-16d	4-16d	2-10dx1½	435	3905	3125		4410	4630	2200	_
2½ x 30											1.94	17.37 4430	13.90 3855			20.60 5980	9.79	_
	WPI330	✓	12	2%16	30	2½	23/16	2-16d	_	2-10dx1½	_	19.71		26.47	24.15	26.60	_	_
	LBV3.12/9.25	_	14	31/8	91/4	2½	2½	6-16d	4-16d	2-10dx1½	435	3905	3125	3905	4410	4630	2200	_
			1.7	5/0	J/4	-/4	-/4	J 100	. 100	L IOUNT/2	1.94	17.37	13.90		19.62	20.60	9.79	_
3 x 91/4	WPI29.25-2	✓	12	31/8	91/4	21/2	2¾16	2-16d	_	2-10dx1½	_	4430 19.71	3855 17.15	5950 26.47	5430 24.15	5980 26.60	_	_
	14/14/00 OF C	/	40	017	017	017	037	0.404.551.11		0.40.1		— 19.7 I	—					6060
	WM29.25-2	√	12	31/8	91/4	21/2	3¾	2-16d DPLX	_	2-10d	_	_	_	_	_	_	_	26.96

^{1.} When I-joist is used as a header, all header fasteners must be 10dx1½. See foonotes on pages 119 and 122 for reduction values when flange material is less than 1½" thick. 2. See pages 118-125 for specific notes on individual model types.

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LT2-159	Joist Masonry Ibs	---	---
No. Rend W	Ibs Ibs KN KN KN KN KN KN KN K		
LT2-159	kN kN 1695 — 7.54 — 1900 — 8.45 — 2200 — 9.79 — — 6060 — 26.96 2200 — — 6060 — 26.96 1695 — 7.54 — 1900 — 8.45 — 2200 — 9.79 — — — 6060 —		
MIT29.5-2	7.54 — 1900 — 8.45 — 2200 — 9.79 — — 6060 — 26.96 2200 — 9.79 — — 6060 — 26.96 1695 — 7.54 — 1900 — 8.45 — 2200 — 9.79 — — 6060 — 6060		
No. No.	8.45 — 2200 — 9.79 — — 6060 — 26.96 2200 — 9.79 — — 6060 — 26.96 1695 — 7.54 — 1900 — 8.45 — 2200 — 9.79 — — 6060		
BV3.12/9.5	2200 — 9.79 — — 6060 — 26.96 2200 — 9.79 — — 6060 — 26.96 1695 — 7.54 — 1900 — 8.45 — 2200 — 9.79 — — 6060		
WP29.5-2	6060 - 26.96 2200 - 9.79 6060 - 26.96 1695 - 7.54 - 1900 - 8.45 - 2200 - 9.79 6060		
WM29.5-2	- 6060 - 26.96 2200 - 9.79 6060 - 26.96 1695 - 7.54 - 1900 - 8.45 - 2200 - 9.79 6060		
LBV3.12-11.25	2200 — 9.79 — — 6060 — 26.96 1695 — 7.54 — 1900 — 8.45 — 2200 — 9.79 — — 6060		
3 x 11¼ WP211.25-2	6060 - 26.96 1695 - 7.54 - 1900 - 8.45 - 2200 - 9.79 6060		
WM211.25-2	6060 26.96 1695 7.54 1900 8.45 2200 9.79 6060		
LT2-151188	1695 — 7.54 — 1900 — 8.45 — 2200 — 9.79 — — — 6060		
MIT211.88-2 — 16 3% 11% 2½ 2¾6 4-16d 4-16d 2-10dx1½ 375 3490 2420 3550 3025 3465 1 1.67 15.52 10.77 15.79 13.46 15.41 8 1.67 15.52 10.77 15.79 13.46 15.41 8 1.67 15.52 10.77 15.79 13.46 15.41 8 1.67 15.52 10.77 15.79 13.46 15.41 8 1.67 15.52 10.77 15.79 13.46 15.41 8 1.67 15.52 10.77 15.79 13.46 15.41 8 1.67 15.52 10.77 15.79 13.46 15.41 8 1.67 15.52 10.77 15.79 13.46 15.41 8 1.67 15.52 10.77 15.79 13.46 15.41 8 1.67 15.52 10.77 15.79 13.46 15.41 8 1.67 15.52 10.77 15.79 13.46 15.41 8 1.67 15.79 13.46 15.41 8 1.67 15.79 13.46 15.41 8 1.67 15.79 13.46 15.41 8 1.67 15.79 13.46 15.41 8 1.67 15.79 13.46 15.41 8 1.67 15.79 13.46 15.41 8 1.67 15.79 13.46 15.41 8 1.67 15.79 13.46 15.41 8 1.67 15.79 13.46 15.41 8 1.67 15.79 13.46 15.41 8 1.67 15.79 13.46 15.41 8 1.67 15.79 13.46 15.41 8 1.67 15.79 13.46 15.41 8 1.67 15.79 13.46 15.41 8 1.67 15.79 13.46 15.41 8 1.67 15.79 13.46 15.41 8 1.67 15.41	1900 — 8.45 — 2200 — 9.79 — — — — 6060		
3 x 11% LBV3.12/11.88	8.45 — 2200 — 9.79 — — — — 6060		
3 x 11⅓ LBV3.12/11.88	2200 — 9.79 — — — — 6060		
3 x 11% LBV3.12/11.88 — 14 3% 11% 2½ 2½ 6-16d 4-16d 2-10dx1½ 1.94 17.37 13.90 17.37 19.62 20.60 9 WP211.88-2 ✓ 12 3½ 11% 2½ 2¾6 2-16d — 2-10dx1½ — 4430 3855 5950 5430 5980 WM211.88-2 ✓ 12 3½ 11% 2½ 3¾ 2-16d DPLX — 2-10d —	9.79 — — — — — — 6060		
WP211.88-2 ✓ 12 3½ 11½ 2½ 2¾6 2-16d — 2-10dx1½ — 4430 3855 5950 5430 5980 — 19.71 17.15 26.47 24.15 26.60 — 2-10dx1½ — 19.71 17.15 26.47 24.15 26.60 — — — — — — — — — — — — — — — — — — —	 6060		
WP211.88-2 ✓ 12 3% 11% 2½ 2½ 6-16d — 2-10dx1½ — 19.71 17.15 26.47 24.15 26.60 — 2-10dx1½ — 19.71 17.15 26.47 24.15 26.60 — 2-10dx1½ — 19.71 17.15 26.47 24.15 26.60 — 2-10dx1½ — 19.71 17.15 26.47 24.15 26.60 — 2-10dx1½ — 2-10dx1½ — 2-10dx1½ 435 3905 3125 3905 4410 4630 2 1.94 17.37 13.90 17.37 19.62 20.60 \$\frac{1}{2}\$\$ 3x 16 LBV3.12/16 — 14 3½ 16 2½ 2½ 6-16d 4-16d 2-10dx1½ 435 3905 3125 3905 4410 4630 2 1.94 17.37 13.90 17.37 19.62 20.60 \$\frac{1}{2}\$\$\$ 1.94 17.37 13.90 17.37 19.62 20.60 \$\frac{1}{2}\$	 6060		
WM211.88-2 ✓ 12 3½ 11½ 2½ 3¾ 2-16d DPLX — 2-10d — — — — — — — — — — — — — — — — — — —	— 6060		
3 x 16 LBV3.12/16 — 14 3% 16 2½ 2½ 6-16d 4-16d 2-10dx1½ 1.94 17.37 13.90 17.37 19.62 20.60 9 3 x 16 LBV3.12/16 — 14 3% 16 2½ 2½ 6-16d 4-16d 2-10dx1½ 435 3905 3125 3905 4410 4630 2 LBV3.67.25 — 14 3% 7½ 3½ 3½ 6-16d 4-16d 2-10dx1½ 435 3905 3125 3905 4410 4630 2	— 26.96		
3 x 16 LBV3.12/16 — 14 31/8 16 21/2 21/2 6-16d 4-16d 2-10dx11/2 435 3905 3125 3905 4410 4630 2 1.94 17.37 13.90 17.37 19.62 20.60 5	2200 — 9.79 —		
LBV3 56/7 25 14 3% 7½ 2½ 6.16d	2200 —		
BV3 56/7 25	9.79 —		
	2200 —		
316 v 716 1.94 17.37 13.90 17.37 19.62 20.60 S	9.79 —		
WPU3.56/7.25 ✓ 12 3% 7¼ 3 2% 3-16d 4-16d 6-10dx1½ 1665 6390 6390 6825 7085 5980 7.41 28.43 28.43 30.36 31.52 26.60			
1153.56/9.5	1375 — 6.12 —		
111350 1 • Z ° 1 18 39% Q1% 2 1 1% A-10d 2-10d 2-#8V1 \/ \(\text{VV} \)	1695 —		
0.47 11.08 7.07 11.03 11.03 11.03	7.54 —		
	1900 — 8.45 —		
435 4990 4370 5835 5885 5820 2	2420 —		
BA3 56/9 5 (MIN) 1 — 1 14 1 3% 1 9% 1 3 1 2% 1 6-160 1 10-160 1 2-100 1 % 1	10.77 —		
BA3.56/9.5 (MAX) 14 3% 9½ 3 2½ 6-16d 10-16d 8-10dx1½ 1960 5940 4370 6490 7075 6185			
BA3.56/9.5 (MAX) 14 3%6 9½ 3 2½ 6-16d 10-16d 8-10dx1½ 8.72 26.42 19.44 28.87 31.47 27.51	_ _		
-118/3.56/9.5 $-121/3.96/9.5$ $-121/3.96/9.5$ $-121/9.5$ $-121/9.5$ $-121/9.5$ $-121/9.5$	2200 —		
1.94 17.37 13.90 17.37 19.62 20.60 8	9.79 —		
HB3.56/9.5 ✓ 10 3% ₆ 9½ 3½ 3 6-16d 16-16d 10-16d 10-1			
WP149.5 ▼ Z 3%16 9%2 Z%2 Z%16 Z-100			
HW49.5			
HWII3 56/9 5 10 3% 91/6 31/6 21/6 4-16d 4-16d 6-10d 1775 10170 8875 10170 8325 8925			
CLTV2 FO			
HCLTV3 59 A 7 33% 916 6 276 6.16d 12.16d 6.16d 2145 13070 9830 15365 11325 13795			
9.54 58.14 43.73 68.35 50.38 61.37			
SCL3.62/9.5 ✓ 3 3%6 9½ 4 3 — 6-16d 6-16d			



^{2.} See pages 118-125 for specific notes in individual noted types. 3. For 16 and 18 gauge, $3\frac{1}{2}$ wite Highst langes, web-stiffeners are when the fact 0.36 kN).



					Dimer		3		Fastener	s			Fac	tored R				
Joist	Model	Web	Co		(iı	n)					Uplift (K _D =1.15)	D Fiv I	евг		nal (K _d PSL	= 1.00		Masonry
Size	No.	Stiff Reqd	Ga	w	Н	В	TF	Head	er	Joist	(KD=1.13)	D.FIT-L	Ibs	LVL	lbs	lbs	I-JOIST	lbs
				VV	"		٠٠.	Тор	Face	30131	kN	kN	kN	kN	kN	kN	kN	kN
	ITC0 FC/11 00	✓3	10	25/	4413/	0	17/	4.104	0.104		175	2235	1690	2280	2005	2615	1375	_
	ITS3.56/11.88	V	18	35/8	11 13/16	2	17/16	4-10d	2-10d	_	0.78	9.94	7.52	10.14	8.92	11.63	6.12	_
	LT351188	√ 3	18	3%16	11%	2	1%	4-10d	2-10d	2-#8x11/4WS	105	2625	1725	2560	2480	2620	1695	_
		1									0.47 375	11.68 3490	7.67	11.39 3550	11.03 3025	11.65 3465	7.54 1900	_
	MIT411.88	√ 3	16	3%16	11%	21/2	25/16	4-16d	4-16d	2-10dx1½	1.67	15.52	10.77	15.79	13.46	15.41	8.45	_
							2				435	4990	4370	5835	5385	5820	2420	_
	BA3.56/11.88 (Min)	_	14	3%16	11%	3	2½	6-16d	10-16d	2-10dx1½	1.94	22.20	19.44	25.96	23.95	25.89	10.77	_
	BA3.56/11.88 (Max)	1	14	3%16	11%	3	2½	6-16d	10-16d	8-10dx1½	1960	5940	4370	6490	7075	6185	_	_
	27.10.007.7.100 (111.07.7)	ļ .		07.10				0.00		0.1000.172	8.72	26.42	19.44	28.87	31.47	27.51		_
	LBV3.56/11.88	—	14	3%16	11%	21/2	2½	6-16d	4-16d	2-10dx1½	435 1.94	3905 17.37	3125	3905 17.37	4410 19.62	4630	9.79	_
											1650	5940	3910	6490	5230	6185	J.7 J	_
	B3.56/11.88	✓	12	3%16	11%	2½	2½	6-16d	8-16d	6-16d	7.34	26.42	17.39	28.87	23.27	27.51	_	_
	HB3.56/11.88	/	10	3%16	11%	3½	2½	6-16d	16-16d	10-16dx2½	3555	9335	5945	9525	9240	10475	_	_
3½ x 11%	1120.00/11.00	, ·		0710	1170	0,2	-/-	0 100	10 100	10 TOUXE72	15.81	41.53	26.45		41.10	46.60	_	_
	WPI411.88	✓	12	3%16	11%	21/2	23/16	2-16d	_	2-10dx1½		4430 19.71	3855 17.15	5950 26.47	5430 24.15	5980 26.60	_	_
											1665	6390	6390	6825	7085	5980		
	WPU3.56/11.88	√	12	3%16	11%	3	25/16	3-16d	4-16d	6-10dx1½	7.41	28.43	28.43		31.52		_	_
	HWI411.88	/	11	3%16	11%	2½	2½	4-16d	_	2-10d		6900	5285	7695	5810	6870	_	_
	11001411.00	•	- ''	3 716	1178	2 /2	2/2	4-100		Z-100	_	30.69	23.51	34.23	25.85	30.56	_	_
	HWU3.56/11.88	✓	10	3%16	11%	31/4	2½	4-16d	4-16d	6-10d	1775 7.90	10170 45.24	8875 39.48	10170 45.24	8325 37.03	8925 39.70		_
											2145	10455	7470				<u> </u>	_
	GLTV3.511	✓	7	3%16	11%	5	21/8	4-16d	6-16d	6-16d	9.54	46.51	33.23	48.44	47.80	38.21	_	_
	LICLEVO 511	1	7	29/	447/	6	07/	C 1C4	10.164	6 164	2145	13070	9830				_	_
	HGLTV3.511	v	1	3%16	11%	O	27/8	6-16d	12-16d	6-16d	9.54	58.14		68.35		61.37	_	_
	SCL3.62/11.88	1	3	35/8	11%	4	3	_	6-16d	6-16d	2155	13245	6775			_		_
											9.59	58.92	30.14	70.51	70.53	_	_	6060
	WM3.56/11.88	✓	12	3%16	11%	21/2	3¾	2-16d DPLX	_	2-10d								26.96
	IT00 50 // 4	/3	40	05/	40157	_	47/	4.40.1	0.40.1		175	2235	1690	2280	2005	2615	1375	_
	ITS3.56/14	√ ³	18	35/8	13 ¹⁵ / ₁₆	2	17/16	4-10d	2-10d	_	0.78	9.94	7.52	10.14	8.92	11.63	6.12	_
	LT3514	√ 3	18	3%16	14	2	15/8	4-10d	2-10d	2-#8x11/4WS	105	2625	1725	2560	2480	2620	1695	_
											0.47	11.68	7.67		11.03		7.54	_
	MIT414	√ 3	16	3%16	14	21/2	25/16	4-16d	4-16d	2-10dx1½	375 1.67	3490 15.52	_	3550 15.79	3025	3465	1900 8.45	_
	DAO 50 (44 (44))			201			01/	0.401	10.10.1	0.401.41/	435	4990	4370		5385		2420	_
	BA3.56/14 (Min)	_	14	3%16	14	3	2½	6-16d	10-16d	2-10dx1½	1.94	22.20		25.96			10.77	_
	BA3.56/14 (Max)	1	14	3%16	14	3	2½	6-16d	10-16d	8-10dx1½	1960	5940	4370		7075	6185	_	_
	()										8.72	26.42		28.87	31.47		2200	_
	LBV3.56/14	—	14	3%16	14	21/2	2½	6-16d	4-16d	2-10dx1½	435 1.94	3905 17.37	3125	3905 17.37	4410 19.62	4630	2200 9.79	_
01/ 4.4	D0 50/14	/	10	00/	4.4	01/	01/	0.40.1	0.40.1	0.404	1650	5940	3910		5230			_
3½ x 14	B3.56/14	√	12	3%16	14	2½	2½	6-16d	8-16d	6-16d	7.34	26.42	17.39	28.87	23.27	27.51	_	_
	HB3.56/14	1	10	3%16	14	3½	3	6-16d	16-16d	10-16d	3555	9335	5945			10475	_	_
											15.81	41.53 4430		42.37 5950		46.60 5980	_	_
	WPI414	✓	12	3%16	14	2½	23/16	2-16d DPLX	_	2-10dx1½	_	19.71		26.47			_	_
	WIDLIO FC/44	/	10	20/	1.1	0	05/	0.404	4 404	C 10d::11/	1665	6390	6390		7085		_	_
	WPU3.56/14	√	12	3%16	14	3	25/16	3-16d	4-16d	6-10dx1½	7.41	28.43	28.43	30.36	31.52	26.60	_	_
	HWI414	1	11	3%16	14	2½	2½	4-16d	_	2-10d	_	6900	5285		5810		_	_
											— 1775	30.69 10170		34.23 10170			<u> </u>	_
	HWU3.56/14	✓	10	3%16	14	31/4	2½	4-16d	4-16d	6-10d	1775 7.90	45.24		45.24			_	_
	CLTV2 514	/	7	20/	4.4	Г	07/	4.40-1	6.404	0.404	2145	10455		10890			_	_
	GLTV3.514	√	7	3%16	14	5	27/8	4-16d	6-16d	6-16d	9.54	46.51		48.44			_	_

^{1.} When I-joist is used as a header, all header fasteners must be 10dx1½. See foonotes on pages 119 and 122 for reduction values when flange material is less than 1½" thick.

2. See pages 118-125 for specific notes on individual model types. 3. For 16 and 18 gauge, 3½° wide 1-joist hanners, web sufferers are requ en the factored reaction is grea han 2330 lbs. (10.36 kN)

Engineered Wood & Structural Composite Lumber Connectors



					Dimer				Fastener	s			Fac		Resista			
Joist	Model	Web	0-		(iı	n)					Uplift	D. Etc. I	0.0.5			= 1.00		D#
Size	No.	Stiff Reqd		\ \		ь	7.5	Head	er	loiet	(K _D =1.15)			LVL	PSL	LSL		Masonry
		IIICqu		W	Н	В	TF	Top	Face	Joist	lbs kN	lbs kN	lbs kN	lbs kN	lbs kN	lbs kN	lbs kN	lbs kN
	11017110 514		_	20/	4.4		07/	0.40.1	10.10.1	0.40.1	2145	13070	9830	15365			_	_
	HGLTV3.514	✓	7	3%16	14	6	27/8	6-16d	12-16d	6-16d	9.54	58.14	43.73	68.35	50.38	61.37	_	_
3½ x 14	SCL3.62/14	/	3	35%	14	4	3	_	6-16d	6-16d	2155	13245	6775	15850	_	_	_	
(cont.)	0020.02/14	,		078					0 100	0 100	9.59	58.92	30.14	70.51	70.53	_	_	_
	WMI414	✓	12	3%16	14	2½	3¾	2-16d DPLX	_	2-10d		_		_	_	_		6060
											— 175	2235	1690	2280	2005	2615	1375	26.96
	ITS3.56/16	√ 3	18	3%	15 ¹⁵ ⁄16	2	1 ½16	4-10d	2-10d	_	0.78	9.94	7.52	10.14	8.92	11.63	6.12	_
	LT3516	√ 3	18	3%16	16	2	15/8	4-10d	2-10d	2-#8x11/4WS	105	2625	1725	2560	2480	2620	1695	_
	L13310	V	10	3716	10		178	4-10u	2-10u	Z-# 0X 1 74 VV 3	0.47	11.68	7.67	11.39	11.03	11.65	7.54	_
	MIT416	✓3	16	3%16	16	2½	25/16	4-16d	4-16d	2-10dx1½	375	3490	2420	3550	3025	3465	1900	
											1.67 435	15.52 4990	10.77 4370	15.79 5835	13.46 5385	15.41 5820	8.45 2420	_
	BA3.56/16 (Min)	-	14	3%16	16	3	2½	6-16d	10-16d	2-10dx1½	1.94	22.20	19.44		23.95		10.77	
	DAG 50/40 /84 \			20/	40		04 (0.40.1	40.40.1	0.401.447	1960	5940	4370	6490	7075	6185	_	_
	BA3.56/16 (Max)	√	14	3%16	16	3	2½	6-16d	10-16d	8-10dx1½	8.72	26.42	19.44	28.87	31.47	27.51	_	_
	B3.56/16	/	12	3%16	16	2½	2½	6-16d	8-16d	6-16d	1650	5940	3910	6490	5230	6185	_	_
	20.00, 10	ļ ,		07.10				0.00		0.00	7.34	26.42	17.39	28.87	23.27	27.51	_	_
	HB3.56/16	√	10	3%16	16	3½	3	6-16d	16-16d	10-16d	3555 15.81	9335	5945 26.45	9525 42.37	9240	10475	_	
												4430	3855	5950	5430	5980		
3½ x 16	WPI416	√	12	3%16	16	2½	23/16	2-16d	_	2-10dx1½	_	19.71	17.15	26.47	24.15	26.60	_	_
	WPU3.56/16	✓	12	29/	16	3	05/	0.164	4 104	6-10dx1½	1665	6390	6390	6825	7085	5980	_	_
	WF03.30/10	V	12	3%16	10	٥	25/16	3-16d	4-16d	0-10ux 1 72	7.41	28.43	28.43		31.52	26.60	_	_
	HWI416	1	11	3%16	16	2½	2½	4-16d	_	2-10d		6900	5285	7695	5810	6870	_	_
			-								4775	30.69	23.51		25.85		_	_
	HWU3.56/16	✓	10	3%16	16	31/4	2½	4-16d	4-16d	6-10d	1775 7.90	10170 45.24	8875 39.48	10170 45.24	8325 37.03	8925 39.70	_	
	_										2145	10455	7470		10745		_	_
	GLTV3.516	✓	7	3%16	16	5	27/8	4-16d	6-16d	6-16d	9.54	46.51	33.23	48.44	47.80		_	_
	HGLTV3.516	1	7	3%16	16	6	27/8	6-16d	12-16d	6-16d	2145	13070	9830	15365			_	_
	11021 10.010	<u> </u>	<u>'</u>	0710	-10		278	0 100	12 100	0 100	9.54	58.14	43.73		50.38		_	_
	SCL3.62/16	✓	3	35/8	16	4	3	_	6-16d	6-16d	2155	13245	6775		15855		_	_
											9.59	58.92	30.14	70.51	70.53	_	_	6060
	WMI416	✓	12	3%16	16	2½	3¾	2-16d DPLX	_	2-10d	_	_	_	_	_	_	_	26.96
	MIT418	√ 3	16	3%16	18	2½	25/16	4-16d	4-16d	2-10dx1½	375	3490	2420	3550	3025	3465	1900	_
	IVII 14 10	V	10	3716	10	272	Z716	4-10u	4-10u	Z-100X172	1.67	15.52	10.77	15.79	13.46		8.45	_
	HIT418	✓3	16	3%16	18	3	23/8	4-16d	6-16d	2-10dx1½	450	4570		3725		3775	_	_
											2.00 435	20.33 3905	3125	16.57 3905			2200	_
	LBV3.56/18	-	14	3%16	18	2½	2½	6-16d	4-16d	2-10dx1½	1.94	17.37			19.62		9.79	_
	LIDO 50/40	/	10	20/	40	01/		0.404	10 104	10 101	3555	9335	5945		9240		_	_
	HB3.56/18	√	10	3%16	18	3½	3	6-16d	16-16d	10-16d	15.81	41.53			41.10		_	_
	WPI418	1	12	3%16	18	2½	23/16	2-16d	_	2-10dx1½		4430	3855		5430		_	_
											1005	19.71			24.15		_	_
	WPU3.56/18	✓	12	3%16	18	3	25/16	3-16d	4-16d	6-10dx1½	1665 7.41	6390 28.43	6390		7085 31.52		_	
3½ x 18				201	40	04/	04/	4.40.1		0.40.1		6900	5285		5810		_	_
	HWI418	✓	11	3%16	18	2½	2½	4-16d		2-10d	_			34.23	25.85	30.56	_	_
	HWU3.56/18	1	10	3%16	18	31/4	2½	4-16d	4-16d	6-10d	1775	10170	8875		8325		_	_
		ļ .	-			ļ					7.90	45.24			37.03		_	_
	GLTV3.518	✓	7	3%16	18	5	27/8	4-16d	6-16d	6-16d	2145 9.54	10455 46.51			10745 47.80		_	_
		-	_		<u> </u>						2145	13070			11325		_	_
	HGLTV3.518	V	7	3%16	18	6	27/8	6-16d	12-16d	6-16d	9.54	58.14			50.38		_	_
	SCL3.62/18	√	3	3%16	18	5	3	_	12-16d	12-16d	3255	17635					_	_
	0010.02/10	_	٥	J 716	10	J	٠	_	12-10U	14-10U	14.48	78.45	51.11		93.04		_	
	WMI418	✓	12	3%16	18	2½	3¾	2-16d DPLX	_	2-10d	_	_	_	_	_	_	_	6060
			\bot								_			_	_	—	_	26.96

^{1.} When I-joist is used as a header, all header fasteners must be 10dx1½. See foonotes on pages 119 and 122 for reduction values when flange material is less than 1½" thick.

2. See pages 118-125 for specific notes on individual model types.

3. For 16 and 18 gauge, 3½" wide I-joist hangers, web stiffeners are required when the factored reaction is creater than 2320 lbs. (10.36 k N).



	I							I					F	d 17				®
		Web				nsions n)	;		Fastener	s	Uplift		Fac	tored R		nce = 1.00	<u> </u>	
Joist	Model	Stiff	Ga		, <u>, , , , , , , , , , , , , , , , , , </u>	,		Head	er		(K _D =1.15)	D.Fir-L	S-P-F	LVL	PSL	LSL		Masonry
Size	No.	Reqd		W	н	В	TF			Joist	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs
								Тор	Face		kN	kN	kN	kN	kN	kN	kN	kN
	GLTV3.56/18.75	/	7	3%16	18¾	5	27/8	4-16d	6-16d	6-16d	2145	10455	7470	10890		8590	_	_
	GE1 V0.00/10.70	ľ		0710	1074			1 100	0 100	0 100	9.54	46.51	33.23	48.44	47.80	38.21	_	_
3½ x 18¾	HGLTV3.56/18.75	✓	7	3%16	18¾	6	27/8	6-16d	12-16d	6-16d	2145	13070	9830		11325		_	_
											9.54	58.14	43.73		50.38		_	_
	SCL3.62/18.75	✓	3	3%16	18¾	5	3	_	12-16d	12-16d	3255		51.11	21600 96.09	93.04			_
											14.48 375	78.45 3490	2420	3550	3025	3465	1900	_
	MIT420	√ ³	16	3%16	20	2½	25/16	4-16d	4-16d	2-10dx1½	1.67	15.52	10.77	15.79	13.46	15.41	8.45	_
		12									450	4570	2705	3725	3220	3775	_	_
	HIT420	√ 3	16	3%16	20	3	23/8	4-16d	6-16d	2-10dx1½	2.00	20.33	12.03	16.57	14.32	16.79	_	_
	L DV/2 EC/20		11	29/	20	01/	01/	6 164	4 104	0.104911/	435	3905	3125	3905	4410	4630	2200	_
	LBV3.56/20	-	14	3%16	20	2½	2½	6-16d	4-16d	2-10dx1½	1.94	17.37	13.90	17.37	19.62	20.60	9.79	_
	HB3.56/20	1	10	3%16	20	3½	3	6-16d	16-16d	10-16d	3555	9335	5945	9525	9240	10475	_	_
	1180.00/20	Ľ	10	0710	20	072		0 100	10 100	10 100	15.81	41.53	26.45	42.37	41.10	46.60	_	_
	WPI420	1	12	3%16	20	2½	23/16	2-16d	_	2-10dx1½		4430	3855	5950	5430	5980		_
3½ x 20		-									-	19.71	17.15	26.47	24.15	26.60	_	_
	WPU3.56/20	✓	12	3%16	20	3	25/16	3-16d	4-16d	6-10dx1½	595 2.65	6390 28.43	6390 28.43	6825 30.36	7085	5980	_	_
		٠.									2.00	6900	5285	7695	5810	6870		
	HWI420	✓	11	3%16	20	2½	2½	4-16d	_	2-10d	_	30.69	23.51		25.85	30.56	_	_
	0171/0 500		_	00/		_	07/	4.40.1	0.40.1	0.40.1	2145	10455	7470	10890			_	_
	GLTV3.520	V	7	3%16	20	5	27/8	4-16d	6-16d	6-16d	9.54	46.51	33.23	48.44	47.80	38.21	_	_
	HGLTV3.520	1	7	3%16	20	6	27/8	6-16d	12-16d	6-16d	2145	13070	9830	15365	11325	13795		_
	11011 V3.320	V	′	3/16	20	0	2/8	0-10u	12-10u	0-100	9.54	58.14	43.73	68.35	50.38	61.37	_	_
	WMI420	1	12	3%16	20	2½	3¾	2-16d DPLX	_	2-10d		_	_	_	_	_	_	6060
		<u> </u>									450	4570			_		_	26.96
	HIT422	√ 3	16	3%16	22	3	23/8	4-16d	6-16d	2-10dx1½	450 2.00	4570 20.33	2705 12.03	3725 16.57	3220 14.32	3775 16.79		_
											435	3905	3125	3905	4410	4630	2200	_
	LBV3.56/22	-	14	3%16	22	2½	2½	6-16d	4-16d	2-10dx1½	1.94	17.37	13.90	17.37	19.62	20.60	9.79	_
	LIDO 50/00		10	20/	00	01/		0.404	10 10 1	10 10 1	3555	9335	5945	9525	9240	10475	_	_
3½ x 22	HB3.56/22	✓	10	3%16	22	3½	3	6-16d	16-16d	10-16d	15.81	41.53	26.45	42.37	41.10	46.60	_	_
372 X ZZ	WPI422	1	12	3%16	22	2½	23/16	2-16d	_	2-10dx1½		4430	3855	5950	5430	5980	_	_
	***************************************	Ľ		0710		-/-	2710	2 100		L 100X172		19.71	17.15	26.47	24.15	26.60	_	_
	WPU3.56/22	✓	12	3%16	22	3	25/16	3-16d	4-16d	6-10dx1½	595	6390	6390	6825	7085	5980		_
											2.65	28.43 6900	5285	7695	5810	26.60 6870	_	_
	HWI422	✓	11	3%16	22	2½	2½	4-16d	_	4-10d		30.69	23.51		25.85			
		/2				_					450	4570	2705	3725	3220	3775	_	_
	HIT424	√ ³	16	3%16	24	3	2%	4-16d	6-16d	2-10dx1½	2.00	20.33			14.32		_	_
	LBV3.56/24		14	3%16	24	2½	2½	6-16d	4-16d	2-10dx1½	435	3905	3125	3905	4410	4630	2200	_
	LD V 3.30/24		14	3716	24	2/2	2/2	0-10u	4-100	Z-100X172	1.94	17.37	13.90	17.37		20.60	9.79	_
	HB3.56/24	1	10	3%16	24	3½	3	6-16d	16-16d	10-16d	3555	9335	5945	9525		10475	_	_
3½ x 24		<u> </u>									15.81	41.53	26.45	42.37	41.10		_	_
	WPI424	✓	12	3%16	24	2½	23/16	2-16d	_	2-10dx1½		4430 19.71	3855 17.15	5950 26.47	5430	5980 26.60		
											595	6390	6390	6825	7085			_
	WPU3.56/24	✓	12	3%16	24	3	25/16	3-16d	4-16d	6-10dx1½	2.65	28.43	28.43		31.52		_	_
	LIMIADA	1	44	29/	0.4	01/	01/	4 404		4 404	_	6900	5285	7695	5810	6870	_	_
	HWI424	√	11	3%16	24	2½	2½	4-16d	_	4-10d	_	30.69				30.56	_	_
	HIT426	✓3	16	3%16	26	3	2%	4-16d	6-16d	2-10dx1½	450	4570	2705	3725	3220	3775	_	_
		Ļ				_			- /00		2.00	20.33		16.57	14.32		_	_
3½ x 26	LBV3.56/26	_	14	3%16	26	2½	2½	6-16d	4-16d	2-10dx1½	435	3905	3125	3905	4410	4630	2200	_
		1									1.94 3555	17.37 9335	13.90 5945	17.37 9525		20.60	9.79	_
	HB3.56/26	✓	10	3%16	26	3½	3	6-16d	16-16d	10-16d	15.81					46.60		_
	1								<u> </u>		10.01	71.00	20.40	72.07	71.10	TU.00		

^{1.} When I-joist is used as a header, all header fasteners must be 10dx1½. See foonotes on pages 119 and 122 for reduction values when flange material is less than 1½" thick.

2. See pages 118-125 for specific notes on individual model types.

3. For 16 and 18 gauge, 3½" wide '-joist hangers, web stiffeners are required when the factored leaction is greater than 2330 lbs. (10.26 kN).



loict						sions			Fastener	S		ı	Fac		Resista			
Joist	Model	Web	00		(iı	1)		Hood	0.4		Uplift	D Fir I	евг			= 1.00	í –	Macaneu
Size	No.	Stiff Reqd	Ga			_		Head	er	1-1-1	(KD=1.15)			LVL	PSL	LSL		Masonry
		nequ		W	Н	В	TF	Тор	Face	Joist	lbs kN	lbs kN	lbs kN	lbs kN	lbs kN	lbs kN	lbs kN	lbs kN
\	WPI426	✓	12	3%16	26	2½	23/16	2-16d	_	2-10dx1½	_	4430 19.71	3855 17.15	5950 26.47	5430 24.15	5980	_	_
3½ x 26	WPU3.56/26	✓	12	3%16	26	3	25/16	3-16d	4-16d	6-10dx1½	595	6390	6390	6825	7085	5980	_	_
(cont.)	LIMI140C		44	09/	00	01/	01/	4 404		4 40 4	2.65 —	28.43 6900	28.43 5285	30.36 7695	31.52 5810	26.60 6870	_	_
1	HWI426	✓	11	3%16	26	2½	2½	4-16d	_	4-10d	- 405	30.69	23.51	34.23		30.56	_	_
ι	LBV3.56/28	-	14	3%16	28	2½	2½	6-16d	4-16d	2-10dx1½	435 1.94	3905 17.37	3125 13.90	3905 17.37	4410 19.62	4630	9.79	_
I	HB3.56/28	✓	10	3%16	28	3½	3	6-16d	16-16d	10-16d	3555 15.81	9335 41.53	5945 26.45	9525 42.37	9240 41.10	10475 46.60		_
3½ x 28	WPI428	✓	12	3%16	28	2½	23/16	2-16d	_	2-10dx1½	- 13.01	4430	3855	5950	5430	5980	_	_
372 X 20	VVF1420	•	12	3/16	20	2 /2	2716	2-10u		2-100X172	— 595	19.71 6390	17.15 6390	26.47 6825	24.15 7085	26.60 5980	_	_
1	WPU3.56/28	✓	12	3%16	28	3	25/16	3-16d	4-16d	6-10dx1½	2.65	28.43	28.43	30.36	31.52	26.60	_	_
											_	6900	5285	7695	5810	6870	_	_
	HWI428	✓	11	3%16	28	2½	2½	4-16d	_	4-10d	_	30.69	23.51	34.23	25.85	30.56	_	_
	I B) (0.50 (0.0			00/	00	04 (04 (0.40.1	4.40.1	0.401.447	435	3905	3125	3905	4410	4630	2200	_
	LBV3.56/30	_	14	3%16	30	2½	2½	6-16d	4-16d	2-10dx1½	1.94	17.37	13.90	17.37	19.62	20.60	9.79	_
Γ.	LIDO 50/00		40	00/	00	01/	_	0.40.1	40.40.4	40.40.1	3555	9335	5945	9525	9240	10475	_	_
	HB3.56/30	✓	10	3%16	30	3½	3	6-16d	16-16d	10-16d	15.81	41.53	26.45	42.37	41.10	46.60	_	_
3½ x 30	WDI 400		40	00/	00	01/	027	0.40:1		0.40-1-41/	_	4430	3855	5950	5430	5980	_	_
\	WPI430	✓	12	3%16	30	2½	23/16	2-16d	_	2-10dx1½	_	19.71	17.15	26.47	24.15	26.60	_	_
Γ,	111111111111	/	44	20/	20	01/	01/	4 104		4 104	_	6900	5285	7695	5810	6870	_	
	HWI430	✓	11	3%16	30	2½	2½	4-16d	_	4-10d	_	30.69	23.51	34.23	25.85	30.56	_	_
,	WPI432	✓	12	3%16	32	2½	23/16	2-16d	_	2-10dx1½	_	4430	3855	5950	5430	5980	_	_
3½ x 32	WF1432	v	12	3 / 16	32	Z /2	2716	2-10u		Z-10UX172	_	19.71	17.15	26.47	24.15	26.60	_	_
	HWI432	/	11	3%16	32	2½	2½	4-16d	_	4-10d		6900	5285	7695	5810	6870	_	
	11001402	•	11	0716	02	L /2	2/2	+ 10u		+ 10u	_	30.69	23.51	34.23	25.85	30.56	_	_
4 x 9½ l	LBV4.12/9.5	_	14	41/8	9½	2½	2½	6-16d	4-16d	2-10dx1½	435	3905	3125	3905	4410	4630	2200	
T X 0/2	LD V 4.12/ 0.0		17	476	372	L /2	L /2	0 100	7 100	L 100X172	1.94	17.37	13.90	17.37	19.62		9.79	_
4 x 11% L	LBV4.12/11.88	_	14	41/8	11%	2½	2½	6-16d	4-16d	2-10dx11/2	435	3905	3125	3905	4410	4630	2200	_
17, 1170	2212,00			1,0	,	_,_	-,,,	0 100		2 100////2	1.94	17.37	13.90	17.37	19.62	20.60	9.79	_
4 x 14 l	LBV4.12/14	_	14	41/8	14	2½	2½	6-16d	4-16d	2-10dx1½	435	3905	3125	3905	4410	4630	2200	_
	·										1.94	17.37	13.90	17.37	19.62	20.60	9.79	_
4 x 16 L	LBV4.12/16	_	14	41/8	16	21/2	21/2	6-16d	4-16d	2-10dx1½	435	3905	3125	3905	4410	4630	2200	_
											1.94	17.37	13.90	17.37	19.62		9.79	_
1	MIT4.28/9.5	_	16	43/32	91/2	21/2	25/16	4-16d	4-16d	2-10dx11/2	375	3490	2420				1900	_
41/8 x 91/2											1.67 435	15.52 3905	10.77 3125	15.79 3905	13.46 4410		8.45 2200	_
l	LBV4.28/9.5	_	14	43/32	91/2	21/2	21/2	6-16d	4-16d	2-10dx1½	1.94	17.37		17.37	_	20.60	9.79	_
											375	3490	2420		3025		1900	_
1	MIT4.28/11.88	_	16	43/32	11%	21/2	25/16	4-16d	4-16d	2-10dx1½	1.67	15.52	10.77		13.46		8.45	_
41/4 x 117/4											435	3905	3125	3905	4410		2200	_
	LBV4.28/11.88		14	43/32	11%	21/2	21/2	6-16d	4-16d	2-10dx1½	1.94	17.37		17.37	_	20.60	9.79	_
											375	3490	2420		3025		1900	_
	MIT4.28/14	_	16	43/32	14	2½	25/16	4-16d	4-16d	2-10dx1½	1.67	15.52	10.77		13.46		8.45	_
41/8 x 14	L DV/4 00/4 4		4.4	427	4.4	01/	01/	0.404	4 404	0.40441/	435	3905	3125				2200	_
	LBV4.28/14	_	14	43/32	14	2½	2½	6-16d	4-16d	2-10dx1½	1.94	17.37	13.90	17.37	19.62	20.60	9.79	_
41/ y 10	I DV/4 00/40		1.1	13/	10	01/	01/	6 101	4 10 1	0.104541/	435	3905	3125	3905			2200	_
4½ x 16 l	LBV4.28/16		14	43/32	16	2½	2½	6-16d	4-16d	2-10dx1½	1.94	17.37	13.90	17.37	19.62	20.60	9.79	_
	MIT250 5 2		16	437	01/	21/	25/	1.164	1_164	2_10dv11/	375	3490	2420	3550	3025	3465	1900	_
ľ	MIT359.5-2		16	43/4	9½	2½	25/16	4-16d	4-16d	2-10dx1½	1.67	15.52	10.77	15.79	13.46	15.41	8.45	_
45% x 9½ L	LBV4.75/9.5	_	14	43/4	9½	2½	2½	6-16d	4-16d	2-10dx1½	435	3905	3125	3905	4410		2200	_
7/8 A 3/2 L	LUV7.1 J/3.J		14	+ 74	372	~ 72	∠72	0-100	4-10u	∠-10UX I 72	1.94	17.37	13.90	17.37	19.62	20.60	9.79	_
Γ,	WP359.5-2	✓	12	43/4	9½	2½	25/16	3-16d	_	2-10d	_	4430	3855	5950			_	_
1,	VVI UUU.U~L	V	14	774	3 /2	L /2	∠/16	0-10u		∠-10U	_	19.71	17.15	26.47	24.15	26.60	_	_

^{1.} When I-joist is used as a header, all header fasteners must be 10dx1½. See foonotes on pages 119 and 122 for reduction values when flange material is less than 1½" thick. 2. See pages 118-125 for specific lotes on individual model types.



					Dimer		;		Fastener	s			Fac	tored R				
Joist	Model	Web Stiff	Ga		(iı	1)		Head	Or		Uplift (KD=1.15)	D Fir-I	S-P-F	LVL	PSL	= 1.00		Masonry
Size	No.	Reqd		w	н	В	TF			Joist	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs
						_		Тор	Face		kN	kN	kN	kN	kN	kN	kN	kN
	MIT3511.88-2	_	16	43/4	11%	2½	2½	4-16d	4-16d	2-10dx1½	375	3490	2420	3550	3025	3465	1900	
											1.67 435	15.52 3905	10.77 3125	15.79 3905	13.46 4410	15.41 4630	8.45 2200	_
	LBV4.75/11.88	-	14	4¾	11%	21/2	25/16	6-16d	4-16d	2-10dx1½	1.94	17.37	13.90	17.37	19.62	20.60	9.79	_
4% x 11%	WP3511.88-2	1	12	43/4	11%	2½	2½	3-16d	_	2-10dx1½	_	4430	3855	5950	5430	5980	_	_
	WF3311.00-2	V	12	474	1178	2 72	2 72	3-10u		Z-100X172	_	19.71	17.15	26.47	24.15	26.60	_	_
	WM3511.88-2	✓	12	43/4	11%	21/2	25/16	2-16d DPLX	_	2-10d		_		_		<u> </u>		6060 26.96
	MITOGAAO		40	40/	4.4	01/	05/	4.40.4	4.40.4	0.40.141/	375	3490	2420	3550	3025	3465	1900	
	MIT3514-2		16	43/4	14	2½	25/16	4-16d	4-16d	2-10dx1½	1.67	15.52	10.77	15.79	13.46	15.41	8.45	_
	LBV4.75/14	l —	14	43/4	14	2½	2½	6-16d	4-16d	2-10dx1½	435	3905	3125	3905	4410	4630	2200	
4% x 14											1.94 —	17.37 4430	13.90 3855	17.37 5950	19.62 5430	20.60 5980	9.79	_
	WP3514-2	✓	12	43/4	14	2½	25/16	3-16d	_	2-10dx1½	_	19.71	17.15	26.47	24.15	26.60	_	_
	WM3514-2	1	12	43/4	14	2½	3¾	2-16d DPLX	_	2-10d		_	_	_	_	_	_	6060
		ľ									375	3490	2420	3550	3025	3465	1900	26.96
	MIT4.75/16	—	16	4¾	16	21/2	25/16	4-16d	4-16d	2-10dx1½	1.67	15.52	10.77	15.79	13.46	15.41	8.45	
	LBV4.75/16		14	43/4	16	2½	2½	6-16d	4-16d	2-10dx1½	435	3905	3125	3905	4410	4630	2200	_
45% x 16	LBV4.73/10		14	474	10	2 72	2 72	0-10u	4-10u	Z-100X172	1.94	17.37	13.90	17.37	19.62	20.60	9.79	_
	WP3516-2	✓	12	4¾	16	21/2	25/16	3-16d	_	2-10dx1½		4430 19.71	3855 17.15	5950 26.47	5430 24.15	5980 26.60	_	_
												19.71		20.47	24.13	20.00		6060
	WM3516-2	✓	12	4¾	16	2½	3¾	2-16d DPLX		2-10d	_	_	_	_	_	_	_	26.96
	LBV4.75/18	_	14	43/4	18	2½	2½	6-16d	4-16d	2-10dx1½	435	3905	3125	3905	4410	4630	2200	_
											1.94	17.37 4430	13.90 3855	17.37 5950	19.62 5430	20.60 5980	9.79	_
4% x 18	WP3518-2	√	12	43/4	18	21/2	25/16	3-16d	_	2-10dx1½		19.71	17.15	26.47	24.15	26.60		_
	WM3518-2	/	12	43/4	18	2½	3¾	2-16d DPLX	_	2-10d	_	_	_	_	_	_	_	6060
	VV IVI33 10-2	,	12	774	10	2 /2	J 74	2-100 DI LX		Z-10u	-		-	_	_	_		26.96
	LBV4.75/20	—	14	43/4	20	2½	2½	6-16d	4-16d	2-10dx1½	435 1.94	3905 17.37	3125	3905 17.37	4410 19.62	4630	9.79	_
45/ 00	WD0500.0	/	40	40/	00	01/	05/	0.404		0.40-1-41/		4430	3855	5950	5430	5980		
4% x 20	WP3520-2	√	12	4¾	20	2½	25/16	3-16d	_	2-10dx1½	_	19.71	17.15	26.47	24.15	26.60	_	_
	WM3520-2	✓	12	43/4	20	21/2	3¾	2-16d DPLX	_	2-10d		_	_	_	_	_		6060
											375	3490	2420	3550	3025	3465	1900	26.96
	MIT39.5-2	-	16	51/8	9½	2½	25/16	4-16d	4-16d	2-10dx1½	1.67	15.52	10.77	15.79	13.46	15.41	8.45	_
5 x 9½	LBV5.12/9.5	_	14	51/8	9½	2½	2½	6-16d	4-16d	2-10dx1½	435	3905		3905			2200	_
											1.94	17.37 4430	13.90 3855	17.37	19.62 5430	20.60 5980	9.79	_
	WPI39.5-2	✓	12	51/8	91/2	21/2	25/16	3-16d	_	2-10dx1½	_	19.71		26.47				_
	MIT311.88-2	_	16	51/8	11%	2½	25/16	4-16d	4-16d	2-10dx1½	375	3490	2420	3550	3025	3465	1900	_
	WIITOT1.00 Z		10	078	1176	L /2	2710	4 100	4 100	2 100X172	1.67	15.52	10.77		13.46		8.45	_
5 x 11%	LBV5.12/11.88	✓	14	51/8	111//8	21/2	2½	6-16d	4-16d	2-10dx1½	435 1.94	3905 17.37	3125 13 90	3905 17.37	4410 19.62	4630	9.79	_
	W/DI311 99.3		10	5 1/	117/	21/	25/	3-164		2-10dv11/	-	4430	3855	5950	5430	5980	_	_
	WPI311.88-2		12	51/8	11%	2½	25/16	3-16d	_	2-10dx1½	_	19.71		26.47	24.15	26.60	_	_
	MIT314-2	—	16	51//8	14	21/2	25/16	4-16d	4-16d	2-10dx1½	375 1.67	3490 15.52	2420	3550	3025 13.46	3465	1900 8.45	_
F 4.	L D) (E 40 (44			F		0.11	64.1	0.40:	4 40 :	0.401	435	3905	3125	3905		4630	2200	_
5 x 14	LBV5.12/14	_	14	51/8	14	2½	2½	6-16d	4-16d	2-10dx1½	1.94	17.37	13.90	17.37	19.62	20.60	9.79	_
	WPI314.2	✓	12	51//8	14	2½	25/16	3-16d	_	2-10dx1½		4430	3855					_
											375	19.71 3490	2420	26.47 3550		26.60 3465	1900	_
	MIT5.12/16	-	16	51/8	16	21/2	25/16	4-16d	4-16d	2-10dx1½	1.67	15.52	10.77		13.46		8.45	_
	LBV5.12/16		14	51/8	16	2½	2½	6-16d	4-16d	2-10dx1½	435	3905	3125	3905	4410	4630	2200	_
5 x 16				0,0	.0			0.700	00	2 .03/(1/2	1.94	17.37		17.37		20.60	9.79	_
	HB5.12/16	✓	10	51/8	16	3½	3	6-16d	16-16d	10-16d	3555 15.81	9335		9525 42.37		10475	_	_
	WDI216-2	./	10	51/	16	21/	25/	2.164		2-10dv11/	-	4430		5950			_	_
	WPI316-2	√	12	51/8	16	2½	25/16	3-16d	_	2-10dx1½	_	19.71	17.15	26.47	24.15	26.60	_	_

^{1.} When I-joist is used as a header all header fastene's must be 10xx1½. See frontes of pages 119 and 122 for reduction values when flange material is less than 1½" thick. 2. See pages 118-125 for specific notes on individual model types.

Engineered Wood & Structural Composite Lumber Connectors



Engineered Wood & Structural Composite Lumber Connectors

					Dimer		;		Fastener	s			Fac	tored F				
Joist	Model	Web	00		(i	n)					Uplift	D Fiv I	e n r			= 1.00	<u></u>	Macaneu
Size	No.	Stiff Reqd	Ga	w	Н	В	TF	Head	er	Joist	(K _D =1.15) lbs	D.FIT-L	S-P-F	LVL	PSL lbs	LSL	I-JOIST lbs	Masonry lbs
		noqu		VV	п п	В	11	Top	Face	30181	kN	kN	kN	kN	kN	kN	kN	kN
	DE 40/40		40	F4.	40	04/	04 (0.401	0.401	0.40.1	1650	5940	3910	6490	5230	6185	_	— KIV
	B5.12/18	✓	12	51/8	18	2½	2½	6-16d	8-16d	6-16d	7.34	26.42	17.39	28.87	23.27	27.51	_	_
5 x 18	HB5.12/18	1	10	51/8	18	3½	3	6-16d	16-16d	10-16d	3555	9335	5945	9525	9240	10475	_	_
3 x 10	1100.12/10	<u> </u>	10	378	10	072		0 100	10 100	10 100	15.81	41.53	26.45	42.37	41.10	46.60	_	_
	WPI318-2	✓	12	51/8	18	2½	25/16	3-16d	_	2-10dx1½		4430	3855	5950	5430	5980		_
											— 1650	19.71 5940	17.15 3910	26.47 6490	24.15 5230	26.60 6185		_
	B5.12/20	✓	12	51/8	20	2½	2½	6-16d	8-16d	6-16d	7.34	26.42	17.39	28.87	23.27	27.51		_
F 00	LIDE 40/00	/	10	F1/	00	01/	_	0.404	10 101	40.404	3555	9335	5945	9525	9240	10475	_	_
5 x 20	HB5.12/20	✓	10	51/8	20	3½	3	6-16d	16-16d	10-16d	15.81	41.53	26.45	42.37	41.10	46.60	_	_
	WPI320-2	1	12	51/8	20	2½	25/16	3-16d	_	2-10dx1½		4430	3855	5950	5430	5980	_	_
		ľ		0,0		-/-	2710	0.00		2 100/172	-	19.71	17.15	26.47	24.15	26.60	_	_
	B5.12/22	✓	12	51/8	22	2½	2½	6-16d	8-16d	6-16d	1650 7.34	5940 26.42	3910 17.39	6490 28.87	5230	6185	_	_
											3555	9335	5945	9525	9240	10475		_
5 x 22	HB5.12/22	√	10	51/8	22	3½	3	6-16d	16-16d	10-16d	15.81	41.53	26.45	42.37	41.10	46.60	_	_
	WDIOOO O	/	10	F1/	00	01/	05/	0.404		0.40441/	_	4430	3855	5950	5430	5980	_	_
	WPI322-2	✓	12	51/8	22	2½	25/16	3-16d		2-10dx1½	_	19.71	17.15	26.47	24.15	26.60	_	_
	B5.12/24	✓	12	51/8	24	2½	2½	6-16d	8-16d	6-16d	1650	5940	3910	6490	5230	6185	_	_
5 x 24	B0.12/24	Ľ	12	078		L /2	L /2	0 100	0 100	0 100	7.34	26.42	17.39	28.87	23.27	27.51	_	_
	HB5.12/24	1	10	51/8	24	3½	3	6-16d	16-16d	10-16d	3555	9335	5945	9525	9240	10475		_
											15.81 3555	41.53 9335	26.45 5945	42.37 9525	41.10 9240	46.60 10475	_	_
	HB5.12/26	✓	10	51/8	26	3½	3	6-16d	8-16d	10-16d	15.81	41.53	26.45	42.37	41.10	46.60		_
5 x 26	IMDIOOC O		40	F1/	00	01/	05/	0.40.1		0.40-1-41/	_	4430	3855	5950	5430	5980	_	_
	WPI326-2	✓	12	51/8	26	2½	25/16	3-16d	_	2-10dx1½	_	19.71	17.15	26.47	24.15	26.60	_	_
	B5.12/28	1	12	51/8	28	2½	2½	6-16d	8-16d	6-16d	1650	5940	3910	6490	5230	6185	_	_
5 x 28	D3.12/20	Ľ	12	378	20	2/2	2/2	0 100	0 100	0 100	7.34	26.42	17.39	28.87	23.27	27.51	_	_
	HB5.12/28	1	10	51/8	28	3½	3	6-16d	16-16d	10-16d	3555	9335	5945	9525	9240	10475		_
											15.81 1650	41.53 5940	26.45 3910	42.37 6490	41.10 5230	46.60 6185	_	_
	B5.12/30	✓	12	51//8	30	2½	2½	6-16d	8-16d	6-16d	7.34	26.42	17.39	28.87	23.27	27.51		_
5 x 30											3555	9335	5945	9525	9240	10475	_	_
	HB5.12/30	✓	10	51/8	30	3½	3	6-16d	16-16d	10-16d	15.81	41.53	26.45	42.37	41.10	46.60	_	_
51/4 x 71/4	WPU5.50/7.25	1	12	5½	71/4	3	25/16	3-16d	4-16d	6-10d	1665	6390	6390	6825	7085	5980	_	_
J/4 X I /4	WI 00.30/1.23	•	12	372	1 /4	J	2716	J-10u	4-10u	0-100	7.41	28.43		30.36			_	_
	HB5.50/9.25	✓	10	5½	91/4	3½	3	6-16d	16-16d	10-16d	3555	9335	5945	9525	9240	10475	_	_
											15.81 1775	41.53 8250	26.45	8250		46.60		_
51/4 x 91/4	HWU5.50/9.25	✓	10	5½	91/4	31/4	2½	4-16d	4-16d	6-10d	7.90	36.70		36.70				_
	01 7) /5 50 /0 05		_	F 0./	04/	_	07/	4.40.1	0.401	0.40.1	2145	10455		10890			_	_
	GLTV5.50/9.25	✓	7	5%16	91/4	5	2%	4-16d	6-16d	6-16d	9.54	46.51		48.44			_	_
	HB5.50/9.5	✓	10	5½	9½	3½	3	6-16d	16-16d	10-16d	3555	9335		9525			_	_
	1100.00/0.0	Ľ	10	372	372	072		0 100	10 100	10 100	15.81			42.37			_	_
	WP5.50/9.5	✓	12	5½	9½	2½	25/16	3-16d	-	2-10d		4430		5950				_
											— 1775	19.71 8250		26.47 8250			_	_
	HWU5.50/9.5	✓	10	5½	9½	31/4	2½	4-16d	4-16d	6-10d	7.90	36.70		36.70				_
F1/ 01/	0171/5 50		_	F 0./	04/	_	07/	4.40.1	0.401	0.40.1	2145	10455		10890			_	_
51/4 x 91/2	GLTV5.59	✓	7	5%16	9½	5	27/8	4-16d	6-16d	6-16d	9.54	46.51	33.23	48.44	47.80	38.21	_	_
	HGLTV5.59	✓	7	5%16	9½	6	27/8	6-16d	12-16d	6-16d	2145	13070		15365			_	_
	1102110.00	Ľ	<u> </u>	0710	072	_		0 100	12 100	0 100	9.54	58.14		68.35			_	_
	SCL5.37/9.5	✓	3	5%	91/2	4	2¾	_	6-16d	6-16d	2155	13245		15850			_	_
											9.59 —	58.92	30.14	70.51	70.53	_	_	6060
	WM5.50/9.5	✓	12	5½	9½	2½	3¾	2-16d DPLX	_	2-10d	_	_		_	_			26.96
	UD5 50/4: 25	,	4.5			011		0.40	40 : 5 :	40.10.	3555	9335	5945			10475	_	
	HB5.50/11.25	√	10	5½	1111/4	3½	3	6-16d	16-16d	10-16d	15.81			42.37			_	_
51/4 x 111/4	HWU5.50/11.25	1	10	5½	111/4	31/4	2½	4-16d	4-16d	6-10d	1775	8250		8250			_	_
J/4 / 11/4	1177 00.00/11.20	ļ ,	10	0/2	1174	0 /4	-/-	7 100	- 10u	0 100	7.90	36.70		36.70			_	_
	GLTV5.50/11.25	✓	7	5%16	1111/4	5	27/8	4-16d	6-16d	6-10d	2145	10455		10890			_	_
		4/1		1				10		TO	9.54	46.51	33.23	48.44	47.80	38.21	_	_

Engineered Wood & Structural Composite Lumber Connectors

Catalogue C-C-CAN2015 © 2015 SIMPSON STRONG-TIE COMPANY INC.

TOP FLANGE HANGERS – I-JOISTS & SCL



					Dimer	sions							Fac	tored R	lesista	nce		
1-1-4	80-4-1	Web			(i)				Fastener	S	Uplift					= 1.00))	
Joist Size	Model No.	Stiff	Ga					Head	er		(KD=1.15)	D.Fir-L	S-P-F	LVL	PSL	LSL	I-Joist	Masonry
0120	No.	Reqd		W	Н	В	TF	Тор	Face	Joist	lbs kN	lbs kN	lbs kN	lbs kN	lbs kN	lbs kN	lbs kN	lbs kN
	LIDE 50/11 00	√	10	E1/	447/	01/	2	C 1C4	10 104	10.164	3555	9335	5945	9525	9240	10475		_
	HB5.50/11.88	•	10	5½	11%	3½	3	6-16d	16-16d	10-16d	15.81	41.53	26.45	42.37	41.10	46.60	_	_
	WP5.50/11.88	✓	12	5½	11%	2½	25/16	3-16d	_	2-10d		4430 19.71	3855 17.15	5950 26.47	5430 24.15	5980 26.60	_	_
	LIMILE 50/11 00	✓	10	E1/	447/	31/4	01/	4.104	4 104	6 104	1775	8250	8250	8250	8250	8250	_	_
	HWU5.50/11.88	V	10	5½	11%	374	2½	4-16d	4-16d	6-10d	7.90	36.70	36.70	36.70	36.70	36.70	_	_
5¼ x 11%	GLTV5.511	✓	7	5%16	11%	5	21//8	4-16d	6-16d	6-16d	2145 9.54	10455 46.51	7470 33.23	10890	10745 47.80	8590 38.21	_	_
	HGLTV5.511	√	7	5%16	11%	6	21/8	6-16d	12-16d	6-16d	2145	13070	9830	15365		13795	_	_
	TIGET VS.STI	V		J716	1178	0	2 78	0-100	12-10u	0-10u	9.54	58.14		68.35	50.38	61.37	_	_
	SCL5.37/11.88	✓	3	5%	11%	5	2¾	_	12-16d	12-16d	3255 14.48	17635 78.45	51.11	21600 96.09	20915 93.04	_	_	_
	WM5.50/11.88	√	12	5½	11%	2½	3¾	2-16d DPLX	_	2-10d		_	_	_	_	_	_	6060
	WW0.30/11.00	ľ	12	072	1170	L //2	074	Z TOU DI EX		2 10u	— 2555	9335		9525	9240	— 10475	_	26.96
	HB5.50/14	✓	10	5½	14	3½	3	6-16d	16-16d	10-16d	3555 15.81	41.53	26.45	42.37	41.10	46.60	_	_
	HWU5.50/14	√	10	5½	14	31/4	2½	4-16d	4-16d	6-10d	1775	8250	8250	8250	8250	8250	_	_
	111100.00711	ļ,		072		0/4		1 100	1 100	0 100	7.90 2145	36.70 10455	36.70 7470	36.70 10890	36.70 10745	36.70 8590	_	_
51/4 x 14	GLTV5.514	✓	7	5%16	14	5	21/8	4-16d	6-16d	6-16d	9.54	46.51	33.23	48.44	47.80	38.21	_	_
	HGLTV5.514	/	7	5%16	14	6	21/8	6-16d	12-16d	6-16d	2145	13070	9830	15365		13795	_	_
				07.10				0 .00	.2 .00	0.00	9.54 3255	58.14 17635		68.35 21600		61.37	_	_
	SCL5.37/14	✓	3	5%	14	5	23/4	_	12-16d	12-16d	14.48	78.45	51.11		93.04	_	_	_
	GLTV5.516	✓	7	5%16	16	5	21/8	4-16d	6-16d	6-16d	2145	10455	7470	10890			_	_
			-								9.54 2145	46.51 13070	33.23 9830	48.44 15365	47.80	38.21 13795		_
5¼ x 16	HGLTV5.516	V	7	5%16	16	6	21/8	6-16d	12-16d	6-16d	9.54	58.14	43.73		50.38	61.37	_	_
	SCL5.37/16	1	3/8	5%	16	6	25/8	_	10-16d	12-16d	4305	23730		29000			_	_
											19.15 3555	105.56 9335	57.94	129.00 9525	121.66 9240	10475	_	
	HB5.50/18	√	10	5½	18	3½	3	6-16d	16-16d	10-16d	15.81	41.53	26.45	42.37	41.10	46.60	_	_
	HWU5.50/18	1	10	5½	18	31/4	2½	4-16d	4-16d	6-10d	1775	8250	8250	8250	8250	8250		_
			_								7.90 2145	36.70 10455	36.70 7470	36.70 10890	36.70 10745	36.70 8590		_
5¼ x 18	GLTV5.518	√	7	5%16	18	5	21/8	4-16d	6-16d	6-16d	9.54	46.51	33.23	48.44	47.80	38.21	_	_
	HGLTV5.518	✓	7	5%16	18	6	21/8	6-16d	12-16d	6-16d	2145 9.54	13070 58.14		15365 68.35			_	_
	001 5 07/40		2.4	F2./	40	_	05/		10.10.1	40.40.1	4305	23730						
	SCL5.37/18	√	3/8	5%	18	6	25/8	_	10-16d	12-16d	19.15	105.56					_	_
	GLTV5.50/18.75	✓	7	5½	18¾	5	21/8	4-16d	6-16d	6-16d	2145 9.54	10455 46.51		10890 48.44			_	_
E1/ v 103/	HGLTV5.50/18.75	✓	7	F1/	103/	c	07/	C 1C4	10.164	6 164	2145	13070		15365			_	_
374 X 1074	HGL1 V3.50/16.75	V	7	5½	18¾	6	2%	6-16d	12-16d	6-16d	9.54	58.14		68.35			_	_
	SCL5.37/18.75	✓	3/8	5%	18¾	6	2%	_	10-16d	12-16d	4305 19.15	23730 105.56					_	_
	HB5.50/20	√	10	5½	20	3½	3	6-16d	16-16d	10-16d	3555	9335		9525		10475	_	_
	1103.30/20	V	10	J72	20	372	3	0-100	10-100	10-100	15.81	41.53				46.60	_	_
	HW5.50/20	✓	11	5½	20	2½	2½	4-16d	_	2-10d		6900 30.69	5285 23.51		5810 25.85		_	_
5¼ x 20	HWU5.50/20	√	10	5½	20	31/4	2½	4-16d	4-16d	6-10d	1490	8250	8250	8250	8250	8250		_
074 A ZU		•	10	372		J /4	-/2	7 100	. 100	5 10u	6.63 2145	36.70 10455		36.70 10890			_	_
	GLTV5.520	✓	7	5%16	20	5	27/8	4-16d	6-16d	6-16d	9.54	46.51					_	_
	HGLTV5.520	✓	7	5%16	20	6	21/8	6-16d	12-16d	6-16d	2145	13070	9830	15365	11325	13795	_	_
	,							- 700			9.54 1775	58.14 8250	43.73 8250	68.35 8250				_
7 x 71/4	HWU7.12/7.25	√	10	71/8	71/4	31/4	21/2	4-16d	4-16d	6-10d	7.90			36.70				_

See pages 118-125 for specific notes in individual model types. TRICONTRUSS.CA



					Dimer	ısions n)	3		Fastener	s	Uplift		Fac	tored F		nce = 1.00)	\	
Joist	Model	Web Stiff	Ga		("	''',		Head	er		ј орин (K _D =1.15)	D Fir-I	S-P-F	LVL	PSL	LSL		Masonry
Size	No.	Regd	au	w	н	В	TF	Houu	01	Joist	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs
						_		Тор	Face	00.01	kN	kN	kN	kN	kN	kN	kN	kN
		,									3555	9335	5945	9525	9240	10475	_	_
	HB7.12/9.25	✓	10	71/8	91/4	3½	3	6-16d	16-16d	10-16d	15.81	41.53	26.45	42.37	41.10	46.60	_	_
	1MD140.05.0	/	40	71/	01/	01/	05/	0.404		0.40441/	_	4430	3855	5950	5430	5980	_	_
7 01/	WPI49.25-2	✓	12	71/8	91/4	2½	25/16	3-16d	_	2-10dx1½	_	19.71	17.15	26.47	24.15	26.60	_	_
7 x 91⁄4	LIM/LIZ 10 /0 05	/	10	71/	01/	01/	01/	4 104	4 104	C 104	1775	8250	8250	8250	8250	8250	_	_
	HWU7.12/9.25	✓	10	71/8	91/4	31/4	2½	4-16d	4-16d	6-10d	7.90	36.70	36.70	36.70	36.70	36.70	_	_
	GLTV49.25-2	✓	7	71/8	91/4	5	27/8	4-16d	6-16d	6-16d	2145	10455	7470	10890	10745	8590	_	_
	GL1 V49.25-2	•	'	1 78	374	J	278	4-10u	0-10u	0-10u	9.54	46.51	33.23	48.44	47.80	38.21	_	_
	B7.12/9.5	1	12	71/8	91/2	2½	2½	6-16d	8-16d	6-16d	1650	5940	3910	6490	5230	6185	_	_
	B7.12, 0.0	Ľ		170	072		-/-	0 100	0 100		7.34	26.42	17.39	28.87	23.27	27.51	_	_
	HB7.12/9.5	/	10	71/8	91/2	3½	3	6-16d	16-16d	10-16d	3555	9335	5945	9525	9240			_
				.,,				0.00			15.81	41.53	26.45	42.37	41.10	46.60	_	_
	WPI49.5-2	1	12	71/8	9½	2½	25/16	3-16d	_	2-10dx1½		4430	3855	5950	5430	5980		_
											4775	19.71	17.15	26.47	24.15	26.60	_	_
7 x 9½	HWU7.12/9.5	✓	10	71/8	9½	31/4	2½	4-16d	4-16d	6-10d	1775	8250	8250	8250	8250	8250		_
											7.90	36.70	36.70 7470	36.70	36.70 10745	36.70	_	_
	GLTV49.5-2	✓	7	71/8	91/2	5	27/8	4-16d	6-16d	6-16d	2145 9.54	10455 46.51	33.23	10890 48.44		8590 38.21		
											2155	13245	6775	15845				
	SCL7.25/9.5	✓	3	71/4	9½	4	23/4	_	6-16d	6-16d	9.59	58.92	30.14	70.48	70.53			
											J.55	- 00.02	- 00.14	70.40	70.50	_	_	6060
	WMI49.5-2	✓	12	71/8	9½	2½	3¾	2-16d DPLX	_	2-10d	_			_	_	_		26.96
											3555	9335	5945	9525	9240	10475		_
	HB7.12/11.25	✓	10	71/8	1111/4	3½	3	6-16d	16-16d	10-16d	15.81	41.53	26.45	42.37	41.10	46.60	_	_
							251				_	4430	3855	5950	5430	5980	_	_
	WPI411.25-2	✓	12	71/8	1111/4	2½	25/16	3-16d	_	2-10dx1½	_	19.71	17.15	26.47	24.15		_	_
	111111710/11 05	/	40	71/	441/	01/	01/	1 101	4 404	0.404	1775	8250	8250	8250	8250	8250	_	_
7 v 441/	HWU7.12/11.25	✓	10	71/8	1111/4	31/4	2½	4-16d	4-16d	6-10d	7.90	36.70	36.70	36.70	36.70	36.70	_	_
7 x 11¼	CLTV411 25 2	1	7	71/8	111/4	5	27/8	4-16d	6-16d	6-16d	2145	10455	7470	10890	10745	8590	_	_
	GLTV411.25-2	V	'	1 78	1174	5	278	4-10u	0-10u	0-10u	9.54	46.51	33.23	48.44	47.80	38.21	_	_
	HGLTV411.25-2	1	7	71/8	1111/4	6	27/8	6-16d	12-16d	6-16d	2145	13070	9830	15365	11325	13795	_	_
	110L1 V411.23-2			1 /8	11/4		2/8	0-10u	12-10u	0-10u	9.54	58.14	43.73	68.35	50.38	61.37	_	_
	WMI411.25-2	1	12	71/8	111/4	2½	3¾	2-16d DPLX	_	2-10d		_	_	_	_	_		6060
	WWW.TTT.20 2	ľ		170	1174		074	E TOU DI EX		2 100		_	_	_	_	_	_	26.96
	B7.12/11.88	1	12	71/8	11%	2½	2½	6-16d	8-16d	6-16d	1650	5940	3910	6490	5230	6185		_
		ļ ·									7.34	26.42	17.39	28.87	23.27	27.51		_
	HB7.12/11.88	✓	10	71/8	11%	3½	3	6-16d	16-16d	10-16d	3555	9335	5945	9525	9240			_
											15.81	41.53						_
	WPI411.88-2	✓	12	71/8	11%	2½	25/16	3-16d	_	2-10dx1½		4430	3855	5950	5430			
											1775	19.71 8250	17.15 8250	26.47 8250	24.15 8250		_	_
	HWU7.12/11.88	✓	10	71/8	11%	31/4	21/2	4-16d	4-16d	6-10d	7.90	36.70		36.70				
7 x 11%											2145	10455	7470	10890			_	_
	GLTV411.88-2	✓	7	71/8	11%	5	27/8	4-16d	6-16d	6-16d	9.54	46.51		48.44				_
											2145	13070		15365			_	_
	HGLTV411.88-2	✓	7	71/8	11%	6	27/8	6-16d	12-16d	6-16d	9.54	58.14		68.35			_	_
	0017054400			747	447/		20.6		10.10.1	40.40.1	3255			21600			_	_
	SCL7.25/11.88	✓	3	71/4	11%	5	2¾	_	12-16d	12-16d	14.48	78.45	51.11	96.09	93.04	_	_	_
	WWW.444 00 0	/	40	71/	447/	01/	02/	0 404 DDI V		0.404	_	_	_	_	_	_	_	6060
	WMI411.88-2	✓	12	71/8	11%	2½	3¾	2-16d DPLX	_	2-10d	_	_	_	_	_	_	_	26.96
	B7.12/14	/	10	71/8	1/	21/	21/	6 164	0 164	E 164	1650	5940	3910	6490	5230	6185	_	_
	D7.12/14	V	12	1 78	14	2½	2½	6-16d	8-16d	6-16d	7.34	26.42	17.39	28.87	23.27	27.51	_	_
	HB7.12/14	/	10	71/8	14	3½	3	6-16d	16-16d	10-16d	3555	9335	5945	9525			_	_
	1101.12/19	Y	10	1 /8	14	J /2	J	U-10U	10-10u	10-100	15.81	41.53		42.37	41.10		_	_
7 x 14	WPI414-2	1	12	71/8	14	2½	25/16	3-16d	_	2-10dx1½		4430	3855	5950				_
/ A 17	*** 1717 4	•	12	1/8	'-	- /2	£/16	0 10u		L TOUNT /2	_	19.71		26.47	24.15		_	_
	HWU7.12/14	1	10	71/8	14	31/4	2½	4-16d	4-16d	6-10d	1775	8250	8250	8250				_
		ļ •		. , 0		J,4	-/-	. 100	50	J 100	7.90	36.70	36.70		36.70		_	_
	GLTV414-2	1	7	71/8	14	5	21/8	4-16d	6-16d	6-16d	2145	10455		10890				_
		1 *					1 0				9.54	46.51	133 23	48.44	4780	38 21	_	_

Engineered Wood & Structural Composite Lumber Connectors

TOP FLANGE HANGERS – I-JOISTS & SCL

SIMPSON Strong-Tie

					D:								Eoo	tored D	Popieto	200		(8)
		Web				ısions n)	i		Fastener	S	Uplift		гас	tored R Norn		= 1.00)	
Joist Size	Model No.	Stiff	Ga					Head	er		(KD=1.15)	D.Fir-L	S-P-F	LVL	PSL	LSL		Masonry
0120	No.	Reqd		W	Н	В	TF	Тор	Face	Joist	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs
								100			kN 014E	kN 13070	kN 9830	kN 15065	kN 11325	kN 13795	kN	kN —
	HGLTV414-2	✓	7	71/8	14	6	21/8	6-16d	12-16d	6-16d	2145 9.54	58.14	43.73			61.37		_
7 x 14	SCL7.25/14	√	3/8	71/4	14	5	2¾	_	12-16d	12-16d	3255	17635		21600		—	_	_
(cont.)	30L7.23/14	•	78	1 74	14	J	2 74		12-100	12-10u	14.48	78.45	51.11	96.09	93.04	_	_	_
	WMI414-2	✓	12	71/8	14	21/2	3¾	2-16d DPLX	_	2-10d			_	_		_	_	6060 26.96
	D740/40	1	10	71/	10	01/	01/	C 4C4	0.404	0.404	1650	5940	3910	6490	5230	6185	_	_
	B7.12/16	√	12	71/8	16	2½	2½	6-16d	8-16d	6-16d	7.34	26.42	17.39	28.87	23.27	27.51	_	_
	HB7.12/16	✓	10	71/8	16	3½	3	6-16d	16-16d	10-16d	3555 15.81	9335 41.53	5945 26.45	9525	9240	10475		_
	WD1440.0		40	747	40	04/	05/	0.401		0.401.447		4430	3855	5950	5430	5980		_
	WPI416-2	√	12	71//8	16	2½	25/16	3-16d	_	2-10dx1½	_	19.71	17.15	26.47	24.15	26.60	_	_
	HWU7.12/16	✓	10	71/8	16	31/4	21/2	4-16d	4-16d	6-10d	1775 7.90	8250	8250	8250 36.70	8250 36.70	8250		_
7 x 16			_			_					2145	36.70 10455	36.70 7470	10890		36.70 8590	_	_
	GLTV416-2	✓	7	71//8	16	5	21/8	4-16d	6-16d	6-16d	9.54	46.51	33.23	48.44	47.80	38.21	_	_
	HGLTV416-2	✓	7	71/8	16	6	21/8	6-16d	12-16d	6-16d	2145	13070				13795		_
											9.54 4305	58.14 23730		68.35 29000		61.37		_
	SCL7.25/16	√	3/8	71/4	16	5	23/4	_	10-16d	12-16d	19.15	105.56		129.00		_	_	_
	WMI416-2	√	12	71/8	16	2½	3¾	2-16d DPLX	_	2-10d		_	_	_	_	_		6060
							• • •				— 1650	— 5940	3910	6490	- 5230	6185		26.96
	B7.12/18	√	12	71/8	18	21/2	21/2	6-16d	8-16d	6-16d	7.34	26.42	17.39	28.87	23.27	27.51		_
	HB7.12/18	✓	10	71/8	18	3½	3	6-16d	16-16d	10-16d	3555	9335	5945	9525	9240	10475		_
	1107.12/10	•	10	1 /0	10	072	J	0 100	10 100	10 100	15.81	41.53		42.37	41.10	46.60	_	_
	HWI418-2	✓	11	71/8	18	21/2	21/2	4-16d	_	2-10d		6900 30.69	5285	7695 34.23	5810 25.85	6870 30.56	_	_
	HWU7.12/18	√	10	71/8	18	31/4	2½	4-16d	4-16d	6-10d	1775	8250	8250	8250	8250	8250	_	_
7 x 18	110007.12/10	•	10	1 78	10	374	2 72	4-10u	4-10u	0-10u	7.90	36.70	36.70	36.70	36.70	36.70	_	_
	GLTV418-2	✓	7	71/8	18	5	21/8	4-16d	6-16d	6-16d	2145 9.54	10455 46.51	7470 33.23	10890	47.80	8590 38.21	_	_
	LICITY/410 O	✓	7	71/	10	6	07/	C 1C4	10.104	6 164	2145	13070		15365		13795		_
	HGLTV418-2	V	1	71/8	18	6	21/8	6-16d	12-16d	6-16d	9.54	58.14		68.35		61.37	_	_
	SCL7.25/18	✓	3/8	71/4	18	6	25/8	_	10-16d	12-16d	4305 19.15	23730 105.56		29000 129.00		_	_	_
			40	71/	40	01/	027	0.40 DDI V		0.40.1	— —			-	-			6060
	WMI418-2	√	12	71/8	18	2½	3¾	2-16d DPLX	_	2-10d	_	-	_	_	_	_	_	26.96
	GLTV418.75-2	✓	7	71/8	18¾	5	21/8	4-16d	6-16d	6-16d	2145 9.54	10455 46.51	7470	10890		8590 38.21		_
7 x 18¾						_					4305	23730						_
	SCL7.25/18.75	√	3/8	71//8	18¾	6	2%	_	10-16d	12-16d	19.15	105.56	57.94	129.00	121.66	_	_	_
	B7.12/20	✓	12	71/8	20	21/2	21/2	6-16d	8-16d	6-16d	1650	5940		6490				_
											7.34 3555	26.42 9335		9525	9240	27.51 10475	_	_
	HB7.12/20	✓	10	71//8	20	3½	3	6-16d	16-16d	10-16d	15.81	41.53				46.60	_	_
	HWI420-2	✓	11	71/8	20	21/2	2½	4-16d	_	2-10d		6900		7695		6870		_
7 x 20											— 1775	30.69 8250		8250		8250		_
	HWU7.12/18	✓	10	71/8	20	31/4	2½	4-16d	4-16d	6-10d	7.90	36.70				36.70	_	_
	GLTV420-2	✓	7	71/8	20	5	21/8	4-16d	6-16d	6-16d	2145	10455						_
											9.54 2145	46.51 13070		48.44				_
	HGLTV420-2	√	7	71/8	20	6	2%	6-16d	12-16d	6-16d	9.54	58.14					_	_
	B7.12/22	✓	12	71/8	22	2½	2½	6-16d	8-16d	6-16d	1650	5940		6490		6185	_	_
											7.34 3555	26.42 9335		28.87 9525	9240	27.51 10475		_
	HB7.12/22	✓	10	71//8	22	3½	3	6-16d	16-16d	10-16d	15.81	41.53				46.60		_
7 x 22	HWI422-2	√	11	71//8	22	2½	2½	4-16d	_	4-10d		6900	5285	7695	5810	6870	_	_
	.,			. 70		-/2	-/-	. 100		. Tou	<u> </u>	30.69 10455						_
	GLTV422-2	✓	7	71/8	22	5	21/8	4-16d	6-16d	6-16d	9.54			48.44			_	_
	HGLTV7.12/22	✓	7	71/8	22	6	21/8	6-16d	12-16d	6-16d	2145	13070	9830	15365	11325	13795	_	_
	11UL1 V1.12/22	V	1	1 /8	22	U	2/8	0-10u	12-10u	0-10U	9.54	58.14	43.73	68.35	50.38	61.37	_	_

SIMPSON Strong-Tie

Engineered Wood & Structural Composite Lumber Connectors

					Dime	nsions	;		Fastener				Fac	tored F	lesista	nce		
Joist	Model	Web			(i	n)					Uplift					= 1.00		
Size	No.	Stiff	Ga			_		Head	ler		(K _D =1.15)			LVL	PSL	LSL		Masonry
		Reqd		W	Н	В	TF	Top	Face	Joist	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs
											kN	kN	kN	kN c400	kN	kN 6185	kN	kN
	B7.12/24	√	12	71/8	24	2½	21/2	6-16d	8-16d	6-16d	1650	5940	3910	6490	5230			
											7.34 3555	26.42 9335	17.39 5945	28.87 9525	23.27 9240	27.51	_	_
	HB7.12/24	✓	10	71/8	24	3½	3	6-16d	16-16d	10-16d	15.81	41.53	26.45	42.37	41.10	46.60		
											15.61	6900	5285	7695	5810	6870	_	_
7 x 24	HWI424-2	✓	11	71/8	24	2½	21/2	4-16d	_	4-10d		30.69	23.51	34.23	25.85	30.56		
											2145	10455	7470	10890	10745	8590		
	GLTV424-2	√	7	71/8	24	5	2%	4-16d	6-16d	6-16d	9.54	46.51	33.23	48.44	47.80	38.21		_
											2145	13070	9830	15365		13795		
	HGLTV7.12/24	✓	7	71//8	24	6	2%	6-16d	12-16d	6-16d	9.54	58.14	43.73	68.35	50.38	61.37	_	
											1650	5940	3910	6490	5230	6185		
	B7.12/26	✓	12	71/8	26	2½	2½	6-16d	8-16d	6-16d	7.34	26.42	17.39	28.87	23.27	27.51	_	_
											3555	9335	5945	9525	9240	10475		_
	HB7.12/26	✓	10	71/8	26	3½	3	6-16d	16-16d	10-16d	15.81	41.53	26.45	42.37	41.10	46.60	_	_
											_	6900	5285	7695	5810	6870	_	
7 x 26	HWI426-2	✓	11	71/8	26	2½	2½	4-16d	_	6-10d	_	30.69	23.51	34.23	25.85	30.56	_	_
	0171/400 0			71/	00		07/	4.40.1	0.404	0.40.1	2145	10455	7470	10890	10745	8590	_	_
	GLTV426-2	✓	7	71/8	26	5	27/8	4-16d	6-16d	6-16d	9.54	46.51	33.23	48.44	47.80	38.21	_	_
	HGLTV426-2	/	7	71/8	26	6	27/8	6-16d	12-16d	6-16d	2145	13070	9830	15365	11325	13795	_	
	HGL1 V420-2	V	′	1 78	20	0	278	0-10u	12-10u	0-10u	9.54	58.14	43.73	68.35	50.38	61.37	_	_
	B7.12/28	/	12	71/8	28	2½	2½	6-16d	8-16d	6-16d	1650	5940	3910	6490	5230	6185	_	_
	D7.12/20	_	12	1 /0		2/2	2/2	0 100	0 100	0 100	7.34	26.42	17.39	28.87	23.27	27.51	_	_
	HB7.12/28	/	10	71/8	28	3½	3	6-16d	16-16d	10-16d	3555	9335	5945	9525	9240	10475	_	_
				.,,		0,,	Ů	0 .00		.0 .00	15.81	41.53	26.45	42.37	41.10	46.60	_	_
7 x 28	HWI428-2	✓	11	71/8	28	2½	2½	4-16d	_	6-10d	_	6900	5285	7695	5810	6870		_
		ļ .										30.69	23.51	34.23	25.85	30.56	_	_
	GLTV428-2	✓	7	71/8	28	5	21/8	4-16d	6-16d	6-16d	2145	10455	7470	10890	10745	8590	_	_
											9.54	46.51	33.23	48.44	47.80	38.21	_	_
	HGLTV428-2	✓	7	71/8	28	6	21/8	6-16d	12-16d	6-16d	2145 9.54	13070 58.14	9830	15365 68.35	11325 50.38	13795		_
											9.54	6900	5285	7695	5810	6870		_
	HWI430-2	✓	11	71/8	30	2½	21/2	4-16d	_	6-10d		30.69	23.51	34.23	25.85	30.56		
											2145	10455	7470	10890	10745			
7 x 30	GLTV430-2	✓	7	71/8	30	5	21/8	4-16d	6-16d	6-16d	9.54	46.51	33.23	48.44	47.80	38.21	_	_
											2145	13070	9830		11325	13795	_	_
	HGLTV430-2	✓	7	71/8	30	6	21/8	6-16d	12-16d	6-16d	9.54	58.14	43.73	68.35	50.38	61.37	_	_
						211					_	6900	5285	7695	5810	6870	_	_
	HWI432-2	✓	11	71/8	32	2½	2½	4-16d	_	6-10d	_	30.69	23.51	34.23	25.85	30.56	_	_
7. 00	0171/400 0		_	71.	00	_	07/	4.40.1	0.40.1	0.40.1	2145	10455	7470	10890	10745	8590	_	_
7 x 32	GLTV432-2	✓	7	71/8	32	5	2%	4-16d	6-16d	6-16d	9.54	46.51	33.23	48.44	47.80	38.21	_	_
	HGLTV432-2	/	7	71/8	32	6	21/8	6-16d	12-16d	6-16d	2145	13070	9830	15365	11325	13795	_	_
	11GL1 V432-Z	v	′	1 7/8	٥2	0	Z'/8	U-10U	12-100	0-100	9.54	58.14	43.73	68.35	50.38	61.37	_	_

See pages 118-125 for specific notes on individual model types.

MATERIAL: THAI-2-14 gauge; all others-18 gauge

FINISH: Galvanized INSTALLATION:

Engineered Wood & Structural Composite Lumber Connectors

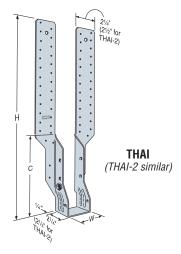
- THAI-2 must be factory-ordered for hanger width needed. See table for allowable widths.
- Use all specified fasteners. Verify that the header can take the required fasteners specified in the table.
- Web stiffeners are required for all I-joists used with these hangers.
- When a total of 20 face nails are used in THAI straps, or 30 face nails are used in THAI-2 straps, the maximum factored resistance is achieved.
- A minimum nailing configuration is shown for top nailing installations.
 The strap must be field-formed over the top of the header by a minimum of 2½".

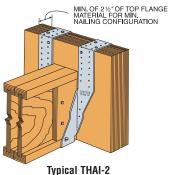
Joist Dime	nsions (in)	Model	Hanger Dimensions (in)					
Width	Depth	No.	W¹	Н	С			
1½	91/4 - 14"	THAI222	1%16	221/8	9%			
13/4	91/4 - 14"	THAI1.81/22	1 13/16	22¾	91/4			
2	91/4 - 14"	THAI2.06/22	21/16	225/8	91/8			
21/4 to 25/16	91/4 - 14"	THAI3522	25/16	22½	9			
2½	91/4 - 14"	THAI322	29/16	223/8	87/8			
3½	91⁄4 - 14"	THAI422	3%6	211//8	8%			
3 to 51/4	91/4 - 14"	THAI-2	31/8 to 55/16	2111/16	813/16			

1. The W dimension should be ordered at 1/16" to 1/8" greater than the joist width.

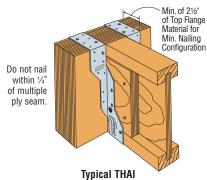
		Fasteners		Factored Resistance					
	Hea	ıder			D.Fir-L	S-P-F	LVL		
Nailing				Uplift (K _D = 1.15)	Normal	Normal	Normal		
Option	Ton	Face	Joist	()	$(K_D = 1.00)$	$(K_D = 1.00)$	$(K_D = 1.00)$		
	Тор	гасе		lbs	lbs	lbs	lbs		
				kN	kN	kN	kN		
	4-10dv114	2-10dx11/2	2-10dx11/2	_	2035	1735	2595		
THAI Minimum	4-10ux172	2-10ux 1 72	Z-10ux 1 72	1	9.05	7.72	11.54		
THAT WITHINGTO	4-10d	2-10d	2-10dx11/2	_	3000	2385	2810		
	4-10u	2-10u	2-10ux 1 72	-	13.35	10.61	12.50		
THAI Maximum		20-10d	2-10dx1½	410	3025	2150	3025		
THAI WAXIIIIUIII		20-10u	Z-10ux 1 72	1.82	13.46	9.56	13.46		
THAI-2 Minimum	4-10d	2-10d	2-10dx1½	_	2800	2800	2800		
TTTAT-Z IVIIIIIIIIIIIIII	4-10u	2-10u	2-10UX 1 72	_	12.46	12.46	12.46		
THAI-2 Maximum		30-10d	2-10dx1½	410	6090	4325	6090		
ITIAI-Z WAXIIIUIII		30-100	2-10UX 1 72	1.82	27.09	19.24	27.09		

- Uplift loads have been increased 15% for wind or earthquake loading with no further increase allowed; reduce where other loads govern.
- 2. The minimum header depth to achieve the maximum nail configuration is 16".
- 3. Applies to LVL headers made primarily from Douglas Fir or Southern Pine. For LVL made primarily from Spruce Pine Fir or similar less dense veneers, use the values found in the S-P-F column.
- Factored uplift resistances shown are for D.Fir-L. Multiply tablulated resistances x 0.71 for either SPF joist or header.
- 5. NAILS: 10d = 0.148° dia. x 3° long, 10dx11/2 = 0.148° dia. x 11/2° long. See pages 22-23 for other nail sizes and information

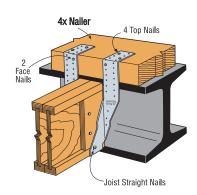




Installation



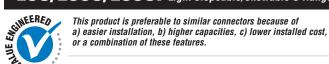
Typical THAI
Installation with Minimum
Nailing Configuration



Typical THAI Minimum Nailing Configuration on a 4x Nailer

LSU/LSSU/LSSUI Light Slopeable/Skewable U Hangers for I-Joists and SCL

Strong-Tie

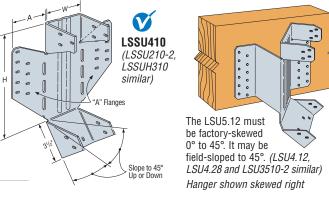


This series attach joists or rafters to headers, sloped up or down, and skewed left or right, up to 45°.

MATERIAL: See table FINISH: Galvanized

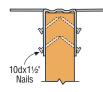
INSTALLATION: • Use all specified fasteners. See General Notes.

- Attach the sloped joist at both ends so that the horizontal force developed by the slope is fully supported by the supporting members.
- · Web stiffeners required for I-joist applications.
- To see an installation video on this product, visit www.strongtie.com.
- 10dx1½" nails cannot be substituted for specified face nails for skewed or sloped and skewed combinations.



LSU and LSSU INSTALLATION SEQUENCE

(For Skewed or Sloped/Skewed Applications)



Actual

Joist

Model

LSSUH310

LSSU210-2

LSSU410

LSU4.12

LSU4.28

LSU5.12

LSU3510-2

3

31/2

4

41/2

41/2 - 43/4

5

16

16

16

14

14

2%16

31/8

3%16

41/8

41/4

81/2

81/2

81/2

9

9

31/8

27/8

25/6

21/4

23/8

14-16d

14-16d

14-16d

24-16d

24-16d

24-16d

12-10dx11/2

12-10dx11/2

12-10dx11/2

16-10dx1½

16-10dx11/2

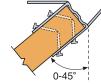
16-10dx11/

6-10dx1

Nail hanger to slope-cut carried member, installing seat nail first. No bevel necessary. Install joist nails at 45° angle.

Dimensions

(in)



Fasteners

STEP 2
Skew flange from 0-45°.
Bend other flange back along centerline of slots until it meets the header.
Bend one time only.

D.Fir-L

Normal

Uplift

1625

7.23 1625

7.23 162

7.23

1960

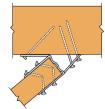
8.72

1960

8.72

1960

Factored Resistance



Normal

S-P-F

Uplift

1155

5.14 1155

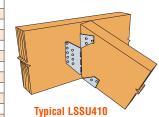
5.14 1155

5.14 1395

6.21

STEP 3
Attach hanger to the carrying member, acute angle side first. Install nails at an angle.

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.



Installation

	Width	No.	Ga	w			Hooder	Joist	$(K_D = 1.15)$	$(K_D=1.00)$	$(K_D=1.15)$	$(K_D = 1.00)$
	(in)			VV	Н	Α	Header	Juist	lbs	lbs	lbs	lbs
									kN	kN	kN	kN
	SLOPED ONLY HANGERS											
	41/	1.0011040	40	40/	01/	45/	10 101	7.40441/	1240	3090	1130	2325
7	1½	LSSU210	18	1%16	81/2	1%	10-10d	7-10dx1½	5.52	13.75	5.03	10.34
	13/4	LSSUI25	18	1 13/16	81/2	11/2	10-10d	7-10dx1½	1240	3090	1130	2325
	174	2000123	10	1 /16	072	172	10 100	7 100X172	5.52	13.75	5.03	10.34
	2	LSSUI2.06	18	21/16	81/2	13/4	10-10d	7-10dx1½	1240	3090	1130	2325
									5.52 1240	13.75 3090	5.03 1130	10.34 2325
	21/16	LSSU2.1	18	21/8	81/2	13/4	10-10d	7-10dx1½	5.52	13.75	5.03	10.34
									1240	3090	1130	2325
	21/4 - 25/16	LSSUI35	18	25/16	81/2	1%	10-10d	7-10dx1½	5.52	13.75	5.03	10.34
	01/	1.001111040	10	00/	01/	01/	10 104	10 10411/	1625	3675	1155	2780
	21/2	LSSUH310	16	2%16	81/2	31/8	18-16d	12-10dx1½	7.23	16.35	5.14	12.37
N.	3	LSSU210-2	16	31/8	81/2	27/8	18-16d	12-10dx1½	1625	3675	1155	2780
	J	L000210-2	10	J /8	0 /2	2/8	10-10u	12-10UX172	7.23	16.35	5.14	12.37
b	31/2	LSSU410	16	3%16	81/2	25/6	18-16d	12-10dx1½	1625	4520	1155	3210
		2000110		07.10	972	270	10 100	12 100/172	7.23	20.11	5.14	14.28
	4	LSU4.12	14	41/8	9	21/4	24-16d	16-10dx1½	1960 8.72	3810 16.95	1395 6.21	2705 12.03
		-							1960	3810	1395	2705
	41/8	LSU4.28	14	41/4	9	23/8	24-16d	16-10dx1½	8.72	16.95	6.21	12.03
	41/ 42/	1.0110540.0	4.4	427	07/	05/	04.40.1		1960	3810	1395	2705
	41/2 - 43/4	LSU3510-2	14	43/4	8%	3%	24-16d	16-10dx1½	8.72	16.95	6.21	12.03
	5	LSU5.12	14	51/8	9	21/4	24-16d	16-10dx1½	1285	4755	910	2770
	J	L303.12	14						5.72	21.15	4.05	12.32
				SKE	NED H	ANGER	S OR SLOP	PED AND SKEV				
Ь	1½	LSSU210	18	1%16	81/2	1%	9-10d	7-10dx1½	1240	2090	910	1485
	172	2000210	10	1710	072	170	0 100	7 100/172	5.52	9.30	4.05	6.61
	13/4	LSSUI25	18	1 13/16	81/2	11/2	9-10d	7-10dx1½	1240 5.52	2090	910	1485
									1240	9.30 2090	4.05 910	6.61 1485
	2	LSSUI2.06	18	21/16	81/2	13/4	9-10d	7-10dx1½	5.52	9.30	4.05	6.61
	01/	1.00110.4	40	01/	01/	427	0.40.1	7.40 1.41/	1240	2090	910	1485
	21/16	LSSU2.1	18	21/8	81/2	1¾	9-10d	7-10dx1½	5.52	9.30	4.05	6.61
	21/4 - 25/-	LSSUI35	18	25/16	81/2	1%	9-10d	7-10dx1½	1240	2090	910	1485
	Z/4 - Z716	1330133	10	∠ 716	0 72	176	9-10u	7-10UX 1 72	5.52	9.30	4.05	6.61
	1		1			1	1	1	4005	0045	4455	1 4005

- Factored uplift resistances include a 15% increase for earthquake or wind loading; no further increase is allowed; reduce where other loads govern.
- 2. LSU3510-2, LSU4.12, LSU4.28 and LSU5.12 skew option must be factory-ordered.
- 3. Minimum 11" joist height for LSU3510-2, LSU4.12, LSU4.28 and LSU5.12; 9½" for all others.
- 4. NAILS: 16d = 0.162" dia. x 3½" long, 10d = 0.148" dia. x 3" long, 10dx1½ = 0.148" dia. x 1½" long. See pages 22-23 for other nail sizes and information



SUR/SUL/HSUR/HSUL Skewed 45° Hangers for 1-Joist and SCL

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

The SUR/L1.81, 2.06, 2.1, 2.37, 2.56 and HSUR/L series are 45° skewed hangers designed specifically to ease the installation of single and double I-joists. In addition to Positive Angle Nailing these hangers encapsulate the top flange of the I-joist, so no web stiffeners are required for standard installation.

The full range of 45° skewed hangers feature obround nail holes on the acute side allowing nails to be easily installed parallel to the header and joist. Installation is further simplified with no required bevel cuts.

MATERIAL: See table

Engineered Wood & Structural Composite Lumber Connectors

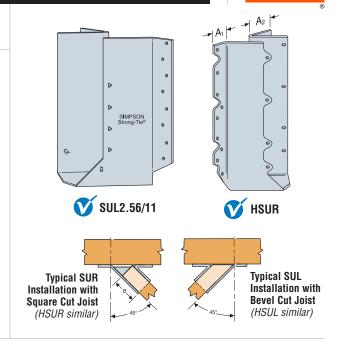
FINISH: Galvanized. Some products available in ZMAX® coating; see Corrosion Information, pages 14-17.

INSTALLATION: • Use all specified fasteners. See General Notes.

- · Illustrations show left and right skews SUR/L (SUR = skewed right; SUL = skewed left).
- The joist end may be square cut or bevel cut.
- Web stiffeners are required for I-joist applications for all hangers requiring more than two joist fasteners or where the hanger does not overlap the top flange of the joist.
- Fill all round and obround nail holes with specified fasteners to achieve table values. Where noted, triangle holes in the joist flange may be filled for additional uplift capacity (see Footnote 2).

OPTIONS: • These hangers will accommodate a 40° to 50° skew.

 Available with the A₂ flange turned in on 2-2x and 4x models only (see illustration). For example, specify HSURC410, HSULC410, SURC210-2, or SULC210-2.



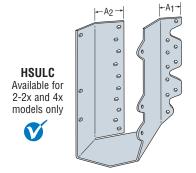
These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

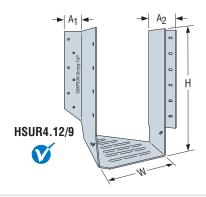
					Dimensions					Fasteners		Factored Resistance			
	Astus		Ga	(in)					ruotonoro		D.Fir-L		S-P-F		
	Actual Joist Size	Model							Header	Joist	Uplift	Normal	Uplift	Normal	
	(in)	No.	ua	W	н	В	A ₁	A ₂			$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$	
	(,			VV	"	"	Al	n 2			lbs	lbs	lbs	lbs	
											kN	kN	kN	kN	
	1½ x 9¼-9½	SUR/L210	16	1%	81/8	2	11/8	15/16	10-16d	10-10dx1½	2085	3820	1480	2710	
	172 X 374-372	30H/LZ10	10	I 716	078		178	I 716	10-100	10-10ux 1 72	9.27	16.99	6.58	12.05	
	1½ x 11¼-16	SUR/L214	16	1%6	10	2	11/8	15/16	12-16d	12-10dx1½	2690	4585	2175	3255	
	172 X 1174 10	0011/2214	10	1710	10		178	1710	12 100	12 100X172	11.97	20.40	9.67	14.48	
	1¾ x 9¼-9½	SUR/L1.81/9	16	1 13/16	9	3	1%	25/16	12-16d	2-10dx1½	275	3140	195	2220	
ļ	1717.071.072	0011/21101/0		. , , ,			170	27.0			1.22	13.97	0.87	9.88	
	1¾ x 11¼-11%	SUR/L1.81/11	16	1 13/16	11	3	1%	25/16	16-16d	2-10dx1½	275	3140	195	2220	
ļ	.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	0011/21101/11		. , , ,			.,,			2 100X172	1.22	13.97	0.87	9.88	
	1¾ x 14-16	SUR/L1.81/14	16	1 13/16	13¾	3	1%	25/16	20-16d	2-10dx1½	275	3140	195	2220	
-											1.22	13.97	0.87	9.88	
	2 x 9½	SUR/L2.06/9	16	21/16	91/16	33/16	1%	21/8	14-16d	2-10dx1½	385	3950	385	2805	
-											1.71	17.57	1.71	12.48	
	2 x 11%	SUR/L2.06/11	16	21/16	111/4	3¾16	1%	21/8	16-16d	2-10dx1½	385 1.71	3950 17.57	385 1.71	2805 12.48	
ŀ											385	3950	385	2805	
	2 x 14-16	SUR/L2.06/14	16	21/16	13%	3¾16	1%	21/8	18-16d	2-10dx1½	1.71	17.57	1.71	12.48	
ł											385	3950	385	2805	
	21/16 x 91/2	SUR/L2.1/9	16	21/8	91/16	3¾16	1%16	21/8	14-16d	2-10dx1½	1.71	17.57	1.71	12.48	
ł											385	3950	385	2805	
ł	21/16 x 111//8	SUR/L2.1/11	16	21/8	113/16	3¾16	1%16	21/8	16-16d	2-10dx1½	1.71	17.57	1.71	12.48	
ľ	211	0.15 // 0.1/14			1001	221		2			385	3950	385	2805	
	21/16 x 14-16	SUR/L2.1/14	16	21/8	13%16	3¾16	1%16	21/8	18-16d	2-10dx1½	1.71	17.57	1.71	12.48	
ľ	01/ 05/01/	0110 // 0 07/0	40	02/	015/	02/	45/	01/	44404	0.40-1-41/	385	3950	385	2805	
ı	21/4-25/16 x 91/2	SUR/L2.37/9	16	23/8	815/16	3¾16	1 ½16	21/8	14-16d	2-10dx1½	1.71	17.57	1.71	12.48	
Ī	21/4-25/16 x 117/8	SUR/L2.37/11	16	2¾	113/16	3¾16	1 5⁄16	21/8	16-16d	2-10dx1½	385	3950	385	2805	
	Z74-Z716 X II78	3UN/L2.31/11	10	2 98	11716	3716	1 716	Z 78	10-10u	Z-100X172	1.71	17.57	1.71	12.48	
	21/4-25/16 x 14-16	SUR/L2.37/14	16	23/8	137/16	33/16	15/16	21/8	18-16d	2-10dx1½	385	3950	385	2805	
Ţ	∠/4-∠716 X 14-10	JUN/LZ.J//14	10	2 78	13716	3 %16	I 7/16	∠ 78	10-100	Z-1UUX 1 1/2	1.71	17.57	1.71	12.48	
	2½ x 9¼-9½	SUR/L2.56/9	16	2%16	813/16	3¾16	11/8	21/8	14-16d 2-10dx	2-10dx1½	385	3950	385	2805	
ļ	E72 X 074 072	0011/12.00/0	10	L / 10	0 / 10	0710	170	.,. E,, 11100	L TOUNT/2	1.71	17.57	1.71	12.48		
	2½-2% x 11¼-11%	SUR/L2.56/11	16	2%16	113/16	33/16	11/8	21/8	16-16d 2-10dx1½	385	3950	385	2805		
ļ		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1			,.5	0 7 .0	.,,			10 10u 2 10ux172	1.71	17.57	1.71	12.48	
	2½ x 14-16	SUR/L2.56/14	16	2%16	135/16	33/16	11/8	21/8	18-16d	2-10dx1½	385	3950	385	2805	
L		0011/12.00/14		2710	10710	0710	170		10 100	_ 100/172	1.71	17.57	1.71	12.48	

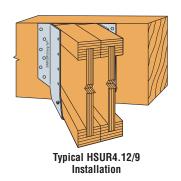
- 1. Factored uplift resistances have been increased by 15% for earthquake or wind loading with no further increase allowed; reduce for other load durations as required by code.
- 2. Triangle holes may be filled (requires web stiffeners) with 10dx1½" nails for additional uplift.
 - SUR/SUL 9 and 11 inch and all HSUR/HSUL models have four additional holes. The SUR/SUL 14 inch models have an additional six notes. The factored up lift resistance 8 kN) D Fir-L and 965 lbs (4.29 kN) S-P-F.
- SUR/SUL 14 inch models have an 3. **NAILS**: 16d = 0.162" dia. x 3 ½" long, 795 lbs (7.98 kV) D.Fir-L ard 1385 lbs (6.16 kV) S-P of or other nail sizes and information.

SUR/SUL/HSUR/HSUL Skewed 45° Hangers for I-Joist and SCL









These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

				D	imensio	ns		Fas	teners		Factored F	Resistance	
Antural					(in)			1 00			ir-L		P-F
Actual Joist Size	Model	Ga								Uplift	Normal	Uplift	Normal
(in)	No.	uu	w	н	В	A ₁	A ₂	Header	Joist	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
` ,			vv	"	В	A1	H2	IICauci	30131	lbs	lbs	lbs	lbs
										kN	kN	kN	kN
	SUR/L210-2	16	31/8	811/16	25/6	17/	23/8	14-16d	6-16dx2½	1695	4065	1540	2875
3 x 91/4-14	3UN/L210-2	10	378	O . 716	Z 76	17/16	Z 78	14-10u	0-10UXZ 72	7.54	18.08	6.85	12.81
3 X 374-14	HSUR/L210-2	14	31/8	811/16	27/16	11/4	27/16	20-16d	6-16dx2½	1840	5270	1540	3745
	110011/1210 2	17	078	0 /16	2/16	174	2/10	20 100	0 100X272	8.18	23.44	6.85	16.66
	SUR/L214-2	16	31/8	1211/16	2½	17/16	21/8	18-16d	8-16dx2½	2265	4095	2090	2895
3 x 14-20	0011,22112					.,,,,		10 100	0 100/12/2	10.08	18.22	9.30	12.90
	HSUR/L214-2	14	31/8	1211/16	27/16	11/4	23/16	26-16d	8-16dx2½	2455	6875	2095	4880
										10.92	30.58	9.32	21.71
	SUR/L410	16	3%16	81/2	25/6	1	23/8	14-16d	6-16d	1695 7.54	4065 18.08	1540 6.85	2875 12.81
3½ x 9¼-14										1840	5270	1540	3745
	HSUR/L410	14	3%16	8½	27/16	1	23/16	20-16d	6-16d	8.18	23.44	6.85	16.66
										2265	4095	2090	2895
	SUR/L414	16	3%16	12½	25/8	1	2%	18-16d	8-16d	10.08	18.22	9.30	12.90
3½ x 14-20	110110 // 444	4.4	00/	401/	07/		02/	00.404	0.40.1	2455	6875	2095	4880
	HSUR/L414	14	3%16	12½	27/16	1	23/16	26-16d	8-16d	10.92	30.58	9.32	21.71
4 x 9½	HSUR/L4.12/9	14	41/8	9	3	1 ½16	2%	12-16d	2-10dx1½	275	2995	195	2350
4 X 972	13Uh/L4.12/9	14	4 78	9	3	I 716	Z98	12-10u	Z-100X172	1.22	13.34	0.87	10.47
4 x 11%	HSUR/L4.12/11	14	41/8	1111/8	3	17/16	2%	16-16d	2-10dx1½	275	4190	195	2965
4 X 1170	110011/124:12/11		770	1176	0	1710	270	16-160	2 100X172	1.22	18.64	0.87	13.21
4 x 14	HSUR/L4.12/14	14	41/8	13¾	3	17/16	23/8	20-16d	2-10dx1½	275	4190	195	2965
										1.22	18.64	0.87	13.21
4 x 16	HSUR/L4.12/16	14	41/8	15¾	3	1 ½16	23/8	24-16d	2-10dx1½	275	4190	195	2965
										1.22 275	18.64 2995	0.87 195	13.21 2350
41/8 x 91/2	HSUR/L4.28/9	14	45/16	9	23/4	1 ½16	23/8	12-16d	2-10dx1½	1.22	13.34	0.87	10.47
										275	4190	195	2965
41/8 x 111/8-16	HSUR/L4.28/11	14	45/16	111/8	23/4	1 ½16	23/8	16-16d	2-10dx1½	1.22	18.64	0.87	13.21
										275	2995	195	2350
4% x 9½	HSUR/L4.75/9	14	43/4	815/16	2¾	1 ½16	2%	12-16d	2-10dx1½	1.22	13.34	0.87	10.47
45/ v 447/	LICUD /I 4 75 /11	1.1	43/	1015/	03/	17/	2%	10 104	0.104511/	275	4190	195	2965
4% x 11%	HSUR/L4.75/11	14	43/4	1015/16	2¾	1 ½16	Z%8	16-16d	2-10dx1½	1.22	18.64	0.87	13.21
4% x 14	HSUR/L4.75/14	14	43/4	13¾	2¾	17/16	2%	20-16d	2-10dx1½	275	4190	195	2965
478 A 14	113011/14.73/14	14	774	1074	274	1716	278	20-10u	Z-10UX172	1.22	18.64	0.87	13.21
4% x 16	HSUR/L4.75/16	14	43/4	15¾	2¾	17/16	2%	24-16d	2-10dx1½	275	4190	195	2965
170 % 10	110011/211/0/10		174	1074	274	1710	2,0	21 100	L TOUXT72	1.22	18.64	0.87	13.21
5 x 9½	HSUR/L5.12/9	14	51/8	9	213/16	17/16	23/8	12-16d	2-10dx1½	275	2995	195	2350
										1.22	13.34	0.87	10.47
5 x 11%	HSUR/L5.12/11	14	51/8	11	213/16	17/16	2%	16-16d	2-10dx1½	275	4190	195	2965
										1.22 275	18.64 4190	0.87 195	13.21 2965
5 x 14	HSUR/L5.12/14	14	51/8	13¾	213/16	17/16	2%	20-16d	2-10dx1½	1.22	18.64	0.87	13.21
										275	4190	195	2965
5 x 16	HSUR/L5.12/16	14	51/8	15¾	213/16	17/16	2%	24-16d	2-10dx1½	1.22	18.64	0.87	13.21
	\ A /\ A										10.01	0.07	10.21

See footnotes on page 144 WWW.TRICONTRUSS.CA

Engineered Wood & Structural Composite Lumber Connectors

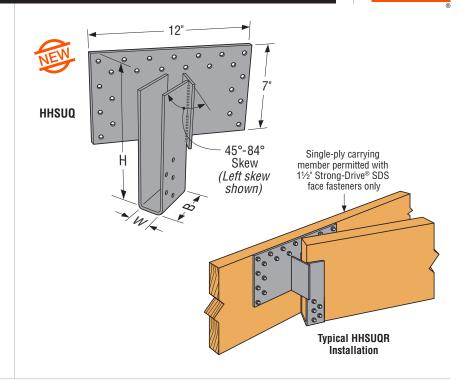
HHSUQ Heavy Severe Skew Hanger

The new HHSUQ is a high-load, face-mount hanger designed to accommodate severe skews (45°-84°), enabling a greater range of installation applications. Fastening the HHSUQ with Simpson Strong-Tie® Strong-Drive® SDS Heavy-Duty Connector screws makes installation fast and easy, while eliminating the inconvenience of bolted applications.

MATERIAL: Back plate—3 gauge; stirrup—7 gauge FINISH: Simpson Strong-Tie® grav paint INSTALLATION: • Use all specified fasteners. See General Notes.

- · Illustrations below show left and right skews HHSUQR/L (HHSUQR=skewed right; HHSUQL=skewed left).
- The joist end may be square cut or bevel cut.
- · Strong-Drive SDS Heavy-Duty Connector screws supplied for all round holes.
- · All multiple members must be fastened together to act as a single unit.
- When Strong-Drive SDS Heavy-Duty Connector screws are installed through metal truss plates the application must be approved by the Truss Designer. Pre-drilling is required using

TO ORDER: • Specify left or right skew and the skew angle (degrees)



	D	imension	ıs	Fact		Factored Resistance						
		(in)		Fasti	eners	D.F	ir-L	S-	P-F			
Model						Uplift	Normal	Uplift	Normal			
No.	w	н	В	Header	Joist	(K _D =1.15)	$(K_D = 1.00)$	(K _D =1.15)	$(K_D = 1.00)$			
	VV	п п	В	пеацег	30131	lbs	lbs	lbs	lbs			
						kN	kN	kN	kN			
HHSUQ1.81/7-SDS	1 13/16	71/4	3½	23-1/4"x3" SDS	5-¼"x1½" SDS	1890	5530	1360	5530			
סעפ-ז/וס.ואטפחח	1 '%16	1 74	3 /2	23-74 X3 SDS	3-74 X 172 3D3	8.41	24.60	6.05	24.60			
HHSUQ1.81/9-SDS	1 13/16	9½	3½	23-¼"x3" SDS	5-¼"x1½" SDS	1890	5530	1360	5530			
ппоицт.от/9-оио	1 '716	972	372	23-74 X3 3D3	3-74 X 172 3D3	8.41	24.60	6.05	24.60			
HHSUQ1.81/11-SDS	113/16	117/8	31/2	23-¼"x3" SDS	5-1/4"x11/2" SDS	1890	5530	1360	5530			
HH3UQ1.01/11-3D3	1 '916	1178	372	23-74 X3 3D3	3-74 X 172 3D3	8.41	24.60	6.05	24.60			
HHSUQ1.81/14-SDS	1 13/16	14	3½	23-1/4"x3" SDS	5-¼"x1½" SDS	1890	5530	1360	5530			
ппоицт.от/ 14-оро	1 '716	14	372	23-74 X3 3D3	3-74 X 172 3D3	8.41	24.60	6.05	24.60			
HHSUQ48-SDS	35/8	71/4	3½	23-1⁄4"x3" SDS	5-1/4"x3" SDS	1890	5530	1360	5530			
11130040-303	378	1 74	372	23-74 X3 3D3	J-74 X3 3D3	8.41	24.60	6.05	24.60			
HHSUQ410-SDS	35/8	91/4	31/2	23-1/4"x3" SDS	5-1/4"x3" SDS	1890	5530	1360	5530			
111304410-303	378	3 74	372	23-74 83 303	J-74 X3 3D3	8.41	24.60	6.05	24.60			
HHSUQ412-SDS	35/8	111/4	3½	23-¼"x3" SDS	5-1/4"x3" SDS	1890	5530	1360	5530			
1111000412-000	378	1174	372	20-74 80 000	23-¼"X3" SDS 5-¼"X3" SDS	8.41	24.60	6.05	24.60			
HHSUQ414-SDS	35/8	131/4	214	22_1/"v2" CDC	5-1/4"x3" SDS	1890	5530	1360	5530			
1111300414-303	378	1374	31/2	23-1/4"x3" SDS	J-74 X3 3D3	8.41	24.60	6.05	24.60			

- 1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed. Reduce where other load durations govern.
- Strong-Drive® SDS Heavy-Duty Connector screws that penetrate all plies of the supporting girder (screws must penetrate a minimum of 1" into the last ply of the truss) may also be used to transfer the load through all of the plies of the supporting girder. When Strong-Drive SDS Heavy-Duty Connector screws do not penetrate all plies of the supporting girder, supplemental Strong-Drive SDS Heavy-Duty Connector screws at the hanger locations may be required to transfer the load to the truss plies not penetrated by the face fasteners, as determined by the Designer. 3" long Strong-Drive SDS header fasteners may be replaced with 41/2" or 6" long Strong-Drive SDS Heavy-Duty Connector screws with no reduction in capacity.
- 3. Resistances shown are based on a minimum 2-ply 2x8 carrying member. For single 2x carrying members, replace the 3" long Strong-Drive SDS Heavy-Duty Connector screws with 11/2" long Strong-Drive SDS Heavy-Duty Connector screws and reduce the factored normal resistances to 3820 lbs (16.99 kN) D.Fir-L and 2750 lbs (12.23 kN) S-P-F. The tabulated uplift resistances do not change.
- 4. Girders must be adequately laterally braced to prevent excessive displacement due to secondary torsional stresses.
- 5. As per 12.2.2.5 CSA 086-14, the carrying member must be evaluated using a reduced cross sectional area at the hanger location. The reduction in area is equal to seven (7) 1/4" diameter x 3" long holes (11/2" long for 1/4"x11/2" Strong-Drive SDS Heavy-Duty Connector screw

MSC Multiple Seat Connector



The MSC supports the ridge and two valleys for roof construction. Ideal for dormer roof applications.

MATERIAL: Top Flange – 3 gauge;

MSC1.81, MSC2, MSC4. Stirrups – 11 gauge; MSC5 stirrups – 7 gauge.

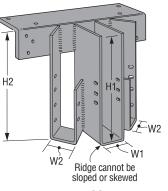
FINISH: Simpson Strong-Tie® gray paint.

INSTALLATION:

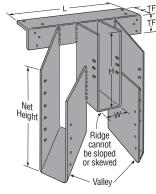
- Distribute the total load evenly about the centerline to avoid eccentric loading.
- Fasten all built-up members together as one unit.
- Net height will be calculated based on specified valley member depth and slope by the factory unless noted otherwise.

SLOPED AND/OR SKEWED VALLEYS

- The valley stirrups can be sloped 45° and skewed 25° to 45° (MSC5 skewed 20° to 40°).
- The total design capacity of the hanger is split between the ridge (20%) and each valley (40%).
- For two valley connections with no ridge member, divide the total capacity by two for each valley load.







MSC with Valley Sloped and Skewed 45°



Typical MSC Installation

		Dimer	nsions		Eo	asteners Valley		lovo	Factored Resistance (K _D = 1.00)								
		(i	n)		га	stellers	Vali	ieys		D.Fir-L			S-P-F		LV	L7 (G = 0.	50)
Model No.									Valley	Ridge	Total	Valley	Ridge	Total	Valley	Ridge	Total
110.	W	H (Min)	TF	L	Header	Joist	Max. Skew	Max. Slope	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs
		()					OKO11	оторо	kN	kN	kN	kN	kN	kN	kN	kN	kN
						18-10dx1½		0°	3085	1545	7715	2335	1170	5840	4150	2075	10375
MSC2	1%6	5½	27/8	12	10-16d	10-10UX 1 72	45°	U	13.72	6.87	34.32	10.39	5.20	25.98	18.46	9.23	46.15
IVIOUZ	1716	372	278	12	10-100	26-10dx1½	40	45°	2450	1225	6120	1855	925	4635	3290	1645	8225
						20-10ux 1 72		45	10.90	5.45	27.22	8.25	4.11	20.62	14.64	7.32	36.59
						18-10dx1½		0°	3085	1545	7715	2335	1170	5840	4150	2075	10375
MSC1.81	113/16	5½	27/8	12	10-16d	10-10ux 1 72	45° –		13.72	6.87	34.32	10.39	5.20	25.98	18.46	9.23	46.15
101301.01	I 716	372	278	12	10-100	26-10dx1½	40	45°	2450	1225	6120	1855	925	4635	3290	1645	8225
						20-10ux172		40	10.90	5.45	27.22	8.25	4.11	20.62	14.64	7.32	36.59
						18-10d		0°	5460	2730	13650	4135	2070	10340	5460	2730	13650
MSC4	3%16	7½	27/8	18	10-16d	10-100	45°	U	24.29	12.14	60.72	18.39	9.21	46.00	24.29	12.14	60.72
101304	3/16	1 /2	2/8	10	10-100	26-10d	40	45°	5460	2730	13650	4135	2070	10340	5460	2730	13650
						20-10u		40	24.29	12.14	60.72	18.39	9.21	46.00	24.29	12.14	60.72
						18-16d	45° 45°	O°	10565	5280	26410	7990	3995	19975	10565	5280	26410
MSC5	51/4	91/2	27/8	26	13-16d	10-100		U	47.00	23.49	117.48	35.54	17.77	88.86	47.00	23.49	117.48
IVIOUS	J 74	372	278	20	13-10u	26-16d		45°	9130	4565	22825	6905	3450	17260	9130	4565	22825
						20-10u		40	40.61	20.31	101.53	30.72	15.35	76.78	40.61	20.31	101.53

- 1. Factored resistances shown for each valley.
- Other valley-ridge load distributions are allowed provided the sum of all three members is distributed symmetrically about the centre of the hanger and combined do not exceed the total resistance.
- 3. MSC4 is also available in 31/8" Glulam width.
- 4. MSC5 is also available in widths up to $5\frac{1}{2}$ ".
- 5. MSC1.81 and MSC2 are available in saddle configurations. (e.g. MSCD1.81)
- 6. For the MSC5 with all three members sloped to 45° (max.) multiply the tabulated resistance x 0.64. This connection requires 30-16d joist nails.
- 7. Factored resistances shown for LVL assume $\phi F_{CP} = 1092 \text{ psi} (7.53 \text{ MPa})$.
- 8. NAILS: 16d = 0.162" dia. x 3 ½" long, 10d = 0 148" dia. x 3" long, 10dx ½ = 0.148" dia. x 11½ long. See pages 2 -23 for other hail sizes and information.

HRC/HHRC Hip Ridge Connectors

The HRC series are field slopeable connectors that attach hip roof beams to the end of a ridge beam. The HRC may be sloped downward a maximum of 45°.

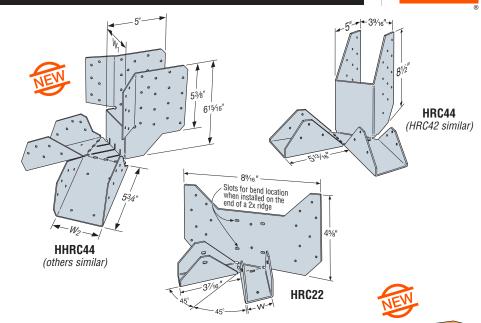
MATERIAL: HRC1.81—16 gauge;

HRC44—14 gaugel; HHRC—12 gauge

FINISH: Galvanized

INSTALLATION:

- · Use all specified fasteners. See General Notes.
- On end of ridge—use optional diamond holes on HRC1.81 to secure the HRC. Bend face flanges on HRC1.81 back flush with ridge, and complete nailing.
- HRC1.81 on face of ridge—adjust to correct height and install nails.
- . Double bevel-cut hip members to achieve full bearing capacity.



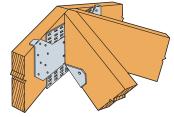
HRC Factored Resistances

	Me	mber Size	Engle	eners	Factored Resistance							
		(in)	Газів	:11612	D.F	ir-L	S-P-F					
Model No. W HRC1.81 1134 HRC44 334					Uplift	Down	Uplift	Down				
No.	w	Ridge	Carrying	Each	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$				
Model No. W	VV	niuye	Member	Hip	lbs	lbs	lbs	lbs				
					kN	kN	kN	kN				
UDC1 91	113/	2x or 13/4"	16-10dy 11/4"	2-10dx11/2	445	1340	400	950				
11001.01	1 716	wide	10-10ux 172	Z-100X172	1.98	5.96	1.78	4.23				
прсии	29/	4x		6-16d	790	2625	560	2035				
IINU44	J 716	48	16-10dx 1½" 24-16d	0-10u	3.51	11.68	2.49	9.05				

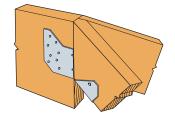


 ${f Strong-Tie}$

- 1. Factored resistances shown are for each hip. Total resistance carried by the connector is double this number.
- 2. Factored uplift resistances include a 15% increase for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
- 3. **NAILS:** 16d = 0.162" dia. x $3\frac{1}{2}$ " long, 10dx1½ = 0.148" dia. x 1½" long. See pages 22-23 for other nail sizes and information.



Optional Installation for HRC1.81 only



Typical HRC Installation on the End of a Ridge

HHRC Factored Resistances

		Membe	r Width	Dimensions		Faste	nore		Factored Resi	stance per Hip	
		(iı	n)	(i	n)	rasit	:11013	D.Fir-L		S-P-F	
	Model							Uplift	Normal	Uplift	Normal
	No.	Ridge	Hip	w.	W.	Ridge	Each	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
		niuge	пір	W ₁	W ₂	niuge	Hip	lbs	lbs	lbs	lbs
								kN	kN	kN	kN
N.	HHRC44	3½	3½	35/8	35/8	40-SD#10x2½	22-SD#10x2½	3365	4185	2390	2970
T	IIIInu44	372	372	378	378	40-3D#10X272	22-3D#10X272	14.97	18.62	10.63	13.21
	HHRC5.37/3.56	51/4	3½	5 3 /8	33/8	40-SD#10x2½	22-SD#10x2½	3930	4205	2790	2985
9	1111103.37/3.30	J /4	372	J78	378	40-00#108272	22-3D#10X2/2	17.48	18.71	12.41	13.28
N.	ннвсел	5½	3½	5 ⁵ /8	35/8	40-SD#10x2½	22-SD#10x2½	3930	4205	2790	2985
9	HHRC64	372	372	J78	3%	40-SD#10X2½	22-3D#10X272	17.48	18.71	12.41	13.28

- 1. Factored resistances shown are per hip, the total load carried by the connector is double this number. Load must be equally distributed to both hips.
- 2. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed. Reduce where other load durations govern.
- 3. Factored resistances shown are applicable for roof slopes up to and including 45° (12:12).
- 4. Do not attach HHRC to columns or studs
- 5. SCREWS: SD#10x2½" (SD10212)

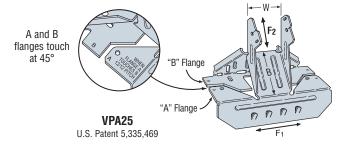
VPA Variable Pitch Connector

The VPA may be sloped in the field, offering a versatile solution for attaching rafters to the top plate. It will adjust to accommodate slopes between 3:12 and 12:12, making it a complement to the versatile LSSU. This connector eliminates the need for notched rafters, beveled top plates and toe nailing.

MATERIAL: 18 gauge FINISH: Galvanized

INSTALLATION: • Use all specified fasteners.

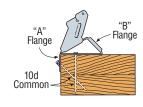
See General Notes.



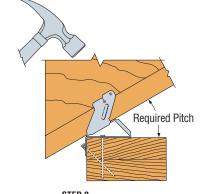
			Fas	teners	Ĭ			Factored R	esistance			
	Antual					D.F	ir-L			S-I	P-F	
Model	Actual Joist	w			Wind/E	arthquake (K	D=1.15)	Normal	Wind/E	arthquake (K	(D=1.15)	Normal
No.	Width	(in)	Carrying Member	Carried Member	Uplift	F ₁	F ₂	$(K_D = 1.00)$	Uplift	F ₁	F ₂	$(K_D = 1.00)$
	(in)		Intelliger	Moniboi	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs
					kN	kN	kN	kN	kN	kN	kN	kN
VPA2	1½	1%6	8-10d	2-10dx1½	405	695	405	1695	370	615	370	1555
VPAZ	1 //2	I 7/16	0-100	2-100X1½	1.80	3.09	1.80	7.54	1.65	2.74	1.65	6.92
VPA25	13/4	1 13/16	0.104	2-10dx1½	405	695	405	1695	370	615	370	1555
VPAZO	194	I '9/16	8-10d	2-100X1½	1.80	3.09	1.80	7.54	1.65	2.74	1.65	6.92
VDA0.00	0	01/	0.404	0.404541/	405	695	405	2050	370	615	370	1855
VPA2.06	2	21/16	9-10d	2-10dx1½	1.80	3.09	1.80	9.12	1.65	2.74	1.65	8.25
\/DA0.1	01/	01/	0.104	0.404541/	405	695	405	2050	370	615	370	1855
VPA2.1	21/16	21/8	9-10d	2-10dx1½	1.80	3.09	1.80	9.12	1.65	2.74	1.65	8.25
VPA35	21/4 - 25/16	25/16	9-10d	2-10dx1½	405	695	405	2050	370	615	370	1855
VPA33	274 - 2716	2 %16	9-100	Z-100X172	1.80	3.09	1.80	9.12	1.65	2.74	1.65	8.25
VDAO	01/	09/	00/	0.40441/	405	695	405	2050	370	615	370	1855
VPA3	2½	29/16	9-10d	2-10dx1½	1.80	3.09	1.80	9.12	1.65	2.74	1.65	8.25
VDA 4	01/	29/	11-10d	0.40-1-41/	405	695	405	2050	370	615	370	1855
VPA4	VPA4 3½ 3% ₁₆	11-100	2-10dx1½	1.80	3.09	1.80	9.12	1.65	2.74	1.65	8.25	

- Factored uplift and lateral resistances have been increased 15% for earthquake or wind loading; no further increase is allowed.
- 2. Resistances may not be increased for short-term load duration.
- 3. NAILS: $10d = 0.148^\circ$ dia. x 3" long, $10dx1\frac{1}{2} = 0.148^\circ$ dia. x $1\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.

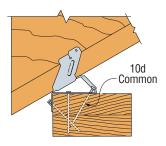
VPA INSTALLATION SEQUENCE



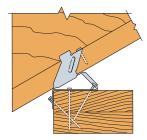
STEP 1
Install top nails and face PAN nails in "A" flange to outside wall top plate.



Seat rafter with a hammer, adjusting "B" flange to the required pitch.



STEP 3
Install "B" flange nails in the obround nail holes, locking the pitch.



STEP 4
Bend tab with hammer and install 10dx1½" nail into tab nail hole.
Hammer nail in at an approximate 45° angle to limit splitting.

HCP Hip Corner Plates

The HCP connects a rafter or joist to double top plates at a 45° angle.

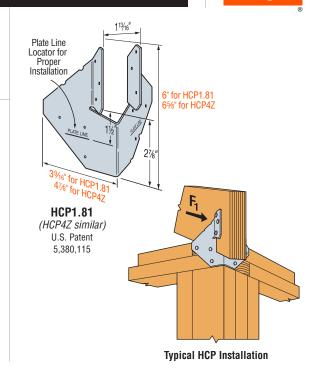
MATERIAL: 18 gauge FINISH: Galvanized

INSTALLATION: • Use all specified fasteners. See General Notes.

- Attach HCP to double top plates; birdsmouth not required for table loads.
- Install rafter and complete nailing. Rafter may be sloped to 45°.

		Faste	eners		Factored F	Resistance	
				D.F	ir-L	S-I	P-F
Model	Hip			Uplift	F ₁	Uplift	F ₁
No.	Size (in)	To Hip	To Plates	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
			1 10100	lbs	lbs	lbs	lbs
				kN	kN	kN	kN
HCP1.81	13/4	6-10dx1½	6-10dx1½	1020	355	890	325
HUF 1.01	174	0-100X172	0-10ux 1 72	4.54	1.58	3.96	1.45
HCP4Z	214	8-10d	8-10d	1485	435	1300	310
	3½	0-10u	0-100	6.61	1.94	5.78	1.38

- The HCP can be installed on the inside and the outside of the wall with a flat bottom chord truss and achieve twice the factored resistance.
- Factored uplift resistances include a 15% increase for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
- 3. NAILS: 10d = 0.148" dia. x 3" long, $10dx1\frac{1}{2} = 0.148$ " dia. x $1\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.



DU/DHU/DHUTF Drywall Hanger

The new DU/DHU face-mount and the DHUTF top-mount hangers are designed to carry joist floor loads to a wood stud wall through two layers of 5% gypsum board (drywall). These hangers install after the drywall is in place. The hangers come in sizes that accommodate most joists used in multi-family construction including I-joists and trusses.

MATERIAL: DU—14 gauge; DHU and DHUTF—12 gauge FINISH: Galvanized

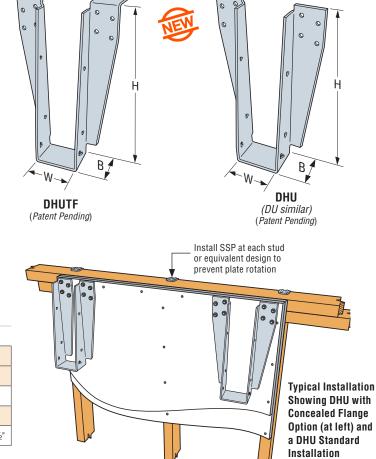
INSTALLATION:

- Use all specified fasteners. See General Notes.
- Install with Simpson Strong-Tie® ¼"x3½" Strong-Drive® SDS Heavy-Duty Connector screws, which are provided with the hanger.
- Drywall is installed first.
- DU and DHU are mounted with top of hanger flush with top of wall and tight to the drywall.
- Wall top plates must be retrained to prevent rotation.
 Use an SSP stud plate tie connector at the back of each stud or provide equivalent restraint by another method as determined by Designer.
- Upper plate splices must occur at a stud location.

OPTION:

 The DHU may be ordered with one flange concealed for widths at least 2½" wide; specify which flange when ordering. Use 74% of the tabulated values.

Model	Gauge	В		Fasteners	
Model	uauye	В	Joist	Face	Тор
DU	14	2	(2) 10dx1½"	(4) 1/4"x31/2" SDS	_
DHU	12	2.5	(2) 10dx1½"	(8) 1/4"x31/2" SDS	_
DHUTF	12	2.5	(2) 10dx1½"	(8) 1/4"x31/2" SDS	(6) 10dx1½"



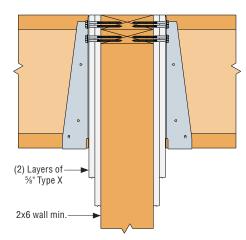
DU/DHU/DHUTF Drywall Hanger



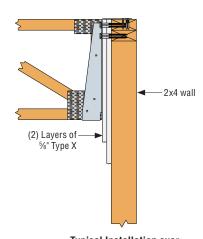
			Factored	Resistance	
		D.	Fir-L	S-I	P-F
Model	Condition	Uplift	Normal	Uplift	Normal
Monei	Condition	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
		lbs	lbs	lbs	lbs
		kN	kN	kN	kN
	Over (1) layer 5/8" drywall	125	1605	125	1605
	Over (1) layer 78 drywan	0.56	7.14	0.56	7.14
	Over (2) layers 5/8" drywall	125	2090	125	1845
DII	Over (2) layers 78 drywaii	0.56	9.30	0.56	8.21
DU	Two-sided over (2) layers	125	1780	125	1780
	%" drywall (minimum 2x6 wall)	0.56	7.92	0.56	7.92
	Over (1) layer structural sheathing	125	1845	125	1845
	and (2) layers %" drywall	0.56	8.21	0.56	8.21
	Over (1) lever 5/ dravell	125	1990	125	1670
	Over (1) layer %" drywall	0.56	8.85	0.56	7.43
	Over (2) levere 5/" drawell	125	2310	125	2295
DHU	Over (2) layers 1/8" drywall	0.56	10.28	0.56	10.21
DHUTF	Two-sided over (2) layers	125	1780	125	1780
	5/8" drywall (minimum 2x6 wall)	0.56	7.92	0.56	7.92
	Over (1) layer structural sheathing	125	2420	125	2030
	and (2) layers 5/8" drywall	0.56	10.77	0.56	9.03

- Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed. Reduce where other load durations govern.
- Triangle nail holes may be filled with four (4) additional 10dx1½" nails to achieve a factored uplift resistance of 1290 lbs (5.74 kN). When I-joists are used, web stiffeners are required. Note that the double top plates must be anchored to the studs/framing to accommodate the uplift load.
- 3. Factored resistances assume 5/s" Type X gypsum board attached per NBC. Wall assembly must consist of a minimum of 2-2x4 top plates with studs spaced no greater than 16" o/c.
- 4. Capacities shown for use over one (1) layer of structural sheathing are based on 5/8" thick OSB, DF plywood or CSP. For thinner panels use the lower of this value or the other applicable values in the table.

Joist	Face	Mount	Top Flange		nsions n)
Size	DU Models	DHU Models	DHUTF Models	W	Н
2x10	DU210	DHU210	DHU210TF	1%16	91/8
2x12	DU212	DHU212	DHU212TF	1%16	111/8
13/4 x 91/2	DU1.81/9.5	DHU1.81/9.5	DHU1.81/9.5TF	1 13/16	97/16
13/4 x 117/8	DU1.81/11.88	DHU1.81/11.88	DHU1.81/11.88TF	1 13/16	11 13/16
13/4 x 14	DU1.81/14	DHU1.81/14	DHU1.81/14TF	1 13/16	1315/16
13/4 x 16	DU1.81/16	DHU1.81/16	DHU1.81/16TF	1 13/16	15 ¹⁵ / ₁₆
2 x 9 ½	DU2.1/9.5	DHU2.1/9.5	DHU2.1/9.5TF	21/8	97/16
2 x 1 1/8	DU2.1/11.88	DHU2.1/11.88	DHU2.1/11.88TF	21/8	11 ¹³ / ₁₆
2 x 14	DU2.1/14	DHU2.1/14	DHU2.1/14TF	21/8	13 ¹⁵ / ₁₆
2 x 16	DU2.1/16	DHU2.1/16	DHU2.1/16TF	21/8	15 ¹⁵ / ₁₆
21/16 x 91/2	DU2.1/9.5	DHU2.1/9.5	DHU2.1/9.5TF	21/8	97/16
2½ x 11%	DU2.1/11.88	DHU2.1/11.88	DHU2.1/11.88TF	21/8	11 ¹³ / ₁₆
21/16 x 14	DU2.1/14	DHU2.1/14	DHU2.1/14TF	21/8	1315/16
2½ x 16	DU2.1/16	DHU2.1/16	DHU2.1/16TF	21/8	1515/16
25/16 x 91/2	DU2.37/9.5	DHU2.37/9.5	DHU2.37/9.5TF	23/8	97/16
25/16 x 117/8	DU2.37/11.88	DHU2.37/11.88	DHU2.37/11.88TF	23/8	11 ¹³ / ₁₆
25/16 x 14	DU2.37/14	DHU2.37/14	DHU2.37/14TF	23/8	13 ¹⁵ / ₁₆
25/16 x 16	DU2.37/16	DHU2.37/16	DHU2.37/16TF	23/8	15 ¹⁵ / ₁₆
25/16 x 18	_	DHU2.37/18	DHU2.37/18TF	23/8	17 ¹⁵ / ₁₆
25/16 x 20	_	DHU2.37/20	DHU2.37/20TF	23/8	19 ¹⁵ / ₁₆
2½ x 9½	_	DHU2.56/9.5	DHU2.56/9.5TF	2%16	97/16
2½ x 11%	_	DHU2.56/11.88	DHU2.56/11.88TF	2%16	11 13/16
2½ x 14	_	DHU2.56/14	DHU2.56/14TF	2%16	1315/16
2½ x 16	_	DHU2.56/16	DHU2.56/16TF	2%16	15 ¹⁵ / ₁₆
2½ x 18	_	DHU2.56/18	DHU2.56/18TF	2%16	17 ¹⁵ / ₁₆
2½ x 20	_	DHU2.56/20	DHU2.56/20TF	2%16	1915/16
3½ x 9½	_	DHU3.56/9.5	DHU3.56/9.5TF	3%16	97/16
3½ x 11%	_	DHU3.56/11.88	DHU3.56/11.88TF	3%16	11 ¹³ / ₁₆
3½ x 14	_	DHU3.56/14	DHU3.56/14TF	3%16	13 ¹⁵ / ₁₆
3½ x 16	_	DHU3.56/16	DHU3.56/16TF	3%16	15 ¹⁵ / ₁₆
3½ x 18	_	DHU3.56/18	DHU3.56/18TF	3%16	1 7¹5⁄₁6
3½ x 20	_	DHU3.56/20	DHU3.56/20TF	3%16	19¹5⁄₁6
3½ x 22		DHU3.56/22	DHU3.56/22TF	3%16	21 15/16
3½ x 24	- \ \	DHU3.56/24	DHU3.56/24TF	39/16	23 15/16



Two-Sided Installation Over (2) Layers of Drywall



Typical Installation over
(2) Layers of Drywall

STRONG-DRIVE® SDW TRUSS-PLY screw



T-40

Driver Bit (included)

BIT40-R1

The Strong-Drive® SDW Truss-Ply screw is a 0.22" diameter, high-strength structural wood screw specifically designed for fastening together multi-ply wood members such as plated trusses, engineered-lumber products and solid-sawn lumber. The Strong-Drive SDW Truss-Ply screw installs easily with no pre-drilling and is available in optimized lengths for fastening 2, 3 and 4-ply trusses or 1¾ structural composite lumber (SCL). The Strong-Drive SDW Truss-Ply screw enables single-side fastening, while still allowing concurrent loading on both sides of the assembly.

- · Low-profile head for reduced interference during handling or installation of hardware on the assembly
- · High shear values enable wider screw spacing
- · Bold thread design firmly cinches plies together to close gaps in multi-ply assemblies
- Optimal screw lengths provide maximum penetration

MATERIAL: Heat-treated carbon steel

FINISH: Black E-coat™

WARNING: Industry studies show that hardened fasteners can experience performance problems in wet or corrosive environments. Accordingly, the Strong-Drive SDW Truss-Ply screws should only be used in dry, interior and non-corrosive environments.

INSTALLATION: • Use all specified fasteners. See General Notes.

- \bullet Strong-Drive SDW Truss-Ply screws install best with a low-speed $1\!\!/\!\!2$ drill and a T-40 6-lobe bit. The matched bit included with the screws is recommended for best results.
- Pre-drilling is typically not required. Strong-Drive SDW Truss-Ply screws may be installed through metal truss plates as approved by the Truss Designer (pre-drilling required through the plate using a maximum of 5/32" bit).
- Screw heads that are countersunk flush to the wood surface are acceptable if the screw has not spun out.

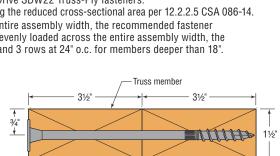
NOTES TO THE DESIGNER:

- 1. Factored lateral and withdrawal resistances are based on testing per ICC-ES AC233 and section 12.11 of CSA 086-14.
- 2. Factored lateral resistances may be increased 15% for short-term load duration ($K_D = 1.15$). For load durations other than standard or short-term, see 5.3.2 of CSA 086-14 for reduction values.
- 3. Fastener spacing, end and edge distances shall conform to Table 12.9.2.1 CSA 086-14 using a diameter value of 0.30" (see table on page 153).
- 4. Maximum fastener spacing is recommended not to exceed 24" on-centre except as approved by a qualified Designer.
- 5. Structural composite lumber (SCL) is laminated veneer lumber (LVL), parallel strand lumber (PSL) or laminated strand lumber (LSL). Verify the effective specific gravity (SG) with structural composite lumber manufacturer for selection of tabulated values.
- 6. Factored resistances are based on the capacity of the Simpson Strong-Tie® Strong-Drive SDW22 Truss-Ply fasteners. The capacity of the multi-ply assembly must be checked by a qualified Designer using the reduced cross-sectional area per 12.2.2.5 CSA 086-14.
- 7. For top loaded solid sawn 2x built-up assemblies that are evenly loaded across the entire assembly width, the recommended fastener spacing is two rows at 32" o.c., For top-loaded SCL 1¾" built up assemblies that are evenly loaded across the entire assembly width, the recommended fastener spacing is two rows at 24" o.c. for up to 18" deep members, and 3 rows at 24" o.c. for members deeper than 18".
- 8. For more information see F-SDWCAN.

Factored Lateral Resistance for 3x2 and 4x2 Parallel-Chord Trusses

		Nominal	Side	Factored Lateral Resistance (K _D = 1.00)			
Assembly	Model No.	Length	Member Thickness	D.Fir-L	S-P-F		
		(in)	(in)	lbs	lbs		
				kN	kN		
2 ply 2v2 DCT	SDW22500	5	21/2	405	290		
2-ply 3x2 PCT	3DW22300	5	Z 72	1.80	1.29		
2-ply 4x2 PCT	CDM33634	63/4	3½	405	290		
	SDW22634	074	372	1.80	1.29		

- 1. To transfer uniform loads applied to simply supported spans on assembly top chord: a. Space screws as required to transfer half the load into the supporting truss.
- b. Minimum screw spacing shall be 4" o.c.
- 2. To transfer concentrated loads applied to simply supported spans on an assembly top chord or vertical web:
 - a. Concentrated loads must be applied at the panel joints.
- b. Screws to be installed within 12" of the concentrated load on top-chord assembly
- 3. Gap between the trusses shall not exceed 1/8" o.c.
- 4. Floor sheathing shall be screwed or nailed to each top-chord ply. (Fastener spacing per the applicable Code requirements, or 12" o.c.)
- 5. Strong-Drive® SDW Truss-Ply screws shall not be installed in areas where lumber wane exceeds 1/4".
- 6. Truss members must be evaluated using a reduced cross-sectional area due to the 0.22" diameter Strong-Drive SDW Truss-Ply screw.
- 7. Other configurations acceptable as long as approved by Truss Designer.



0.22" ->

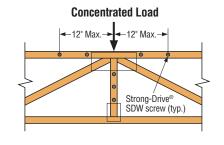
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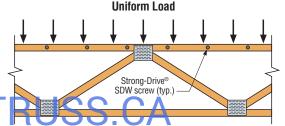
Strong-Drive® SDW TRUSS-PLY Screw

U.S. Patents 5,897,280; 7,101,133;

and patent pending

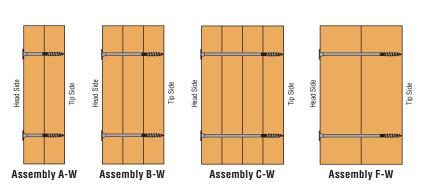
Strong-Drive® SDW TRUSS-PLY Screw Position in 2-Ply 4x2 Truss (2-ply 3x2 similar)





STRONG-DRIVE® SDW TRUSS-PLY Screw





Spacing between fasteners parallel to grain

Spacing Requirement (See table below)

Sideloaded Multi-Ply Truss Assemblies - Factored Uniform Load Applied to Outer Ply

				Maximum Factored Uniform Load Applied to Outer Ply											
							Maxim	ıum Factor	red Unifori	n Load Ap	olied to Ou	ter Ply			
Multiple	Members					D.F	ir-L					S-I	P-F		
		Nominal	Loaded	SDW @ 12" o.c.		SDW @ 16" o.c.		SDW @	24" o.c.	SDW @	12" o.c.	SDW @ 16" o.c.		SDW @ 24" o.c.	
		Length (in)	Side	2 Rows	3 Rows	2 Rows	3 Rows	2 Rows	3 Rows	2 Rows	3 Rows	2 Rows	3 Rows	2 Rows	3 Rows
Assembly	Component	(111)		plf	plf	plf	plf	plf	plf	plf	plf	plf	plf	plf	plf
				kN/m	kN/m	kN/m	kN/m	kN/m	kN/m	kN/m	kN/m	kN/m	kN/m	kN/m	kN/m
			Head	1340	2010	1005	1508	670	1005	1160	1740	870	1305	580	870
A-W	2-ply 2x Truss	3	Heau	19.55	29.33	14.66	22.00	9.78	14.66	16.93	25.39	12.69	19.04	8.46	12.69
A-vv	2x Truss	3	Tip	1220	1830	915	1373	610	915	1040	1560	780	1170	520	780
			ПΡ	17.80	26.70	13.35	20.03	8.90	13.35	15.17	22.76	11.38	17.07	7.59	11.38
	3-ply 2x Truss	45/8	Head	1364	2046	1023	1535	682	1023	1214	1822	911	1366	607	911
B-W			Hoad	19.91	29.86	14.93	22.39	9.95	14.93	17.72	26.58	13.29	19.93	8.86	13.29
D W			Tip	1229	1844	922	1383	615	922	1094	1642	821	1231	547	821
			ПР	17.94	26.91	13.45	20.18	8.97	13.45	15.97	23.95	11.98	17.96	7.98	11.98
			Head	1213	1820	910	1365	607	910	1080	1620	810	1215	540	810
		6	Hoad	17.70	26.55	13.28	19.92	8.85	13.28	15.76	23.64	11.82	17.73	7.88	11.82
		0	Tin	1093	1640	820	1230	547	820	973	1460	730	1095	487	730
C-W	4-ply		Tip	15.95	23.93	11.96	17.95	7.98	11.96	14.20	21.30	10.65	15.98	7.10	10.65
C-W	2x Truss		Head	1213	1820	910	1365	607	910	1080	1620	810	1215	540	810
		63/8		17.70	26.55	13.28	19.92	8.85	13.28	15.76	23.64	11.82	17.73	7.88	11.82
		0/8	Tip	1093	1640	820	1230	547	820	973	1460	730	1095	487	730
			ιιρ	15.95	23.93	11.96	17.95	7.98	11.96	14.20	21.30	10.65	15.98	7.10	10.65

Sideloaded Multi-Ply SCL Assemblies - Factored Uniform Load Applied

							Maxim	um Factor	ed Uniforr	n Load Apı	olied to Ou	ter Ply			
Multiple	e Members	l				SCL (SI	G = 0.5)					SCL (SG	i = 0.42)		
		Nominal Length	Loaded	SDW @	12" o.c.	SDW @	16" o.c.	SDW @	24" o.c.	SDW @	12" o.c.	SDW @	16" o.c.	SDW @	24" o.c.
		(in)	Side	2 Rows	3 Rows	2 Rows	3 Rows	2 Rows	3 Rows	2 Rows	3 Rows	2 Rows	3 Rows	2 Rows	3 Rows
Assembly	Component	(,		plf	plf	plf	plf	plf	plf	plf	plf	plf	plf	plf	plf
				kN/m	kN/m	kN/m	kN/m	kN/m	kN/m	kN/m	kN/m	kN/m	kN/m	kN/m	kN/m
			Head	1560	2340	1170	1755	780	1170	1300	1950	975	1463	650	975
A-W	2-ply 1¾" SCL	3%	Heau	22.76	34.14	17.07	25.61	11.38	17.07	18.97	28.45	14.23	21.34	9.48	14.23
A-vv	1¾" SCL	378	Tip	1360	2040	1020	1530	680	1020	1140	1710	855	1283	570	855
			Пр	19.84	29.76	14.88	22.32	9.92	14.88	16.63	24.95	12.48	18.71	8.32	12.48
		5	Head	1484	2226	1113	1670	742	1113	1289	1934	967	1451	645	967
B-W	3-ply 1¾" SCL		Heau	21.66	32.48	16.24	24.36	10.83	16.24	18.81	28.22	14.11	21.16	9.41	14.11
D-44	1¾" SCL	J	Tip	1244	1867	933	1400	622	933	1094	1642	821	1231	547	821
			Пр	18.16	27.23	13.62	20.43	9.08	13.62	15.97	23.95	11.98	17.96	7.98	11.98
		Hea	Head	1320	1980	990	1485	660	990	1147	1720	860	1290	573	860
C-W	4-ply	63/	Heau	19.26	28.89	14.44	21.67	9.63	14.44	16.73	25.10	12.55	18.82	8.37	12.55
U-VV	1¾" SCL	6¾	***	1107	1660	830	1245	553	830	973	1460	730	1095	487	730
			Tip	16.15	24.22	12.11	18.17	8.07	12.11	14.20	21.30	10.65	15.98	7.10	10.65
			Hood	2280	3420	1710	2565	1140	1710	2020	3030	1515	2273	1010	1515
F-W	2-ply 3½" SCL	6¾	Head	33.27	49.90	24.95	37.43	16.63	24.95	29.47	44.21	22.10	33.16	14.74	22.10
1 2 00	3½" SCL	074	Tip	2280	3420	1710	2565	1140	1710	1960	2940	1470	2205	980	1470
	372 30L		ıψ	33.27	49.90	24.95	37.43	16.63	24.95	28.60	42.90	21.45	32.17	14.30	21.45

- 1. Each ply is assumed to carry same proportion of load.
- Loads may be applied to the head side and tip side concurrently provided neither published capacity is exceeded. (Example: a 4-ply D.Fir-L truss assembly with a head side load of 1300 plf and tip side load of 1200 plf may be fastened together with 3 rows of 6° Strong-Drive® SDW Truss-Ply screws @ 16° o.c.)
- When hangers are installed on tip side, hanger face fasteners must be a minimum of 3" long.
- 4. Hanger load spacing on the multi-ply assembly should not exceed 24" o.c. for side-loaded members unless approved by the Designer.

Spacing Requirements (See diagram above)

Geometry	Minimum Dir	mensions (in)
deometry	D.Fir-L	S-P-F
a - Spacing parallel to grain	6	5
b - End distance parallel to grain	6	6
c - Spacing perpendicular to grain	3	2½
d Edge distance perpendicular to grain	17/16	1 ½16



LUL/LUS/LJS/HUS/HHUS/HGUS Standard & Double Shear Joist Hangers



This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double shear nailing – an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

MATERIAL: See tables below and on page 155

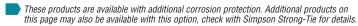
FINISH: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pages 14-17.

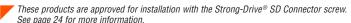
INSTALLATION: • Use all specified fasteners. See General Notes.

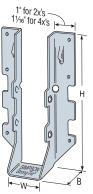
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With 3x carrying members, use 16dx2½" nails into the header and 16d commons into the joist with no reduction in resistance. With 2x carrying members, use 10dx11/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value.

OPTIONS: • LUS and LUL hangers cannot be modified.

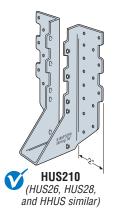
- HUS hangers available with the header flanges turned in for 2-2x (31/8") and 4x only, with no load reduction. See HUSC Concealed Flange illustration.
- · Concealed flanges are not available for HGUS.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See hanger options on page 230.

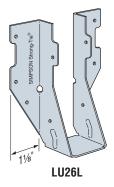


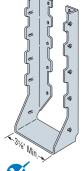




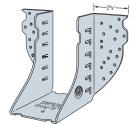




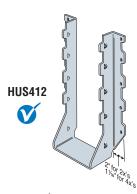


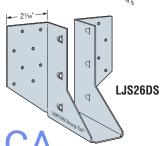


🚺 HUSC **Concealed Flanges** (not available for HHUS. HGUS and HUS2x)









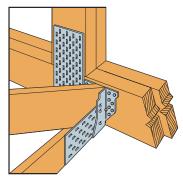
				Dime	nsions	S	Engl	lonoro				
				(i	n)		газі	teners	D.F	ir-L	S-I	P-F
	Model	Ga							Uplift	Normal	Uplift	Normal
	No.	ua	w	н	В	de3	Header	Joist	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
			VV	п	D	ue	пеацеі	Juist	lbs	lbs	lbs	lbs
									kN	kN	kN	kN
ĺ							SI	NGLE 2x SIZ	ES			
	LUS24	18	19/	21/	13/4	21/4	4 104	0.104	710	1625	645	1155
4	LU524	10	1%16	31/8	1 74	274	4-10d	2-10d	3.16	7.23	2.87	5.14
	LU24L	22	1%16	3	15%	211/16	4-10d	2-10dx1½	360	1020	320	725
	LUZ4L	22	1716	٥	178	2.716	4-10u	Z-10UX 1 72	1.60	4.54	1.42	3.22
	LU26L	22	1%16	5	15%	45/8	6-10d	4-10dx1½	720	1605	645	1140
l	LUZUL	22	1716	J	178	7/8	0-10u	4-10UX172	3.20	7.14	2.87	5.07
	LUS26	18	1%16	43/4	13/4	3¾	4-10d	4-10d	1420	2170	1290	1630
	LU320	10	1716	774	1 74	J74	4-10u	4-100	6.32	9.65	5.74	7.25
ı	HUS26	16	15%	1% 5% 3		215/	1/1-164	6-16d	2705	4940	2065	3875
l	110020	10	1 78	J78	J	315/16 14-16d	0-100	11.30	21.97	9.20	17.24	
	LJS26DS	18	1%16	5	3½	45/8	16-16d	6-16d	2055	4265	1460	4115
	LUUZUDU	10	1716	J	372	478	10-100	0-100	9.14	18.97	6.49	18.31
	HGUS26	12	15/8	5%	5	41/8	20-16d	8-16d	2685	6625	2685	5700
ļ	1100020	12	170	J/8	3	7/0	20 100	0 100	11.96	29.51	11.96	25.35
	LU28L	20	1%16	63/4	15/8	57/8	8-10d	6-10dx1½	1140	2185	1020	1550
	LUZUL	20	1716	074	178	J /8	0-10u	0-100X172	5.07	9.72	4.54	6.89
l	LUS28	18	1%16	65/8	13/4	3¾	6-10d	4-10d	1420	2520	1290	1790
l	LU320	10	1716	078	174	J74	0-10u	4-100	6.32	11.21	5.74	7.96
l	HUS28	16	1%	71/16	3	61/16	22-16d	8-16d	3605	5365	2675	4345
ļ	110020	10	170	1 / 10	J	0716	22 10u	0 100	16.04	23.86	11.90	19.33
	HGUS28	12	1%	71/8	5	61/8	36-16d	12-16d	3310	7675	3310	6900
l	1100020	12	170	1 /0	0	078	00 10u	12 10u	14.74	34.19	14.74	30.73
	LU210L	20	1%16	8	15%	75/8	10-10d	6-10dx1½	1140	2495	1020	1770
	LUZIUL	20	1 / 10	U	176	1 /0	10 100	0 10UX172	5.07	11.10	4.54	7.87
	LUS210	18	1%16	713/16	13/4	31/8	8-10d	4-10d	1420	2785	1290	2210
	L00210	10	1 / 10	7 716	174	070	0 100	7 100	6.32	12.39	5.74	9.83
;	See footnotes	on pa	age 15	5.	V	W	W	.TR		DN.	TRI	JS:

FACE MOUNT HANGERS



- These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.
- These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

				Dimensions (in)			Fast	eners	D.Fir-L S-P-F				
	Model			(11	11)				U,F Uplift	ir-L Normal	Uplift	7-F Normal	
	No.	Ga							(K _D = 1.15)	$(K_D = 1.00)$	(K _D = 1.15)	$(K_D = 1.00)$	
	110.		W	Н	В	d _e ³	Header	Joist	lbs	lbs	lbs	lbs	
									kN	kN	kN	kN	
							DOU	BLE 2x SIZ	ES				
	LUS24-2	18	31/8	31/8	2	1½	4-16d	2-16d	835	2020	590	1435	
	10024 2	10	070	070		172	4 10u	2 100	3.71	8.99	2.62	6.38	
)	LUS26-2	18	31//8	47/8	2	4	4-16d	4-16d	1720 7.65	2595 11.54	1545 6.87	1920 8.54	
ŀ	IIIIIICOC O	14	35/16	5%	3	315/16	14 164	C 1C4	2850	7335	2065	5205	
'	HHUS26-2	14	3716	378	٠	3 '916	14-16d	6-16d	12.68	32.63	9.20	23.15	
	HGUS26-2	12	35/16	51/16	4	41/8	20-16d	8-16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27	
	111000 0	10	01/	7	2	4	C 1C4	4 104	1720	3325	1545	2575	
1	LUS28-2	18	31/8	/		4	6-16d	4-16d	7.65	14.79	6.87	11.45	
	HHUS28-2	14	35/16	75/16	3	61//8	22-16d	8-16d	3765 16.75	8940 39.77	2675 11.90	6345 28.22	
-		40	05/	70/		04/	00.40.1	10.10.1	6070	12980	4310	9215	
	HGUS28-2	12	35/16	73/16	4	61/8	36-16d	12-16d	27.00	57.74	19.17	40.99	
	LUS210-2	18	31/8	9	2	6	8-16d	6-16d	2580 11.48	4500 20.02	2320 10.32	3195 14.21	
ŀ					_	_			4745	9660	4310	7000	
1	HHUS210-2	14	35/16	9¾16	3	8	30-16d	10-16d	21.11	42.97	19.17	31.14	
	HGUS210-2	12	35/16	93/16	4	81/8	46-16d	16-16d	6840	14645	4855	10400	
ŀ							TDIE	PLE 2x SIZI	30.43	65.14	21.60	46.26	
ŀ								-	4385	8950	3110	6355	
	HGUS26-3	12	415/16	5½	4	41/8	20-16d	8-16d	19.51	39.81	13.83	28.27	
	HGUS28-3	12	415/16	71/4	4	6%	36-16d	12-16d	6070	12980	4310	9215	
-									27.00 4745	57.74 10545	19.17 4310	40.99 7485	
	HHUS210-3	14	411/16	9	3	715/16	30-16d	10-16d	21.11	46.91	19.17	33.29	
	HGUS210-3	12	415/16	91/4	4	8%	46-16d	16-16d	6840	14645	4855	10400	
								RUPLE 2x S	30.43	65.14	21.60	46.26	
ŀ									4385	8950	3110	6355	
	HGUS26-4	12	6%16	57/16	4	41/8	20-16d	8-16d	19.51	39.81	13.83	28.27	
	HGUS28-4	12	6%16	73/16	4	61/8	36-16d	12-16d	6070	12980	4310	9215	
-									27.00 4745	57.74 10545	19.17 4310	40.99 7485	
	HHUS210-4	14	61/8	87/8	3	713/16	30-16d	10-16d	21.11	46.91	19.17	33.29	
	HGUS210-4	12	6%16	93/16	4	81/8	46-16d	16-16d	6840	14645	4855	10400	
					-				30.43 7640	65.14 14995	21.60 5425	46.26 10645	
	HGUS212-4	12	6%16	10%	4	101/8	56-16d	20-16d	33.98	66.70	24.13	47.35	
	HGUS214-4	12	6%16	125/8	4	1111//8	66-16d	22-16d	10130	16400	7195	11645	
	1100021111		0710	1270		1170			45.06	72.95	32.00	51.80	
-								4x SIZES	1720	2595	1545	1920	
	LUS46	18	3%16	43/4	2	37/16	4-16d	4-16d	7.65	11.54	6.87	8.54	
	HHUS46	14	35/8	51/4	3	315/16	14-16d	6-16d	2540	7335	2065	5205	
			0,0	0,4		710			11.30 4385	32.63 8950	9.20 3110	23.15 6355	
	HGUS46	12	35/8	51/4	4	41/16	20-16d	8-16d	19.51	39.81	13.83	28.27	
	LUS48	18	3%16	6¾	2	37/16	6-16d	4-16d	1720	3325	1545	2575	
'	200-10	10	0/10	074		0/10	0 100	- 10u	7.65	14.79 8940	6.87 2675	11.45 6345	
	HHUS48	14	35/8	71/8	3	61/8	22-16d	8-16d	3765 16.75	39.77	11.90	28.22	
r	HGUS48	12	35/8	71/16	4	61/16	36-16d	12-16d	6070	12980	4310	9215	
	1100010	12	U/0	1 / 10	-T	0/10	30 100	12 100	27.00	57.74	19.17	40.99	
	LUS410	18	3%16	8¾	2	5%16	8-16d	6-16d	2580 11.48	4500 20.02	2320 10.32	3195 14.21	
	HGUS410	12	35/8	9	4	81/16	46-16d	16-16d	6840	14645	4855	10400	
	11000410	12	J/8	9	4	U / 16	70-10u	10-100	30.43	65.14	21.60	46.26	
	HGUS412	12	35/8	107/16	4	101/16	56-16d	20-16d	7640 33.98	14995 66.70	5425 24.13	10645 47.35	
h	HGUS414	12	35/	1274.	A	1/.	66-16d	22-16d	10130	16400	7195	11645	
	11000414	12	37/8	127/16	TA TA	716	00-100	24 IUU	45.06	72 05	32 00	51.80	



Typical HUS26 Installation (Truss Designer to provide fastener quantity for connecting multiple members together)



Double Shear Nailing Top View



Double Shear Nailing Side View Do not bend tab back



Dome Double Shear Nailing prevents tabs breaking off (available on some models)

U.S. Patent 5,603,580

- Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
- Designer must ensure that hanger is compatible with truss when reduced heel height is used.
- 3. de is the distance from the bearing seat to the top joist nail.
- Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-N10FORTRUSS and/or see installation notes.
 NAILS: 16d = 0.162" dia. x 3½" long.
- NAILS: 16d = 0.162" dia. x 3½" long.
 See pages 22-23 for other nail sizes and information.

The HTU face mount truss hangers have

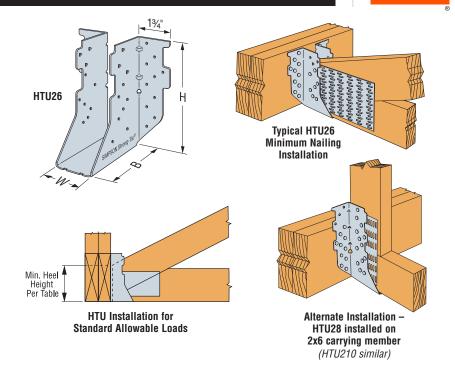
nail patterns designed specifically for shallow heel heights, so that full factored resistances (with minimum nailing) apply to heel heights as low as 31/8". Minimum and maximum nailing options provide solutions for varying heel heights and end conditions. MATERIAL: 16 gauge

FINISH: Galvanized INSTALLATION:

- · Use all specified fasteners. See General Notes.
- · Can be installed filling round holes only, or filling round and triangle holes for maximum values.
- See alternate installation for applications using the HTU26 on a 2x4 carrying member or HTU28 or HTU210 on a 2x6 carrying member for additional uplift capacity.

OPTIONS:

• See Hanger Options on page 230 for skew options.



Standard Installation

		Di	mensio	ns	Eac	teners		D.Fir-L	Resistance	
	Min.		(in)		газ	ICHCI S	D.F	ir-L	S-I	P-F
Model	Heel						Uplift	Normal	Uplift	Normal
No.	Height	w	н	В	Header	Joist	(K _D = 1.15)	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
	(in)	VV	п	D	пеацег	Juist	lbs	lbs	lbs	lbs
							kN	kN	kN	kN
		,			SINGL	E 2x SIZES				
HTU26	21/	15/6	57/16	3½	20-16d	11-10dx1½	1370	4990	975	3145
П1020	3½	17/6	3 ¹ /16	3 1/2	20-100	11-10ux1½	6.09	22.20	4.34	13.99
HTU26 (Min)	31/8	1%	57/16	3½	20-16d	14-10dx1½	2350	5240	1670	3300
HTUZO (WIIII)	378	176	J716	372	20-10u	14-10ux 1 72	10.45	23.31	7.43	14.68
HTU26 (Max)	5½	1%	57/16	3½	20-16d	20-10dx1½	2925	6565	2075	4660
111020 (IVIAX)	372	176	J /16	J /2	20-10u	20-100X172	13.01	29.20	9.23	20.73
HTU28 (Min)	37/8	1%	71/16	3½	26-16d	14-10dx1½	2325	6380	1650	4530
111020 (WIIII)	J/8	176	1 / 16	J /2	20-10u	14-10UX172	10.34	28.38	7.34	20.15
HTU28 (Max)	71/4	15/6	71/16	31/2	26-16d	26-10dx1½	4035	8900	2865	6320
111020 (Max)	174	170	1710	072	20 100	20 100/172	17.95	39.59	12.74	28.11
HTU210 (Min)	3%	15/6	91/16	3½	32-16d	14-10dx1½	2510	7135	1780	5065
1110210 (11111)	078	170	3710	072	02 100	14 100/172	11.17	31.74	7.92	22.53
HTU210 (Max)	91/4	15/6	91/16	3½	32-16d	32-10dx1½	6245	9820	4435	6970
TITOL TO (Max)	074	170	0710	072			27.78	43.68	19.73	31.00
		,			DOUB	LE 2x SIZES				
HTU26-2 (Min)	3%	35/16	57/16	3½	20-16d	14-10d	2430	6275	1725	4035
111020 2 (Willi)	078	0710	0710	072	20 100	14 100	10.81	27.91	7.67	17.95
HTU26-2 (Max)	5½	35/16	57/16	3½	20-16d	20-10d	3495	7195	2480	5110
	0,2	07.10	0710	0,,,	20 .00	20 .00	15.55	32.00	11.03	22.73
HTU28-2 (Min)	37/8	35/16	71/16	3½	26-16d	14-10d	2460	6920	1745	4915
	075	07.10	17.0	0,,,	20 .00		10.94	30.78	7.76	21.86
HTU28-2 (Max)	71/4	35/16	71/16	3½	26-16d	26-10d	5590	9790	3970	6950
,							24.87	43.55	17.66	30.92
HTU210-2 (Min)	37/8	35/16	91/16	3½	32-16d	14-10d	2470	7730	1755	5490
, ,							10.99	34.38	7.81	24.42
HTU210-2 (Max)	Max) 91/4	35/16	91/16	3½	32-16d	32-10d	7585	11955	5385	8490
, ,		1 1	/\ A	/ 3 /			33.74	53.18	23.95	37.77

- 1. Minimum heel heights required for tabulated values are based on a minimum 2:12 roof pitch.
- 2. Factored uplift resistances has been increased 15% for wind or earthquake: reduce where other loads
- govern. 3. NAILS:

16d = 0.162" dia. x 3½" long, 10d = 0.148" dia. x 3" long, 10dx1½ = 0.148" dia. x 1½" long. See pages 22-23 for other nail sizes and information.

HTU Face Mount Truss Hangers



Alternate Installation for 2-2x4 and 2-2x6 Headers

			Fas	teners		Factored F	Resistance	
	Min				D.F	ir-L	S-I	P-F
Model	Min. Heel	Minimum Header			Uplift	Normal	Uplift	Normal
No.	Height	Size	Header	Joist	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
	(in)				lbs	lbs	lbs	lbs
					kN	kN	kN	kN
HTU26 (Min)	37/8	2-2x4	10-16d	14-10dx1½	1740	3340	1235	2370
HTUZU (MIII)	378	2-284	10-16u	14-10ux 1 /2	7.74	14.86	5.49	10.54
HTU26 (Max)	5½	2-2x4	10-16d	20-10dx1½	2470	4015	1755	2850
HTUZO (Max)	372	2-284	10-16u	20-10ux 1 /2	10.99	17.86	7.81	12.68
HTU28 (Max)	97/	2.276	20-16d	26-10dx1½	4150	6395	2945	4540
HTUZO (Max)	31/8	2-2x6	20-10u	20-10ux 1 /2	18.46	28.45	13.10	20.19
HTU210 (Max)	71/.	2-2x6	20-16d	32-10dx1½	4150	6395	2945	4540
1110210 (IVIAX)	71/4	2-280	20-10u	32-10UX172	18.46	28.45	13.10	20.19

See table footnotes on page 156.

THAR/L422 Adjustable Truss Hangers

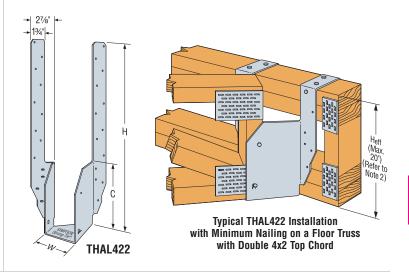
Designed for 4x2 floor trusses and 4x beams, the THAR/L422 has a standard skew of 45 degrees. Straps must be bent for top flange installation. PAN nailing helps eliminate splitting of 4x2 truss bottom chords.

MATERIAL: 16 gauge FINISH: Galvanized

Catalogue C-C-CAN2015 © 2015 SIMPSON STRONG-TIE COMPANY INC.

INSTALLATION: Use all specified fasteners. See General Notes. Two different installation methods may be used:

- Maximum Nailing—A minimum of four top and 12 face nails must be used. Straps must be field-formed over the header a minimum of $2\frac{1}{2}$ ". Install $10dx1\frac{1}{2}$ " nails into carried member PAN nail holes and 10d common nail into round nail hole. Install 10d common nails into carrying member.
- Minimum Nailing—A minimum of four top and 2 face nails must be used. Straps must be field-formed over the header a minimum of 2½". Install nails as detailed above. For single 4x carrying members, use 10dx1½" nails and refer to the table for reduced values.



	Din	nensio	ns				Faste	eners			Factored F	Resistance	
		(in)			Effective	Hea	ıder	Jo	ist	D.F	ir-L	S-I	P-F
Model				Minimum Carrying	Height					Uplift	Normal	Uplift	Normal
No.	w	н	С	Member	H _{eff} (in)	Top	Face	Straight	Slant	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
	vv	"	U		(111)	TOP	raut	Straight	Siaiit	lbs	lbs	lbs	lbs
										kN	kN	kN	kN
				Cinalo Av2	9 min.	4-10dx1½	2-10dx1½	1-10dx1½	2-10dx1½	_	1445		1025
				Single 4x2	9 111111.	4-10ux172	Z-100X172	1-10ux 1 72	Z-10UX 1 72	_	6.44	_	4.56
THAR/L422 (Min)	3%	225/8	Ω		9 to 12	4-10d	2-10d	1-10d	2-10dx1½	_	2215	_	1575
TTIATI/L422 (WIIII)	J /8	22/8		Double 4x2	3 10 12	4-10u	Z-10u	1-100	Z-10ux172	_	9.87	_	7.01
				Double 4X2	. 10	1-10d	2-10d	1-10d	2-10dx1½	_	1695	_	1205
THAR/L422 (Max)					> 12	4-10d	2-10u	1-100	Z-10UX172	_	7.55	_	5.36
	35%	22 5/8		Double 4x2	9 min.	4-10d	8-10d	1-10d	2-10dx1½	585	2585	415	1835
111A11/L422 (Wax)	J /8		0		3 111111.	4-100	0-100	1-100	Z-10ux172	2.61	11.51	1.85	8.16

- 1. Factored uplift resistances have been increased 15% for wind or earthquake loading with no further increase allowed.
- Where the top of the carried member is flush with the top of the carrying member, H_{eff} is equal to the depth of the carried member.
 Otherwise, H_{eff} shall be measured from the top of the bearing seat to the top of the carrying member.
- 3. NAILS: 10d = 0.148" dia. x 3 long, 70 lx1 ½ = 0.148" dia. x 1½ long. She larges 22-23 for other hall sizes and information.

or a combination of these features.

WIMEERED This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost,

The THA series have extra long straps that can be field-formed to give height adjustability and top flange hanger convenience. THA hangers can be installed as top flange or face mount hangers.

The THA218-2, THA222-2, THA418, THA422, and THA426 models have added nail holes in the straps to ease top-flange installation and provide more nail hole options for meeting top and face nailing requirements.

MATERIAL: See table

FINISH: Galvanized. Some products available in ZMAX® coating; see Corrosion Information, pages 14-17.

INSTALLATION: • Use all specified fasteners. See General Notes.

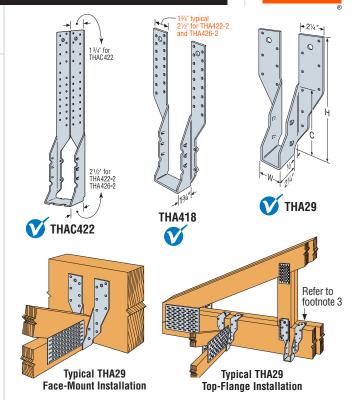
Two different installation methods may be used:

• Top-Flange Installation—The straps must be field formed over the header a minimum of 2½" for the THA29, 1½" for the THA213 and THA413, and 2" for all others. Install top and face nails according to the table. Top nails shall not be within 1/4" from the edge of the top flange members.

For the THA29, nails used for joist attachment must be driven at an angle so that they penetrate through the corner of the joist and into the header. For all other top-flange installations, straighten the double shear nailing tabs and install the nails straight into the joist.

- Face-Mount Installation—Install all face nails according to the table. Not all nail holes will be filled on all models. On models where there are more nail holes than required, the lowest 4 face holes must be filled. Nails used for the joist attachment must be driven at an angle so that they penetrate through the corner of the joist into the header.
- Uplift—Lowest face nails must be filled to achieve uplift resistances

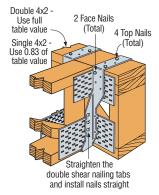
OPTIONS: • THA hangers available with the header flanges turned in for 35%" (except THA413) and larger, with no load reduction – order THAC hanger.



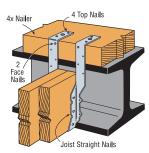
			Di	mensio	ns		Fas	teners			Factored F	Resistance	
				(in)		Hea	ader	Jois	t	D.F	ir-L	S-I	P-F
Min.	Model	Ga								Uplift	Normal	Uplift	Normal
Joist Size	No.	uа	w	н	C	Ton	Face	Ctroight	Slant	(K _D =1.15)	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
			VV	п	U	Тор	гасе	Straight	Siaiit	lbs	lbs	lbs	lbs
										kN	kN	kN	kN
						TOP	-FLANG	E INSTALL <i>a</i>	TION				
2x4	THA29	18	15/8	911/16	51/8	4-10d	4-10d		4-10d	1050	3450	750	2720
2.84	ITAZ9	10	178	9.716	378	4-10u	4-10u		4-10u	4.67	15.35	3.34	12.10
	THA213	18	15%	135/16	5½	4-10d	2-10d	4-10dx1½		_	2225	_	1760
2x6	IIIAZIO	10	1 /8	10/16	J /2	4-10u	Z-10u	4-10UX172		_	9.90	_	7.83
2.00	THA218	18	15/8	173/16	5½	4-10d	2-10d	4-10dx1½	_		2225	_	1760
	11171210		170	17710	072	1 100		1 100/172			9.90	_	7.83
	THA218-2	16	31/8	1711/16	8	4-16d	2-16d	6-16dx2½	_		2675	_	2405
2-2×10				,				O TOURE 72			11.90	_	10.70
	THA222-2	16	31/8	223/16	8	4-16d	2-16d	6-16dx2½	_		2675	_	2405
											11.90	_	10.70
4x6	THA413	18	35/8	135/16	41/2	4-10d	2-10d	4-10d	_	_	2225	_	1655
											9.90	_	7.36
4x10	THA418	16	35/8	17½	71//8	4-16d	2-16d	6-16d	_	_	2675 11.90	_	2405 10.70
											2675		2405
4x2	THA422	16	35/8	22	71/8	4-16d	2-16d	6-16d	_		11.90		10.70
Truss											3590		2660
	THA426	14	35/8	26	71/8	4-16d	4-16d	6-16d	_	_	15.97	_	11.83
											4605	_	3225
2-4x2	THA422-2	14	71/4	2211/16	9¾	4-16d	4-16d	6d 6-16d	_	_	20.48	_	14.35
Truss	TUA 400 C	4.4	71/	001/	02/	4.40.1	4.40.1	0.40.4		_	4605	_	3225
	THA426-2	14	71/4	261/16	9¾	4-16d	4-16d	6-16d		_	20.48	_	14.35

- 1. Factored uplift resistances have been increased 15% for earthquake or wind loading with no further increase allowed: reduce for other load durations as required by code.
- For single 4x2 top chord carrying members, THA 4x hangers can be used with 10dx11/2" nails and a reduced resistance to 0.83 of the table value. Values are based on hanger installations at panel points.
- 3. For the THA2X models, one strap may be installed vertically according to the face mount nailing requirements and the other strap wrapped over the top chord according to the top flange nailing requirements (see drawing above)





Typical THA422 Top-Flange Installation on a 4x2 Floor Truss



Typical THA Top-Flange Nailing Configuration on a 4x Nailer (except THA29)

Catalogue C-C-CAN2015 © 2015 SIMPSON STRONG-TIE COMPANY INC

THA/THAC Adjustable Truss Hangers



		Dimensions (in)	ne		Fas	teners			Factored F	Resistance			
			וט		1113	He	ader	Jois	t	D.F		S-I	P-F
Min.	Model					- 110		0010	Ì	Uplift	Normal	Uplift .	Normal
Joist	No.	Ga								(K _D =1.15)	(K _D =1.00)	(K _D =1.15)	(K _D =1.00)
Size			W	Н	C	Top	Face	Straight	Slant	lbs	lbs	lbs	lbs
										kN	kN	kN	kN
						FAC	E-MOUN	T INSTALL <i>i</i>	TION				
04	TUAGO	10	45/	011/	F1/		10 104		4 404	1050	3440	750	2455
2x4	THA29	18	15/8	911/16	51/8	_	16-10d	_	4-10d	4.67	15.30	3.34	10.92
	THA213	18	15/8	135/16	5½		14-10d		4-10d	1420	2785	1290	2210
2x6	ITAZIS	10	178	13916	J 72		14-100	_	4-10u	6.32	12.39	5.74	9.83
2.00	THA218	18	15/8	173/16	5½		18-10d		4-10d	1420	2785	1290	2210
	ITIAZIO	10	178	17 716	J 72		10-100		4-10u	6.32	12.39	5.74	9.83
	THA218-2	16	31/8	1711/16	8	_	16-16d	_	6-16d	2540	4765	1805	3385
2-2x10	TITAZ TO Z	10	070	17 716			10 100		0 100	11.30	21.20	8.03	15.06
L LXIO	THA222-2	16	31/8	223/16	8	_	22-16d	_	6-16d	2540	5550	1805	4150
	111712222	10	070	LL710					0 100	11.30	24.69	8.03	18.46
4x6	THA413	18	35/8	135/16	41/2		14-10d	_	4-10d	1420	3555	1290	2525
			0,0	107.10	172					6.32	15.81	5.74	11.23
4x10	THA418	16	35/8	171/2	77/8	_	16-16d	_	6-16d	2540	4765	1805	3385
										11.30	21.20	8.03	15.06
	THA422	16	35/8	22	71/8	_	22-16d	_	6-16d	2540	5850	1805	4150
4x2										11.30	26.02	8.03	18.46
Truss	THA426	14	35/8	26	71/8	_	30-16d	_	6-16d	2540	6295	1805	4545
										11.30	28.00	8.03	20.22
0.40	THA422-2	14	71/4	2211/16	93/4	_	30-16d	_	6-16d	2845 12.66	7715 34.32	2585	5475 24.35
2-4x2 Truss										2845	7715	11.50 2585	5475
11433	THA426-2	14	71/4	261/16	9¾	_	38-16d	6d — (— 6-16d	12.66	34.32	11.50	24.35
										12.00	34.32	11.30	24.33



Double Shear Nailing Top View



Double Shear Nailing Side View Do not bend tab back



Dome Double Shear Nailing prevents tabs breaking off (available on some models)

U.S. Patent 5,603,580

See footnotes on page 158.

THJM Multiple Truss Hip Jack Hanger

The new THJM is a non-welded hanger designed to carry radial-end jack framing and provide optimal efficiency for those multi-plane, angled bay roofs over breakfast, study and library alcoves. The unique patent pending design of the THJM accommodates 2x6 girder bottom chords and uses our Strong-Drive® SDS Heavy-Duty Connector screws for easy installation with minimal fasteners.

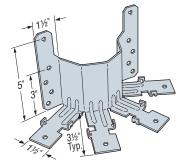
FEATURES:

- The THJM hangers are designed for installation with 1/4"x3" Strong-Drive SDS Heavy-Duty Connector screws that are included with the parts.
- The THJM2-4-SDS3 is designed for four incoming jack trusses with the outer jacks being 22½° from the face of the girder and the inner jacks being 45° from each other and the outer jacks.
- The THJM2-5-SDS3 is designed for five jacks coming into the hanger at 30° from the girder and each other.
- Tabs on the seats of the THJM assist in the placement of the jacks.

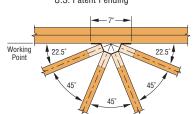
MATERIAL: 12 gauge FINISH: Galvanized

INSTALLATION: • Use all specified fasteners. See General Notes.

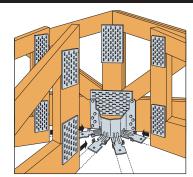
- Each carried jack truss requires one 1/4"x3" Strong-Drive SDS Heavy-Duty Connector screw installed into the bottom chord through the bottom of the hanger seat.
- · Fill all round and triangular holes.
- Strong-Drive® SDS Heavy-Duty Connector screws driven through truss plates must be approved by the Truss Designer. Pre-drilling using a 5/32" bit is required.



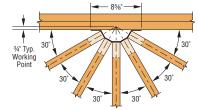
THJM2-4-SDS3 (THJM2-5-SDS3 similar) U.S. Patent Pending



THJM2-4-SDS3 **Top View Installation**



Typical THJM Installation



THJM2-5-SDS3 **Top View Installation**

	Faste	nore		Factored F	Resistance		1
	Газів	ille18	D.F	ir-L	S-I	P-F	
Model			Uplift	Normal	Uplift	Normal	
No.	Header	Joist	(K _D =1.15)	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$	2
	пеацеі	(Total)	lbs	lbs	lbs	lbs	
			kN	kN	kN	kN	
THJM2-4-SDS3	10 1/"v2" CDC	4-1/4"x3" SDS	890	4565	640	3290	3
1 111112-4-3033	10-1/4"x3" SDS	4-74 X3 3D3	3.96	20.31	2.85	14.64	_
TILIMO E CDCO	10-¼ x3/SDS	Ar // alore	970	5250	700	3905	
THJM2-5-SDS3		5-1/4 x3 SDS	4.31	23.35	3.11	17.37	

- Factored resistances shown are for all carried members combined. The load on any single member shall not exceed 25% of the tabulated factored resistace for THJM2-4 or 20% for THJM2-5.
- Factored uplift resistances are only applicable to short term load duration. This connector cannot be used to resist uplift due to other load durations (for example: cantilever construction).
- A minimum 2-ply header is required to achieve
- the factored resistances shown.
 For single-ply headers, use ½"x1½" Strong-Drive® y Connector screw into the header tayulated normal resistances x 0.80. uplift resistances still apply.

THASR/L Adjustable/Skewable Truss Hangers



The THASR/L hangers combine the height adjustability of THA hangers with field skewability, offering maximum flexibility for the installer and eliminating the need for special orders. Shipped at 22½° right or left, the THASR/L hangers can be field skewed from 22½° to 75°.

The new THASR/L29, 29-2 and 422 are replacing the former 218, 218-2 and 418 versions.

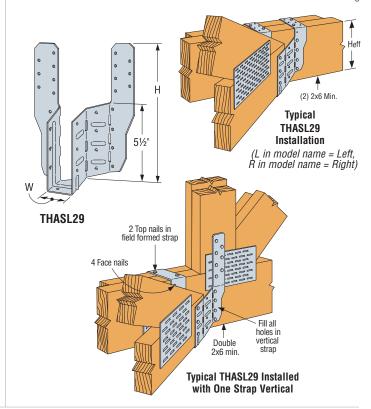
FEATURES:

- The new THASR/L single and two-ply versions have straps
 g" tall. The 4x version has 22" straps to fit more parallel-chord truss applications.
- The new versions have only one acute side bend line to ease design and installation.
- Joist fasteners are only required from one side for skews greater than 22½°.
- Rated for installation with either nails or Simpson Strong-Tie[®] Strong-Drive[®] SD Connector screws.

MATERIAL: 16 gauge FINISH: Galvanized

INSTALLATION: • Use all specified fasteners. See General Notes.

- Product is factory skewed to 22½° and may be field skewed from 22½° to 75°. See Installation Sequence below for skews greater than 22½°.
- For 22½° skew installations, fill all triangle holes. Triangle holes do not need to be filled for skews greater than 22½°.
- For all installations, fill the fastener hole(s) in the bottom of the hanger seat (THASR/L29 has one and all other models have two).
- For top-flange installations, the straps must be field-formed over the header a minimum of 2".
- THASR/L29 and THASR/L29-2—For installations where either strap cannot be field-formed over the header, install the strap(s) vertical and fill all holes. Capacities must be reduced as noted in the table footnotes.
- THASR/L422—For face-mount installations, install the carrying member fasteners into the lowest holes.



These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details. These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

			Dime	nsions				Fasten	ers		Factored F	Resistance	
			(i	n)			Hea	ider		D.F	ir-L	S-I	P-F
	Min	Model			Min.	Skew				Uplift	Normal	Uplift	Normal
	Carried Member	No.		١	H _{eff} (in)	Angle (degrees)	_	_	Joist	$(K_D = 1.15)$	$(K_D = 1.00)$	(K _D =1.15)	$(K_D = 1.00)$
	Michigor		W	Н	(111)	(ucgrees)	Top	Face		lbs	lbs	lbs	lbs
										kN	kN	kN	kN
							TOP-FL	ANGE INST	ALLATION				
										1315	2850	935	2020
						22½	4-10d	8-10d	7-10dx1½	5.85	12.68	4.16	8.99
	2x	T11400 // 00	457	07/	F4./	00 : 45	4.40.1	0.401	4.40.1.447	635	2145	450	1520
	Truss	THASR/L29	15/8	9%	5½	23 to 45	4-10d	8-10d	4-10dx1½	2.82	9.54	2.00	6.76
						40 to 75	4 404	0.404	4.40441/	590	2145	420	1520
						46 to 75	4-10d	8-10d	4-10dx1½	2.62	9.54	1.87	6.76
						22½	4-10d	8-10d	8-10d	1360	2380	965	1690
	2-2x	THASR/L29-2				ZZ72	4-10u	0-10u	0-10u	6.05	10.59	4.29	7.52
			31/8	97/8	51/2	23 to 45	4-10d	8-10d	5-10d	425	1870	300	1325
	Truss		378	3/8	372	20 10 40	4-10u	0-10u	J-10u	1.89	8.32	1.33	5.89
						46 to 75	4-10d	8-10d	5-10d	375	1870	270	1325
						40 10 70	- 10u	0 100	0 100	1.67	8.32	1.20	5.89
						22½	4-10d	4-10d	8-10d		1605	_	1140
									0.00		7.14		5.07
	4x	THASR/L422	35/8	22	8	23 to 45	4-10d	4-10d	5-10d		1345		955
	Truss										5.98	_	4.25
						46 to 75	4-10d	4-10d	5-10d		1080	_	770
							E40E N	OUNT MAT	ALL ATION	_	4.80	_	3.43
							FACE-IV	IOUNT INST	ALLATION		4450		
						221/2	_	8-10d	8-10d		1170		830
											5.20		3.69
	4x Truce	THASR/L422	35/8	22	5½	23 to 45	_	8-10d	5-10d	_	1050 4.67	_	745
	Truss				5½						1050		3.31 745
						46 to 75	_	8-10d	5-10d		4.67	_	3.31
Į											4.07		3.31

Uplift resistances have been increased 15% for wind or earthquake loading with no further increase permitted. Reduce where other load durations govern.

2. Minimum carried truss (joist) heel height shall be 41/2".

 H_{eff} is the distance from the top of the hanger bearing seat to the top of the carried member (header). header a minimum of 2". Factored download resistances for the THASR/L29 and THASR/L29-2 with one or both straps installed vertically (with all holes filled) are 86% of the tabulated values. Factored uplift resistances are 100%

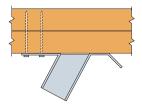
of the tabulated values.

5. NALS: 10d = 0.148" dia. x 3" ong. 10d x 1½ = 0. 48" dia. x 1½" long. See pages 22 23 for other nail sizes and information.

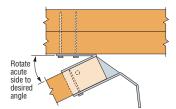
THASR/L Adjustable/Skewable Truss Hangers



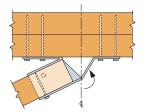
INSTALLATION SEQUENCE FOR SKEWS > 22½°



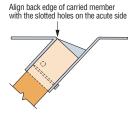
Step 1: Install acute side top and/or face header fasteners.



Step 2: Utilizing a piece of scrap fastened to the hanger *(on obtuse side only)*, bend the hanger along the acute side bend line to the desired angle.



Step 3: Bend the obtuse side of the hanger back toward the header until the narrow nailing flange lies flat against the header, and install obtuse side header top and/or face fasteners.



Step 4: Install joist/truss and install the carried member fasteners on the obtuse side and seat only.

For 22½° skew installations fill all triangle holes.

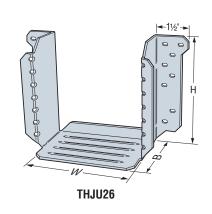
THJU Truss Hip/Jack Hanger

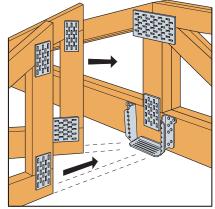
The THJU hip/jack hanger offers the most flexibility and ease of installation without sacrificing performance. The U-shaped hanger works for right and left hand hips and can be ordered to fit a range of hip skews (up to 67½ degrees) as well as various single and 2-ply hip/jack combinations. Also can be installed before or after the hip and jack.

THJU26 is sized for the standard hip/jack combination with a 45-degree left or right-hand hip. The wide seat of THJU26-W accommodates a 2-ply hip and 2-ply jack combination with a 45 degree maximum hip skew, or a standard single-ply hip/jack configuration with a maximum 67½-degree hip skew. Intermediate seat widths are available for other hip/jack or hip/hip combinations.

MATERIAL: 12 gauge FINISH: Galvanized INSTALLATION: • Use all specified fasteners. See General Notes.

OPTIONS: Other seat widths available. See Hanger Options on page 230 for more information.



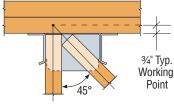


Typical THJU26 Installation

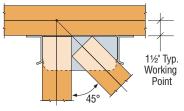
		Dir	nensi	ons		asteners			Factored F	Resistance	
	Min.		(in)			astellela	•	D.F	ir-L	S-I	P-F
Model	Heel							Uplift	Normal	Uplift	Normal
No.	Height	w	н	В	Header	Hip	Jack	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
	(in)	VV	п п	D	пеацеі	пір	Jack	lbs	lbs	lbs	lbs
								kN	kN	kN	kN
	3½				16-10d	4-10d	4-10d	1045	2675	745	1915
THJU26		E1/	5%	21/		4-10u	4-10u	4.65	11.90	3.31	8.52
111020	514	51/8	378	3½		7-10d	7-10d	1825	3280	1310	2350
	5½						/-10u	8.12	14.59	5.83	10.45
	21/		5%	3½	16 104	4 104	4-10d	990	2550	705	1825
THJU26-W	3½	Q 1/.			16-10d	4-10d	4-10u	4.40	11.34	3.14	8.12
	5½	81/8			16-10d	7-10d	7-10d	1730	2550	1240	1825
					10-100	7-10u	7-10u	7.70	11.34	5.52	8.12

- For full capacity, the jack requires either a min. 2x6 bottom chord or a min. 2x4 end vertical; the hip requires either a min. 2x6 bottom chord or a min. 2x6 end vertical for hip skews up to 60°. For hip skews greater than 60° (THJU26-W only), a min. 2x6 bottom chord or min. 2x8 end vertical is required.
- 2. Tabulated values are the total factored loads of the hip and jack members combined; 65%-85% of the total load shall be distributed to the hip member, and the remaining percentage of total load shall be distributed to the jack. The combined hip and jack load may not exceed the total factored resistances.
- 3. Factored uplift resistances have been increased 15% for wind or earthquake loading with no further increase permitted, reduce where other loads govern.
- For single 2x jacks, 10dx1½" nails may be substituted for the specified 10d commons with no reduction in capacity.
- 5. For single ply 2x headers use 10dx1½" nails into the header and multiply the tabulated factored resistances by 0.77.

 6. NAILS: 10d = 0.148" dia x 3" long. See pages 22-23 for



THJU26 Top View Right Hand Hip Installation



THJU26-W Top View 2-Ply Hip/2-Ply Jack Installation

LTHJA26/THJA26 Truss Hip/ Jack Hangers





This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

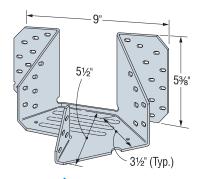
The LTHJA26 is the lighter capacity version of the THJA26. The LTHJA26 is designed for the common 8 foot hip girder setback. Consult with truss engineer or refer to truss engineering for actual demand load information.

MATERIAL: LTHJA26—18 gauge; THJA26—14 gauge

FINISH: Galvanized INSTALLATION:

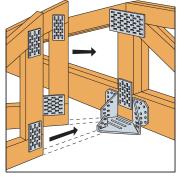
- Use all specified fasteners. See General Notes.
- All multiple members must be fastened together to act as a single unit.
- 10dx1½" nails must be installed into bottom of hip members through bottom of hanger seat for factored resistances (LTHJA26).

OPTIONS: These hangers can not be modified.

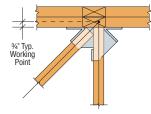




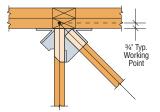
U.S. Patent 5,253,465 and other Patent Pending



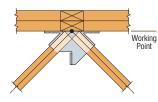








Top View Right Hand Hip Installation



Top View Terminal Hip without Center Common Jack

			Fasteners				Factored F	Resistance	
						D.F	ir-L	S-I	P-F
Model	Carried Member				Carried	Uplift	Normal	Uplift	Normal
No.	Combination	Carrying Member	Hip² (each)	Jack	Member	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
			(ouon)			lbs	lbs	lbs	lbs
						kN	kN	kN	kN
					Jack	120	400	85	285
					Jack	0.53	1.78	0.38	1.27
	Side Hip &	20-10d	7-10dx1½	4-10dx11/2	Hip	360	1205	260	860
	Center Jack	20-10u	1-10ux 172	4-10ux172	ΠΙΡ	1.60	5.37	1.16	3.83
LTHJA26					Total	480	1605	345	1145
LINJAZO					TULAI	2.14	7.15	1.54	5.10
					Hip (each)	550	1040	395	745
	Double	20-10d	7-10dx1½	_	The (each)	2.45	4.63	1.76	3.32
	(Terminal Hip)	20-10u	7-10UX172		Total	1100	2080	790	1490
					Τοται	4.90	9.27	3.52	6.64
					Hip	1365	3810	960	2890
					ППР	6.08	16.97	4.28	12.87
	Side Hip &	20-16d	6-10dx11//	4-10dx11/2	Jack	455	1270	320	965
	Center Jack	20 100	0 100X172	4 100X172	oack	2.03	5.66	1.43	4.30
THJA26					Total	1820	5080	1280	3855
IIIIAZU					Total	8.11	22.63	5.70	17.17
					Hip (each)	910	2540	640	1925
	Double	20-16d	6-10dx1½		Trip (Gacil)	4.05	11.31	2.85	8.59
	(Terminal Hip)	20-10d	0 · 10 u x 1 /2		Total	1820	5080	1280	3850
		1 1	1 4 / 1	A/ T	Total	8.11	22.63	5.70	17.17
		VV	VVV	/V . I	KIL	الار		KU	22

- Factored uplift resistances have been increased 15% for earthquake or wind loading with no further increase allowed; reduce where other loads govern.
- 2. For LTHJA26, one 10dx1½" nail must be installed into bottom of each hip member through bottom of hanger seat.
- 3. With single 2x carrying members, use 10dx1½" nails and use 0.77 of the table value for LTHJA26 and 0.64 for THJA26.
- 4. Tabulated hip and jack allowable loads assume that 75% of the total load is distributed to the hip and 25% to the jack. It is permitted to distribute 65% to 85% of the tabulated total load to the hip, and the remaining percentage of total load to the jack. The combined hip and jack load may not exceed the published Total Load.
- 5. NAILS: 16d = 0.162" dia. x 3½" long, 10d = 0.148" dia. x 3" long, 10dx1½ = 0.148" dia. x 1½" long. See pages 22-23 101 other nail sizes and information.

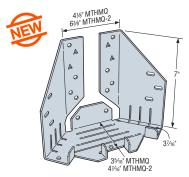
MTHMQ Multiple Truss Hangers

SIMPSON
Strong-Tie

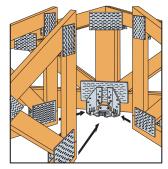
The MTHMQ and MTHMQ-2 are redesigned versions of our medium-to-high load capacity hangers for carrying 2 or 3 trusses. The new design offers concealed flanges and installs with Strong-Drive® SDS Heavy-Duty Connector screws for easier installation.

MATÉRIAL: 12 gauge FINISH: Galvanized (G90) INSTALLATION: • Use all specified fasteners.

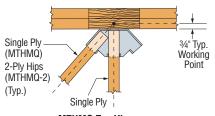
- MTHMQ-for 2x6 header do not fill upper four holes; for 2x8 header fill all holes.
- MTHMQ2-for 2x6 header do not fill upper six holes; for 2x8 header do not fill lower triangle holes.
- With single ply 2x carrying members, use 1/4"x11/2" Strong-Drive SDS Heavy-Duty Connector screws and reduce capacity x 0.68.
- When Strong-Drive SDS Heavy-Duty Connector screws are installed through metal truss plates pre-drilling using a ½ bit is required.
- All multiple members must be fastened together to as one unit.



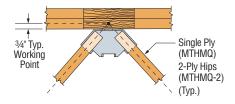




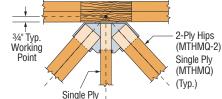
Typical MTHMQ Min. Installation at Panel Point



MTHMQ Top View Left Hand Hip Installation (MTHMQ-2 similar)



MTHMQ Top View Terminal Installation without Center Common Jack (MTHMQ-2 similar)



MTHMQ-2 Top View Terminal Installation with Center Common Jack (MTHMQ similar)

Right or Left Hand Hip Installation (Two-Member Connection)

			Fasteners						F	actored F	Resistan	e					
			rastellers				D.F	ir-L					S-I	P-F			
Model	Header				Upl	ift (K _D =1	.15)	Dov	/n (K _D =1	.00)	Upl	ift (K _D =1	.15)	Dow	/n (K _D =1	.00)	
No.	пеацеі	Carrying	Hip	Jack	Hip	Jack	Total	Hip	Jack	Total	Hip	Jack	Total	Hip	Jack	Total	
		Member	пір	Jack	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	
					kN	kN	kN	kN	kN	kN	kN	kN	kN	kN	kN	kN	
	2-ply 2x6	10-1/4"x3"	4-1/4"x3"	1-1/4"x3"	785	265	1050	3075	1025	4100	565	190	755	2215	735	2950	
MTHMQ	Z-piy Zxo	SDS	SDS	SDS	3.49	1.18	4.67	13.68	4.56	18.24	2.51	0.85	3.36	9.85	3.27	13.12	
IVITITIVIQ	2-ply 2x8	14-1/4"x3"	4-1/4"x3"	1-1/4"x3"	785	265	1050	4245	1415	5660	565	190	755	3055	1020	4075	
	2-piy 2x0	SDS	SDS	SDS	3.49	1.18	4.67	18.88	6.29	25.18	2.51	0.85	3.36	13.59	4.54	18.13	
	2-ply 2x6	10-1/4"x3"	5-1/4"x3"	1-1/4"x3"	1255	415	1670	3785	1265	5050	900	300	1200	2725	910	3635	
MTHMQ-2	Z-piy Zxo	SDS	SDS	SDS	5.58	1.85	7.43	16.84	5.63	22.46	4.00	1.33	5.34	12.12	4.05	16.17	
WITHINQ-2	2-ply 2x8		5-1/4"x3"		1-1/4"x3"	1255	415	1670	4375	1460	5835	900	300	1200	3150	1050	4200
	2-hiy 2xo	SDS	SDS	SDS	5.58	1.85	7.43	19.46	6.49	25.96	4.00	1.33	5.34	14.01	4.67	18.68	

Terminal Type Installation (Three-Member Connection)

			Fasteners						F	actored F	Resistano	e				
			rastellers				D.F	ir-L					S-I	P-F		
Model	Header				Upl	ift (K _D =1	.15)	Dow	/n (K _D =1	.00)	Upl	ift (KD=1	.15)	Dow	/n (K _D =1	.00)
No.	пеацег	Carrying	Hips	Jack	Hip	Jack	Total	Hip	Jack	Total	Hip	Jack	Total	Hip	Jack	Total
		Member	(Total)	Jack	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs
					kN	kN	kN	kN	kN	kN	kN	kN	kN	kN	kN	kN
	2-ply 2x6	10-1/4"x3"	8-1/4"x3"	1-1/4"x3"	775	390	1940	2295	1150	5740	560	275	1395	1655	825	4130
MTHMQ	2-piy 2x0	SDS	SDS	SDS	3.45	1.73	8.63	10.21	5.12	25.53	2.49	1.22	6.21	7.36	3.67	18.37
IVITIIVIQ	2-ply 2x8	14-1/4"x3"	8-1/4"x3"	1-1/4"x3"	775	390	1940	3025	1510	7560	560	275	1395	2175	1090	5440
	2-piy 2x0	SDS	SDS	SDS	3.45	1.73	8.63	13.46	6.72	33.63	2.49	1.22	6.21	9.68	4.85	24.20
	2-ply 2x6	10-1/4"x3"	10-1/4"x3"	1-1/4"x3"	1070	530	2670	2815	1410	7040	770	385	1925	2030	1010	5070
MTHMQ-2	2-piy 2x0	SDS	SDS	SDS	4.76	2.36	11.88	12.52	6.27	31.32	3.43	1.71	8.56	9.03	4.49	22.55
WITHINQ-2	2-ply 2x8	14-1/4"x3"	10-1/4"x3"	1-1/4"x3"	1070	530	2670	3635	1815	9085	770	385	1925	2615	1310	6540
	Z-piy ZXO	SDS	SDS	SDS	4.76	2.36	11.88	16.17	8.07	40.41	3.43	1.71	8.56	11.63	5.83	29.09

- Factored uplift resistances have been increased 15% for short term loading, no further increase is permitted.
- 2. A minimum 2-ply 2x carrying member is required for the tabulated resistances. With single ply 2x carrying members use ¼"x1½" Strong-Drive® SDS Heavy-Duty Connector screws into the carrying member and multiply the tabulated Down capacities x 0.68.
- 3. Tablulated Two-Member Connection capacities assume that 75% of the total load is distributed to the hip and 25% to the jack. It is permitted to distribute between 65% and 85% of the total load to the hip and the remaining load to the jack. The combined
- hip and jack loads may not exceed the total published factored resistances.

 4. For terminal hips with no centre jack, divide the total factored resistance by 2 to deterimine the factored resistance for each hip.
- 5. Tablulated Three-Member connection capacities assume that each hip carries 40% of the total load and the jack carries 20%. Other hip/jack load distributions are permitted if the sum of all three carried members does not exceed the total load and the hips are equally loaded.
- equally loaded.

 6. When Strong-Drive SDS Heavy-Duty Connector screws are installed through metal truss plates, the application must be approved by the Truss Designer. Pre-drilling using a %2" bit is required.
- 7. As per 12.2.2.5 CSA 086-14, the carrying member must be evaluated using a reduced cross sectional arra at the hanger location. The reduction in area shall be based on seven 7.9 ½" diameter loles on a 2x8 bottom cho'd and five (5) ¼" diameter holes on a 2x6 bottom cho'd.

HTHMQ Heavy Multiple Truss Hangers



The HTHMQ is a versatile, high-capacity truss hanger designed for various lumber types and multiple-ply trusses. The truss hanger accommodates a greater range of structural designs carry while accommodating right or left hand hips (at 30°- 60° skews), which can be used for terminal hips with or without the center common jack. The HTHMQ can accommodate various widths of lumber.

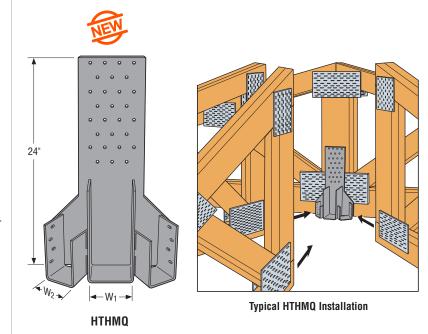
FEATURES:

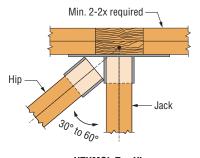
- Available in various stirrup widths to accommodate various lumber types and multiple ply trusses
- Installed with Simpson Strong-Tie® Strong-Drive® SDS Heavy-Duty Connector screws that eliminate the inconvenience of bolted installation
- Enables 2-3 member connection for a broader range of structural designs

MATERIAL: Back plate—3 gauge; stirrup—7 gauge **FINISH:** Simpson Strong-Tie® gray paint

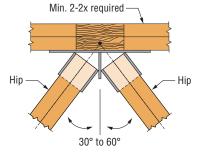
INSTALLATION: • Use all specified fasteners. See General Notes.

- Strong-Drive SDS Heavy-Duty Connector screws supplied for all round and obround holes.
- All multiple members must be fastened together to act as a single unit.
- Shall be attached to a minimum 2-ply girder truss.
- Girders must be adequately laterally braced to prevent excessive displacement due to secondary torsional stresses.
- Maximum girder bottom chord depth is 2x10.
- Must be installed centred on a minimum 2x8 girder vertical web.
- · See below for different installation options.

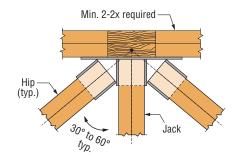




HTHMQL Top View Left Hand Hip Installation



HTHMQN Top View Terminal Hip Installation without Common Center Jack



HTHMQ Top View Terminal Installation with Center Common Jack

	Dimensi	ons (in)			Fasteners	
Model No.	W ₁	W ₂	Hip Skew Angle	Header	Hips (Total)	Jack
HTHMQ-SDS	15⁄8 - 4 ¹⁵ ⁄16	15⁄8	30°-60°	34-1/4"x3" SDS	8-1/4"x11/2" SDS	4-1/4"x11/2" SDS
HTHMQ-2-SDS	15⁄8 - 415⁄16	35/16	30°-60°	34-1/4"x3" SDS	8-1/4"x21/2" SDS	4-1/4"x11/2" SDS
HTHMQN-SDS	_	15⁄8	30°-60°	34-1/4"x3" SDS	8-1/4"x11/2" SDS	4-1/4"x11/2" SDS
HGHMQN-2-SDS	_	35/16	30°-60°	34-1/4"x3" SDS	8-1/4"x21/2" SDS	4-1/4"x11/2" SDS
HTHMQR/L-SDS	15⁄8 - 4 ¹⁵ ⁄16	15⁄8	30°-60°	34-1/4"x3" SDS	8-1/4"x11/2" SDS	4-1/4"x11/2" SDS
HTHMQR/L-2-SDS	15/8 - 415/16	35/16	30°-60°	34-1/4"x3" SDS	8-1/4"x21/2" SDS	4-1/4"x11/2" SDS

HTHMQ Heavy Multiple Truss Hangers



					Factor	ed Resistar	ice – Joist B	earing				
			D.F	ir-L					S-	P-F		
Model	Ur	olift (Kp=1.1	15)	Do	wn (K _D =1.0	10)	Up	lift (KD=1.1	15)	Do	wn (K _D =1.0	10)
No.	Hip (ea)	Jack	Total	Hip (ea)	Jack	Total	Hip (ea)	Jack	Total	Hip (ea)	Jack	Total
	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs
	kN	kN	kN	kN	kN	kN	kN	kN	kN	kN	kN	kN
HTHMQ-SDS	2005	1005	5015	5820	2910	14550	1445	720	3610	4735	2365	11835
TTTTIVIQ-3D3	8.92	4.47	22.31	25.89	12.94	64.72	6.43	3.20	16.06	21.06	10.52	52.65
HTHMQ-2-SDS	2005	1005	5015	6750	3375	16875	1445	720	3610	5845	2920	14610
HTHIVIQ-2-3D3	8.92	4.47	22.31	30.03	15.01	75.07	6.43	3.20	16.06	26.00	12.99	64.99
HTHMQN-SDS	1295	_	2590	5820	_	11640	935	_	1870	4735	_	9470
HTHIVIQIN-3D3	5.76	_	11.52	25.89	_	51.78	4.16	_	8.32	21.06	_	42.13
HGHMQN-2-SDS	1295	_	2590	6820	_	13640	935	_	1870	4910	_	9820
nuniwuw-2-3D3	5.76	_	11.52	30.34	_	60.68	4.16	_	8.32	21.84	_	43.68
HTHMQR/L-SDS	2140	715	2855	5820	1940	7760	2000	665	2665	4735	1580	6315
TITTIWIQN/L-3D3	9.52	3.18	12.70	25.89	8.63	34.52	8.90	2.96	11.85	21.06	7.03	28.09
HTHMQR/L-2-SDS	2715	905	3620	10140	3380	13520	2015	670	2685	8190	2730	10920
n i niviun/L-2-5D5	12.08	4.03	16.10	45.11	15.04	60.14	8.96	2.98	11.94	36.43	12.14	48.58

See footnotes below.

					Factored	Resistance	– End Grair	Bearing		-		
			D.F	ir-L					S-	P-F		
Model	Ur	lift (Kp=1.1	5)	Do	wn (K _D =1.0	10)	Up	lift (KD=1.1	5)	Do	wn (K _D =1.0	10)
No.	Hip (ea)	Jack	Total	Hip (ea)	Jack	Total	Hip (ea)	Jack	Total	Hip (ea)	Jack	Total
	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs
	kN	kN	kN	kN	kN	kN	kN	kN	kN	kN	kN	kN
HTHMQ-SDS	1070	535	2675	6750	3375	16875	1000	500	2500	5845	2920	14610
птпійіц-зиз	4.76	2.38	11.90	30.03	15.01	75.07	4.45	2.22	11.12	26.00	12.99	64.99
HTHMQ-2-SDS	1355	680	3390	6750	3375	16875	1265	630	3160	5845	2920	14610
H I HIVIQ-2-5D5	6.03	3.02	15.08	30.03	15.01	75.07	5.63	2.80	14.06	26.00	12.99	64.99
HTHMQN-SDS	1070	_	2140	6820	_	13640	935	_	1870	4910	_	9820
ทาทเงเนเง-อบอ	4.76	_	9.52	30.34	_	60.68	4.16	_	8.32	21.84	_	43.68
HGHMQN-2-SDS	1295	_	2590	6820	_	13640	935	_	1870	4910	_	9820
ทินทีเพ่นเพ-2-จับจั	5.76	_	11.52	30.34	_	60.68	4.16	_	8.32	21.84	_	43.68
HTHMQR/L-SDS	1070	355	1425	10140	3380	13520	1000	335	1335	8215	2735	10950
H I HIVIUN/L-SUS	4.76	1.58	6.34	45.11	15.04	60.14	4.45	1.49	5.94	36.54	12.17	48.71
LITUMOD/L 2 CDC	1355	450	1805	10140	3380	13520	1265	420	1685	8215	2735	10950
HTHMQR/L-2-SDS	6.03	2.00	8.03	45.11	15.04	60.14	5.63	1.87	7.50	36.54	12.17	48.71

- 1. Uplift resistances have been increased 15% for wind or earthquake loading with no further increase allowed.
- 2. Specify W_2 where applicable and Hip Skew Angle.
- 3. Connector must be installed centered on minimum 2x8 girder vertical web.
- 4. A minimum 2-ply carrying member is required for the tabulated loads.
- 5. Carrying truss plies must be adequately fastened together as determiend by Designer.
- 6. Simpson Strong-Tie® Strong-Drive® SDS Heavy-Duty Connector screws are permitted to be installed through metal truss plates as approved by the Truss Designer (pre-drilling required through the plate using a maximum of %2" bit).
- 7. Tabulated resistances for three-member configurations assume that each hip carries 40% of the total load and the jack carries 20% of the total load. Tabulated resistances for single hip-jack configurations assume that 75% of the total load is distributed to the hip and 25% to the jack.
- 8. Joist bearing assumes the bottom chord of the truss is sitting in the hanger seat where Q_r/A_b and Q_r'/A_b' = 812 psi D.Fir-L and 615 psi S-P-F. See 6.5.4 and 7.5.9 TPIC 2014.
- 9. End grain bearing assumes a vertical web is sitting in the hanger seat.

CGH Corner Girder Hangers

The CGH is a multi-purpose connector used for connecting hip and jack trusses to bottom chords of girder trusses at a 45° skew.

MATERIAL: Face plate - 3 gauge; Stirrups – 11 gauge

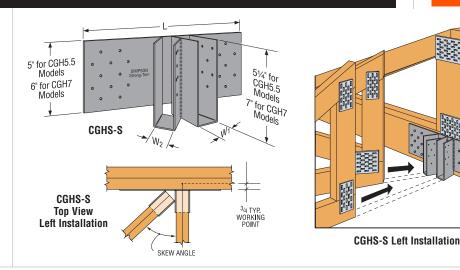
FINISH: Simpson Strong-Tie® gray paint INSTALLATION:

- · Use all specified fasteners.
- · All multiple members must be fastened together to act as a single unit.
- When using single ply hip or jack trusses, fasten the member to the connector with 10dx11/2" nails.

TO ORDER:

· Specify left or right hip skew.

OPTIONS: None



	Dii	mensio	ns		Faatanara			Factored F	Resistance	
		(in)			Fasteners		D.F	ir-L	S-I	P-F
Model							Uplift	Normal	Uplift	Normal
No.	14/	147		Hooder	Uin	look	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
	W ₁	W ₂	L	Header	Hip	Jack	lbs	lbs	lbs	lbs
							kN	kN	kN	kN
CGH5.5SS	1%	1%	14				1180	5205	1035	3695
CGH5.5SD	1%	31/4	15	24-16d	4-10dx1½	4-10dx1½	1100	3203	1033	3093
CGH5.5DS	31/4	1%	16	24-10u	4-10UX172	4-10ux 1 72	5.26	23.18	4.61	16.46
CGH5.5DD	31/4	31/4	17				J.20	23.10	4.01	10.40
CGH7SS	1%	1%	14				1765	7820	1555	5550
CGH7SD	1%	31/4	15	24-16d	6-10dx11//	6-10dx1½	1703	7020	1333	3330
CGH7DS	31/4	1%	16	24-10u	U-100X172	U-100X172	7.86	34.83	6.93	24.72
CGH7DD	31/4	31/4	17				7.00	04.00	0.90	27.72

1. Factored uplift resistances have been increased 15% for short term loading, and are for each connecting member. Reduce where other loads govern.

SIMPSON

- 2. The factored normal resistances are based on the combined load from both connecting members.
- 3. For single ply hips or jacks verify that the 3" bearing length does not govern.
- 4. Factored uplift resistances shown are for each joist.
- 5. **NAILS:** 16d = 0.162" dia. x 3½" long, $10dx1\frac{1}{2} = 0.148$ " dia. x $1\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.

TJC37 Jack Truss Connector

TJC37 is a versatile connector for jack trusses. Adjustable from 0 to 67.5 degree (shipped with 67.5 degree bend). Nail hole locations allow for easy installation. Minimum nailing option provides faster installation and lower installed cost.

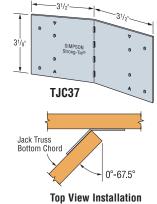
MATERIAL: 16 gauge FINISH: Galvanized

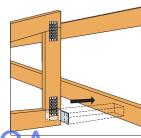
INSTALLATION: • Use all specified fasteners; see General Notes.

- Can be installed filling round holes only, or filling round and triangle holes for maximum values.
- To reduce the potential for splitting, install the TJC37 with a minimum 3/16" edge distance on the chord members (must be centered on 2x4 chords).
- Position the jack truss on the inside of the bend line with the end of the jack truss flush with the bend line.
- Bend the TJC37 to the desired position (one bend cycle only).
- . No bevel cut required.
- Applications involving attachment of TJC37 to the top chord requires minimum 2x6 carrying member for jack truss pitches up to 7:12, and 2x8 or larger for pitches greater than 7:12.

	Faste	eners		Fact	ored Resist	ance (K _D =	1.00)	
				D.Fir-L			S-P-F	
Model No.	Carrying	Carried	0°	1°-60°	61°-67.5°	0°	1°-60°	61°-67.5°
140.	Member	Member	lbs	lbs	lbs	lbs	lbs	lbs
			kN	kN	kN	kN	kN	kN
TIC27 (Min)	4-8dx1½	4.04.41/	495	495	495	350	350	350
TJC37 (Min)	4-0UX 1 72	4-8dx1½	2.20	2.20	2.20	1.56	1.56	1.56
TJC37 (Max)	6-8dx1½	6-8dx1½ 950 795	795	650	675	565	465	
10007 (IVIAX)	U-0UX 1 72	U-OUX 1 72	4.23	3.54	2.90	3.00	2.51	2.07

- 1. No load duration increase is permitted for short-term loading ($K_D = 1.15$).
- Factored resistances are for uplift and downward/direction





Typical TJC37 Installation

HHSUQ Heavy Severe Skew Truss Hangers

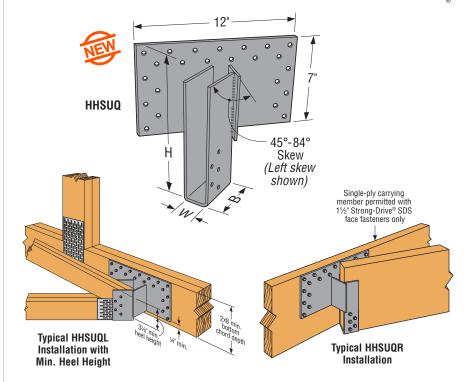


The new HHSUQ is a high-load, face-mount, truss-to-truss hanger designed to accommodate severe skews (45°-84°) enabling a greater range of installation applications. Fastening the HHSUQ with Simpson Strong-Tie® Strong-Drive® SDS Heavy-Duty Connector screws makes installation fast and easy, while eliminating the inconvenience of bolted applications.

MATERIAL: Back plate—3 gauge; stirrup—7 gauge FINISH: Simpson Strong-Tie® gray paint INSTALLATION: • Use all specified fasteners. See General Notes.

- Illustrations below show left and right skews HHSUQR/L (HHSUQR = skewed right; HHSUQL = skewed left).
- The joist/truss end may be square cut or bevel cut with a 3½ minimum heel height.
- Strong-Drive SDS Heavy-Duty Connector screws supplied for all round holes.
- All multiple members must be fastened together to act as a single unit.
- When Strong-Drive SDS Heavy-Duty Connector screws are installed through metal truss plates the application must be approved by the Truss Designer. Pre-drilling is required using a %2" bit.

TO ORDER: • Specify left or right skew and the skew angle (degrees).



	0	imension	ıs	Foots	eners		Factored F	Resistance	
		(in)		rasii	ellers	D.F	ir-L	S-I	P-F
Model						Uplift	Normal	Uplift	Normal
No.			_	Handa	1-1-1	(K _D =1.15)	(K _D =1.00)	(K _D =1.15)	(K _D =1.00)
	W	Н	В	Header	Joist	lbs	lbs	lbs	lbs
						kN	kN	kN	kN
HHSUQ28-SDS	1%	71/4	3½	23-¼"x3" SDS	5-1/4"x11/2" SDS	1890	5530	1360	5400
nnsuuzo-sus	1 98	7 74	3 72	23-74 X3 SDS	3-74 X 172 SDS	8.41	24.60	6.05	24.02
HHSUQ210-SDS	15/8	91/4	3½	23-¼"x3" SDS	5-1/4"x11/2" SDS	1890	5530	1360	5400
HH30Q210-3D3	178	974	372	23-74 X3 3D3	J-74 X 172 3D3	8.41	24.60	6.05	24.02
HHSUQ212-SDS	15%	111/4	3½	23-¼"x3" SDS	5-¼"x1½" SDS	1890	5530	1360	5400
ппоиц212-оио	178	1174	3 72	23-74 X3 SDS	3-74 X 172 SDS	8.41	24.60	6.05	24.02
HHSUQ28-2-SDS	35/16	71/4	31/2	23-¼"x3" SDS	5-1/4"x3" SDS	1890	5530	1360	5530
HH3UQ20-2-3D3	3716	1 74	372	23-74 X3 3D3	3-74 X3 3D3	8.41	24.60	6.05	24.60
HHSUQ210-2-SDS	35/16	91/4	3½	23-¼"x3" SDS	5-1/4"x3" SDS	1890	5530	1360	5530
HH30Q210-2-3D3	3716	974	372	23-74 X3 3D3	3-74 X3 3D3	8.41	24.60	6.05	24.60
HHSUQ212-2-SDS	35/16	111/4	3½	23-1/4"x3" SDS	5-1/4"x3" SDS	1890	5530	1360	5530
111130 Q212-2-303	3716	1174	372	23-74 X3 3D3	J-74 X3 3D3	8.41	24.60	6.05	24.60
HHSUQ48-SDS	35/8	71/4	3½	23-¼"x3" SDS	5-1/4"x3" SDS	1890	5530	1360	5530
nnouu40-ouo	378	1 74	372	23-74 X3 3D3	3-% X3 3D3	8.41	24.60	6.05	24.60
HHSUQ410-SDS	35/8	91/4	3½	23-¼"x3" SDS	5-1/4"x3" SDS	1890	5530	1360	5530
111304410-303	378	374	372	20-74 80 000	J-74 XO ODO	8.41	24.60	6.05	24.60
HHSUQ412-SDS	35/8	111/4	3½	23-1⁄4"x3" SDS	5-1/4"x3" SDS	1890	5530	1360	5530
1111000412-000	J78	1174	372	20-74 80 303	J-74 AU UUU	8.41	24.60	6.05	24.60
HHSUQ414-SDS	35/8	131/4	3½	23-¼"x3" SDS	5-1/4"x3" SDS	1890	5530	1360	5530
1111300414-303	378	1374	372	23-74 X3 3D3	J-74 XO ODO	8.41	24.60	6.05	24.60

- Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed. Reduce where other load durations govern.
- 2. Strong-Drive® SDS Heavy-Duty Connector screws that penetrate all plies of the supporting girder (screws must penetrate a minimum of 1* into the last ply of the truss) may also be used to transfer the load through all of the plies of the supporting girder. When Strong-Drive SDS Heavy-Duty Connector screws do not penetrate all plies of the supporting girder, supplemental Strong-Drive SDS Heavy-Duty Connector screws at the hanger locations may be required to transfer the load to the truss plies not penetrated by the face fasteners, as determined by the Designer. 3* long Strong-Drive SDS header fasteners may be replaced with 41% or 6 long Strong-Drive SDS le vy-Duty Connector screws with no reduction in capacity.
- 3. Resistances shown are based on a minimum 2-ply 2x8 carrying member. For single 2x carrying members, replace the 3" long Strong-Drive SDS Heavy-Duty Connector screws with 1½" long Strong-Drive® SDS Heavy-Duty Connector screws and reduce the factored normal resistances to 3820 lbs (16.99 kN) D.Fir-L and 2750 lbs (12.23 kN) S-P-F. The tabulated uplift resistances do not change.
- Girders must be adequately laterally braced to prevent excessive displacement due to secondary torsional stresses.
- 5. As per 12.2.2.5 CSA 086-14, the carrying member must be evaluated using a reduced cross sectional area at the hanger location. The reduction in area is equal to seven (7 1½" diameter x 3" long holes (1½" long for ½"x1½" Strong-Drive SDS Heavy-Duty Connector sorew)

THGQ/THGQH/HTHGQ SCL and Truss Girder Hangers





This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

A lower cost alternative to bolted hangers, the THGQ and THGQH hangers for multi-ply girder trusses use Simpson Strong-Tie® Strong-Drive® SDS Heavy-Duty Connector screws to provide high load capacities and easier installation compared to bolts. The SDS screws help transfer the load between the plies of the supporting girder when they penetrate all plies.

THGQ and THGQH models offer minimum and optional maximum fastener quantities to accommodate varying design needs. Allowable loads for various girder web member sizes provide additional installation options.

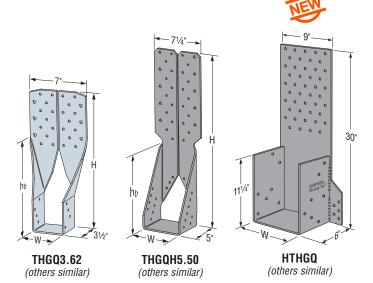
The HTHGQ is a high-load version designed to carry multi-ply trusses or composite lumber up to 5-ply girder trusses. For high-load capacities and easier installation compared to bolts, the HTHGQ is designed for use with Strong-Drive® SDS Heavy-Duty Connector screws. MATERIAL: THGQ-7 gauge, THGQH/HTHGQ-3 gauge

FINISH: THGQ—Galvanized, THGQH/HTHGQ—Simpson Strong-Tie® gray paint

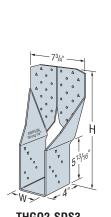
INSTALLATION: • Use all specified fasteners. See General Notes.

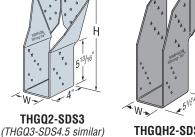
- Can be installed filling round holes only (minimum values), or filling round and triangle holes for maximum values.
- · Strong-Drive SDS Heavy-Duty Connector screws supplied for all round and triangle holes. Installation may not require use of all Strong-Drive SDS Heavy-Duty Connector screws.
- All multiple members must be fastened together to act as a single unit.
- . The thickness of the supporting girder must be equal to or greater than the screw length. For applications where the length of the supplied screws exceeds the thickness of the supporting girder, 3" or 4½" screws may be substituted for the longer length screws with no load reduction, or a shim block may be used as approved by the Designer.
- · Girders must be adequately laterally braced to prevent excessive displacement due to secondary torsional stresses.
- Strong-Drive SDS Heavy-Duty Connector screws driven through truss plates must be approved by the Designer. Pre-drilling using a 5/32" bit is required.

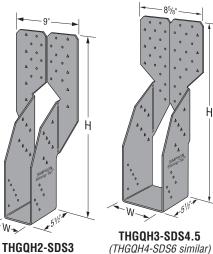
OPTIONS: THGQH hangers for multi-ply truss girders may be skewed 45 degrees. THGQH for structural composite lumber (SCL) cannot be skewed. See Hanger Options on pages 230-231.



U.S. Patent Pending



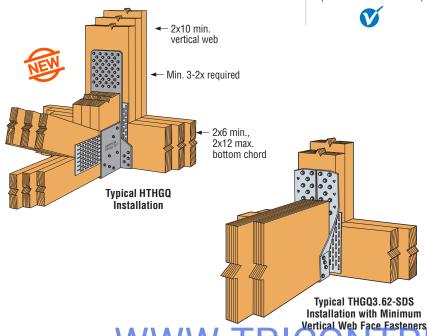


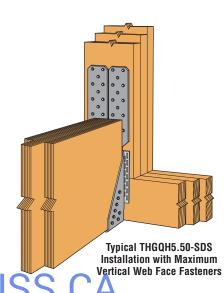












THGQ/THGQH/HTHGQ SCL and Truss Girder Hangers



			nsions			Faste	nore			Resistance	
		(i	n)	Max.	Min	Tuste	,11013	D.F		S-I	
	Model			Girder	Min. Vert Web			Uplift	Normal	Uplift	Normal
	No.	w	Н	Truss B.C. Depth	Size	Header	Joist	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
				Dehtii			00.01	lbs	lbs	lbs	lbs
							~~ ~~	kN	kN	kN	kN
						MULTI-PLY TRU	SS SIZES		11055		
	THGQ2-SDS3 (Min)				2x8	22-1/4x3 SDS	10-1/4x3 SDS	5205	11655	3750	8395
			16	2x6				23.15	51.85	16.68	37.34
	THGQ2-SDS3 (Max)				2x10	28-1/4x3 SDS	14-1⁄4x3 SDS	6555 29.16	18055 80.32	4720 21.00	13000 57.83
								5790	12555	4170	9040
	THGQH2-SDS3 (Min)	35/16			2x8	18-1/4x3 SDS	14-1⁄4x3 SDS	25.76	55.85	18.55	40.21
			25	2x10				14190	18455	10215	13285
	THGQH2-SDS3 (Max)				2x10	28-1/4×3 SDS	26-1/4×3 SDS	63.12	82.10	45.44	59.10
				2 12		// // 000		7420	26375	5345	19320
颤	HTHGQ2-SDS4.5		30	2x12	2x10	55-1/4×41/2 SDS	14-1/4x3 SDS	33.01	117.33	23.78	85.94
	THOOLOGO CDC4 E (Min)				00	00.1/41/.000	40.1/41/.CDC	5205	11655	3750	8395
	THGQ3-SDS4.5 (Min)		16	2x6	2x8	22-1/4×41/2 SDS	10-1/4×41/2 SDS	23.15	51.85	16.68	37.34
	THGQ3-SDS4.5 (Max)		10	2.00	2x10	28-1/4×41/2 SDS	14-1/4×41/2 SDS	6555	17760	4720	12785
	TTICCO-SDO4.5 (Wax)				2.710	20-74/472303	14-748472 000	29.16	79.00	21.00	56.87
	THGQH3-SDS4.5 (Min)	415/16			2x10	32-1/4x41/2 SDS	14-1/4×41/2 SDS	5790	17860	4170	12860
			25	2x10	2///0	02 /4//1/2020	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	25.76	79.45	18.55	57.21
	THGQH3-SDS4.5 (Max)				2x12	38-1/4x41/2 SDS	26-1/4×41/2 SDS	14190	21055	10215	15160
								63.12	93.66	45.44	67.44
凾	HTHGQ3-SDS4.5		30	2x12	2x10	55-1/4×41/2 SDS	14-1/4x3 SDS	7420 33.01	33685 149.84	5345 23.78	24850 110.54
~								5790	17860	4170	12860
	THGQH4-SDS6 (Min)				2x10	34-1/4x6 SDS	14-1/4x6 SDS	25.76	79.45	18.55	57.21
			25	2x12				14190	24870	10215	17905
	THGQH4-SDS6 (Max)	6%16			2x12	40-1/4×6 SDS	26-1/4×6 SDS	63.12	110.63	45.44	79.65
<u>a</u>	HT11004 0D04 5			0.40	0.40	55.4/ 41/ 0D0	11.1/ 0.000	7420	33930	5345	28400
鄭	HTHGQ4-SDS4.5		30	2x12	2x10	55-1/4x41/2 SDS	14-1⁄4x3 SDS	33.01	150.93	23.78	126.33
鹹	HTHGQ5-SDS4.5	81/8	30	2x12	2x10	55-1/4×41/2 SDS	14-1/4×3 SDS	7420	33930	5345	28400
	11111003-3034.3	078	30	2.8.12				33.01	150.93	23.78	126.33
					STRUCTU	RAL COMPOSITE I	LUMBER (SCL) SIZ				
鹹	THGQ3.62-SDS3 (Min)				2x8	22-1/4x3 SDS	8-1/4x3 SDS	4725	9510	3400	6850
			161/16	2x6				21.02	42.30	15.12	30.47
凾	THGQ3.62-SDS3 (Max)				2x10	28-1/4×3 SDS	8-1/4x3 SDS	4725 21.02	12815 57.01	3400 15.12	9225 41.04
_		35/8						6640	16540	4780	11910
凾	THGQH3.62-SDS3 (Min)				2x6	26-1/4x3 SDS	18-1⁄4x3 SDS	29.54	73.58	21.26	52.98
		1	241/2	2x8	_			6640	17695	4780	12740
鄭	THGQH3.62-SDS3 (Max)				2x8	36-1/4x3 SDS	18-¼x3 SDS	29.54	78.71	21.26	56.67
	THOOF FO CDC4 F (Mic)				00	04.1/2/1/ 000	0.1/9/1/ 000	4725	10945	3400	7880
靊	THGQ5.50-SDS4.5 (Min)		171/.	2x8	2x8	24-1/4×41/2 SDS	8-1/4×41/2 SDS	21.02	48.69	15.12	35.05
靈	THGQ5.50-SDS4.5 (Max)		171/4	2.80	2x10	32-1/4×41/2 SDS	8-1/4×41/2 SDS	4725	12570	3400	9050
	1110 Q0.00 0D04.0 (Max)	5½			2.710	02 /4A4/20D0	0 /4/47/2000	21.02	55.92	15.12	40.26
鹹	THGQH5.50-SDS4.5 (Min)	7,2			2x6	28-1/4×41/2 SDS	16-1/4×41/2 SDS	5900	17415	4250	12535
9			25	2x10				26.25	77.47	18.91	55.76
凾	THGQH5.50-SDS4.5 (Max)				2x8	38-1/4×41/2 SDS	16-1/4×41/2 SDS	5900	25155	4250	18110
								26.25 5900	111.90 19750	18.91 4250	80.56 14220
鲰	THGQH7.25-SDS6 (Min)				2x8	28-1/4×6 SDS	16-1/4x6 SDS	26.25	87.86	18.91	63.26
		71/4	241/2	2x10				5900	26660	4250	19195
鄭	THGQH7.25-SDS6 (Max)				2x12	46-1/4×6 SDS	16-1/4x6 SDS	26.25	118.59	18.91	85.39
l							<u> </u>	_0.20			00.00

^{1.} Factored uplift resistances have been increased 15% for short term load duration no further incrase is permitted.

2. A minimum 3-ply girder truss (header) is required for all HTHGQ sizes.

3. Minimum bottom chord depth of the joist shall be 2x6.

When end grain bearing is used with HTHGQ models, the factored normal resistances are 33930 lbs (150.93 kN) D.Fir-L and 28400 lbs (126.33 kN) S-P-F. The factored uplift resistances are 5300 lbs (23.57 kN) D.Fir-L and 3815 lbs (16.97 kN) S-P-F.

^{5.} Designer must ensure that the girder truss is capable of supporting the applied loads based on the reduced cross

^{6.} Strong-Drive SDS Heavy-Duty Connector screws that penetrate all plies of the supporting girder a minimum of 1" into the last ply may also be used to Transfer the load between plies of the supporting girder. When Strong-Drive SDS Heavy-Duty Connector screws do not penetrate all plies supplemental Strong-Drive SDS Heavy-Duty Connector screws may be required to transfer the load to thee truss plies not penetrated by the face fasteners, as determined by the Pacings. by the Designer.

^{= 812} psi D.Fir-L and 615 psi 9 TPIC 2014.

THGB/THGBH/THGW Truss Girder Hangers

SIMPSON

High-capacity, welded hangers for multi-ply girder trusses. Two models offer higher design load values and optional installation with the Strong-Drive® SDS Heavy-Duty Connector screw.

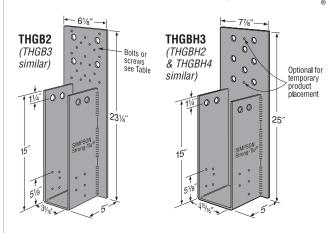
MATERIAL: 3 gauge FINISH: Simpson Strong-Tie® gray paint DESIGN: • Vertical web on supporting girder truss must be 2x8 (min.) for 4-bolt and 6-bolt applications and 2x12 for 8-bolt applications per 5.3.8.2 CSA 086-14.

- · Designer must ensure that vertical web member supporting hanger is capable of resisting applied loads based on net cross sectional area.
- 1/4"x3" Strong-Drive SDS Heavy-Duty Connector screw must be attached to a minimum 2-ply header (3").
- Joist bearing assumes Q_r/A_h and Q_r'/A_h' = 812 psi D.Fir-L and 615 psi S-P-F. See 6.5.4 and 7.5.9 TPIC 2014.
- Maximum bottom chord depth on header shall be 11%".
- To achieve the tabulated uplift resistances the maximum bottom chord depth of the joist shall be 71/4".

INSTALLATION: • Use all specified fasteners.

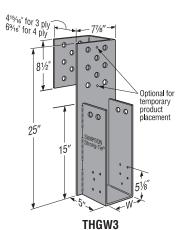
· All multiple members must be fastened together to act as a single unit. OPTIONS: • See Hanger Options, page 230.

Fasteners

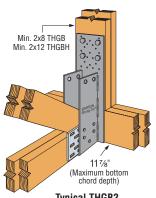


Factored Resistance

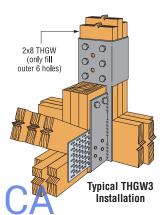
		Fas	steners	Minimum	D.F	ir-L	S-I	P-F	
Model	Width			Header	Uplift	Normal	Uplift	Normal	
No.	(in)	1-1-4		Thickness	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$	
		Joist	Header	(in)	lbs	lbs	lbs	lbs	
					kN	kN	kN	kN	
				0	5175	8290	4085	6545	
				3	23.02	36.88	18.17	29.11	
			4 2/11 8 4 15	417	5175	12435	4085	9815	
THORO	05/	10-10d	4-¾" MB	4½	23.02	55.32	18.17	43.66	
THGB2	35/16	& 2-¾" MB		6	5175	13615	4085	10750	
		2 /4 IVID		0	23.02	60.56	18.17	47.82	
			10 1/"va" CDC	3	5175	13805	4085	9940	
			19-1/4"x3" SDS	3	23.02	61.41	18.17	44.22	
				3	5175	12435	4085	9815	
				3	23.02	55.32	18.17	43.66	
THGBH2	25/	10-10d &	8-¾" MB	41/2	5175	14385	4085	11355	
INGBNZ	35/16	2-¾" MB	0-%4 IVID	4 7/2	23.02	63.99	18.17	50.51	
		2 /4 IVID		6	5175	14385	4085	11355	
				ь	23.02	63.99	18.17	50.51	
				3	7760	8290	6125	6545	
				3	34.52	36.88	27.25	29.11	
			4 3/" MD	4½	7760	12435	6125	9815	
THGB3	415/16	10-10d &	10-10d &	4-¾" MB	4 72	34.52	55.32	27.25	43.66
Indbo	4 '916	2-¾" MB		6	7760	13615	6125	10750	
		2 /4 1015		0	34.52	60.56	27.25	47.82	
			19-1/4"x3" SDS	3	7760	13805	6125	9940	
			19-74 X3 3D3	3	34.52	61.41	27.25	44.22	
				3	7760	12435	6125	9815	
		40.40.1		S	34.52	55.32	27.25	43.66	
THGBH3	415/16	10-10d &	8-¾" MB	41/2	7760	18390	6125	14520	
HIGDIIS	4.216	2-¾" MB	0-74 IVID	472	34.52	81.81	27.25	64.59	
		2 /4 1015		6	7760	18605	6125	14690	
				0	34.52	82.76	27.25	65.35	
THGW3-3				41/23	7760	18650	6125	14725	
1110110-0	415/16	10-10d &	6-¾" MB	7 /2-	34.52	82.96	27.25	65.50	
THGW3-4	4 /16	2-¾" MB	0-74 IVID	6 ³	7760	20830	6125	16065	
1110110-4				U	34.52	92.66	27.25	71.46	
				3	8850	12435	8170	9815	
		10-10d		J	39.37	55.32	36.34	43.66	
THGBH4	6%	10-10d &	8-¾" MB	41/2	8850	18650	8170	14725	
HIUDHA	0716	2-¾" MB	0-74 IVID	772	39.37	82.96	36.34	65.50	
				6	8850	21865	8170	17265	
	1	1	I .		00.00	0=00	0001	=0.00	



U.S. Patent 6,230,466



Typical THGB2 Installation



Uplift resistances have been increased 15% for wind or earthquake loading with no further increase allowed.

6-3/4" MB

2. When using 10-10d nails only on carried member, uplift resistance is 2945 lbs. (13.10 kN) for D.Fir-L and 2590 lbs (11.52 kN) for S-P-F.

41/23

63

39.37

8850

39.37

8850

39.37

97.26

18650

82.96

24870

110.63

36.34

8170

36.34

8170

36.34

76.80

14725

65.50

19630

87.32

3. THGW is sized to fit the header thickness sho 4. **NAILS:** 10d = 0.148" dia. x 3" lor g. See pages

10-10d &

2-3/4" MB

6%6

other nail siz

THGW4-3

THGW4-4

THGBV/THGBHV/THGWV SCL-to-Truss Girder Hangers



An extension of the THGB/THGBH/THGW series, these high-capacity, welded hangers are designed for attaching multi-ply structural composite lumber (SCL) beams to girder trusses. Two models offer higher design values and optional installation with the Strong-Drive® SDS Heavy-Duty Connector screws. Two bucket heights are available for each width to accommodate a range of SCL sizes. Options for skewing or dropping the buckets for conditions where the SCL joist is lower than the girder bottom chord provide design flexibility for a variety of SCL-to-truss connections.

MATERIAL: 3 gauge FINISH: Simpson Strong-Tie® gray paint

DESIGN: • Vertical web on supporting girder truss must be 2x8 (min.) for 4-bolt and 6-bolt applications and 2x12 for 8-bolt applications per 5.3.8.2 CSA 086-14.

- Designer must ensure that vertical web member supporting hanger is capable of resisting applied loads based on net cross sectional area.
- ¼"x3" Strong-Drive SDS Heavy-Duty Connector screws must be attached to a minimum 2-ply header (3").
- Maximum bottom chord depth on header shall be 11%".

INSTALLATION:

Catalogue C-C-CAN2015 © 2015 SIMPSON STRONG-TIE COMPANY INC

THGWV7.25/9

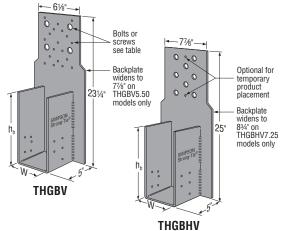
THGWV7.25/11

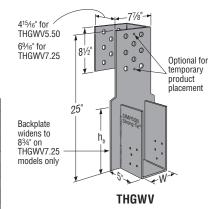
- · Use all specified fasteners.
- All multiple members must be fastened together to act as a single unit.

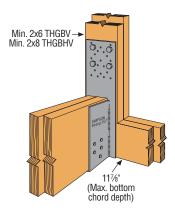
OPTIONS: • See Hanger Options, page 230.

	Depth 9¼ - 14 11¼ - 20 9¼ - 14	Model No.	Han Dimensi	
Width	Depth		W	hb
	01/ 1/	THGBV3.62/9		9
31/2	974 - 14	THGBHV3.62/9	35%	9
3 7/2	111/ 00	THGBV3.62/11	3%	11
	1174 - 20	THGBHV3.62/11		11
		THGBV5.50/9		
	91/4 - 14	THGBHV5.50/9		9
51/4		THGWV5.50/9	5½	
374		THGBV5.50/11	372	
	111/4 - 20	THGBHV5.50/11		11
		THGWV5.50/11		
	91/4 - 14	THGBHV7.25/9		9
7	374 - 14	THGWV7.25/9	71/4	9
1	111/4 - 20	THGBHV7.25/11	1 74	11
	1174 - 20	THGWV7.25/11	[''

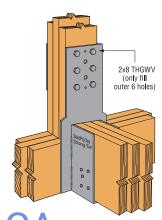
Factored Resistance







Typical THGBV3.62/9 Installation



		Fa	esteners		n c	ir-L	0 1	P-F
Model	Width			Minimum				Normal
No.	(in)							(K _D =1.00)
110.	(,	Joist	Header			(2)	,	,
				3 4½ 6				
				3				
		Joist Header Thicknes (in) 3 4-3/4" MB 41/2 6 19-1/4"x3" SDS 3 10-10d 8-3/4" MB 41/2 6 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3					9815	
THGBV3.62/9			4-¾" MB	41/2				43.66
THGBV3.62/11	35/8	10-10d						10750
11102 70.02711				6				47.82
								9940
			19-¼"x3" SDS	Header Thickness (in)			44.22	
								9815
				3				43.66
THGBHV3.62/9								11355
THGBHV3.62/11	3%	10-10d	8-¾" MB	4½	$ \begin{array}{c c c c c c c c c c c c c c c c c c c $			
					2945	Normal	11355	
				Thickness (in) Thickness (ibs ibs NN KN KN KN KN KN KN KN KN K	50.51			
				0	2945	8290	2590	6545
			4-¾" MB	3	13.10	36.88	11.52	29.11
				4½ 6 8S 3 4½ 6 3 4½ 6 3 4½ 6 4½ 6 4½ 6 4½ 6 4½ 3 4½ 6	2945	12435	2590	9815
THGBV5.50/9	5½				13.10	55.32	11.52	43.66
THGBV5.50/11	372				2945	13615	2590	10750
					13.10	60.56	11.52	47.82
			10-1//"v3" SDS					9940
			13 74 80 000			-		44.22
				3				9815
							-	43.66
THGBHV5.50/9	5½	10-10d	8-3/4" MB	41/6				14520
THGBHV5.50/11	0,2	10 100	0 /4 1015	13.10 55.32 11.52 4 2945 13615 2590 1 13.10 60.56 11.52 4 2945 13805 2590 9 3 13.10 55.32 11.52 4 2945 14385 2590 1 3.10 63.99 11.52 5 6 2945 14385 2590 1 13.10 63.99 11.52 5 6 13.10 63.99 11.52 5 3 2945 8290 2590 6 13.10 36.88 11.52 2 2945 12435 2590 9 3 13.10 36.88 11.52 2 3 2945 12435 2590 9 6 13.10 60.56 11.52 4 2945 13615 2590 1 6 13.10 60.56 11.52 4 2945 13615 2590 9 3 13.10 61.41 11.52 4 3 2945 13805 2590 9 3 13.10 61.41 11.52 4 3 2945 13805 2590 9 3 13.10 82.96 11.52 6 3 4½ 2945 18650 2590 1 3 13.10 82.96 11.52 6 3 2945 18650 2590 9 3 13.10 55.32 11.52 4 3 2945 18650 2590 1 3 2945 12435 2590 9 3 13.10 82.96 11.52 6 3 2945 18650 2590 1 3 2945 18650 2590 1 3 2945 12435 2590 9 3 13.10 82.96 11.52 6	64.59			
				6				14690
								65.35
THGWV5.50/9	5½	10-10d	6-¾" MB	41/22				14725
THGWV5.50/11								65.50
				3				9815
THORN (7.05 /2				-				43.66
THGBHV7.25/9	71/4	10-10d	8-¾" MB	41/2				14725
THGBHV7.25/11				172				
			A-¾" MB					
					13.10	97.20	11.52	76.80

1. Uplift resistances have been increased 15% for wind or earthquake loading with no further increase allowed

6-3/4" MB

10-10d

2. THGWV is sized to fit the header thickness shown.

3. NAILS: 10d = 0.148" dia. x 3" long. See bages 22 23 for other nail sizes and information.

24870

110.63

2590

11.52

19630

87.32

2945

13.10

171

Flattened

The Simpson Strong-Tie® TSBR truss spacer-restraint is a time-saving lateral-restraint product for wood and CFS framing that improves quality and safety while helping to meet the prescriptive recommendations of the BCSI-08. Easier to install than wood bracing, the TSBR firmly grips the trusses, capturing on-center spacing and keeping them vertical and plumb after placement, resulting in a better truss installation. The unique design eliminates additional time spent measuring truss spacing and laying out temporary lateral bracing. And once installed, the TSBR can remain in place to be sheathed over, thereby eliminating the need to remove temporary bracing and creating a safer, more stable work platform.

FEATURES: • Enables the quick and accurate spacing of trusses without measuring or adjusting

- Helps meet prescriptive temporary bracing recommendations of the BCSI-08
- Easily "grabs" onto the truss may be put in place with one hand
- Stays in place during sheathing, saving time and making the roof more stable for workers
- Installs in less time and requires less total bracing material than prescriptive wood bracing methods - reducing labor costs
- The TSBR is a direct replacement for the TSB Truss Spacer Bracer.

MATERIAL: 22 gauge FINISH: Galvanized

INSTALLATION: • Use all specified fasteners; see General Notes.

- TSBR lateral restraint locations are as recommended in Table B2-1 of BCSI-08. For more information see the Simpson Strong-Tie Wood Truss Bracing and Restraint Guide (F-TSBRTBD22).
- · Fill all round and triangular holes.

	Dir	nensi	ons		tance $(K_D = 1.15)$)		
Model		(in)			D.Fir-	L	S-P-I	•
No.				Fasteners	Compression	Tension	Compression	Tension
NO.	L	W	D		lbs	lbs	lbs	lbs
					kN	kN	kN	kN
TSBR2-16	171/2	11/4	11/4	4-10dx11//	885	740	630	525
13002-10	17 72	174	174	4-10ux172	3.94	3.29	2.80	2.34
TSBR2-24	251/2	13/4	1	4-10dx1½	685	625	485	445
13bh2-24	2372	174	<u>'</u>	4-10UX172	3.05	2.78	2.16	1.98

TSBR2-24 (TSBR2-16 similar) U.S. Patent 6 993 882 TSBR may be replaced at ridge Typical TSBR2-24 Installation on 2x Wood Truss 1. No load duration increase allowed. 2. Meets or exceeds the temporary lateral restraint recommendations of BCSI-08 3. **NAILS:** $10dx1\frac{1}{2} = 0.148$ " dia. $x1\frac{1}{2}$ " long.

See pages 22-23 for other nail sizes and information.

TBD22 Diagonal Brace

The TBD22 diagonal truss brace offers a time-saving substitute for 2x4 diagonal bracing that helps meet the recommendations of BCSI-08. The TBD travels in a box like a flat strap, and is formed into an A-shape as it is pulled from the carton to provide rigidity and prevent sagging between trusses during installation. As it is fastened to the trusses the brace flattens, allowing sheathing to be installed right over it and saving the time typically needed to remove 2x4 bracing.

When installed on the top and bottom chords as well as the web planes, the TBD captures the lateral construction and wind forces delivered by the TSB truss spacer/bracer and transfers it diagonally in tension to the edge of the braced-truss system. When used in conjunction with the TSBR, the TBD22 meets or exceeds the the recommendations set forth by the BCSI-08.

FEATURES: • Helps meet prescriptive temporary bracing recommendations of the BCSI-08.

- Rigid A-shape design virtually eliminates sagging between trusses spaced 16"-24" on center.
- Can be sheathed over after installation, no need to remove bracing.
- Dimpled nailing grid allows installation with standard pneumatic fasteners.

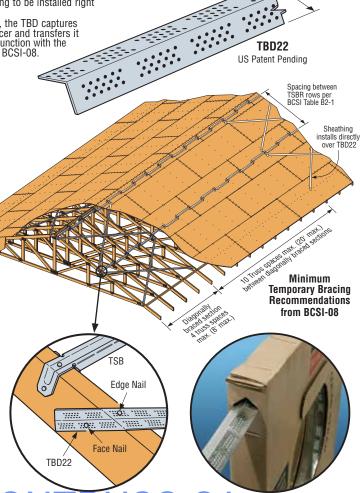
MATERIAL: 22 gauge

INSTALLATION: • Use all specified fasteners; see General Notes.

- Strap does not have holes for fasteners. Nails shall be installed in the dimpled areas and placed to maintain a minimum of 1/4" strap edge distance and a minimum of 1/2" center to center distance. Nails should be installed in the center of the lumber narrow face and with a minimum edge distance of 1" on the lumber wide face.
- TBD22 straps span diagonally at approximately 45°
- Strap shall not be slack, but tight and ready to engage in tension.
 To resist construction forces, diagonal X-bracing is required at each end and every 10 truss spaces (20' max). Refer to BCSI-08 for additional information.
- · At the end of the TBD braces trusses shall be laterally braced to resist out of plane forces.
- Bracing locations shown in the drawing are recommendations for temporary bracing only. Installation of TBD braces for permanent lateral bracing shall be per the Building Designer.

Madal	Fastener	s	Factored Tensile Resistance (K _D =1.15		
Model No.		Intermediate	D.Fir-L	S-P-F	
140.	Strap Ends	Intermediate Trusses	lbs	lbs	
		1103553	kN	kN	
TBD22	1-10dx1½ in face and	1-10dx1½	680	615	
(Min)	1-10dx1½ in edge	1-10ux 172	3.02	2.74	
TBD22	2-10dx1½ in face and	1-10dx1½	895	820	
(Max)	1-10dx1½ in edge	1-10ux 172	3.98	3.65	

- 1. Factored resistances have been increased for construction and wind loading with no further increase allowed.
- Minimum nailing meets or exceeds the temporary bracing recommendations of BCSI-08.
- recommendations of BCSI-08. 3. **NAILS:** $10dx1\frac{1}{2} = 0.148^{\circ}$ dia. x $1\frac{1}{2}$ long. See pages 22-23 for other nail sizes and information Typical TBD 22 Top Chord Installation with Minimum Nailing



TBD22

Dispenser Detail

Plated Truss Connectors

VTCR Single-Sided Valley Truss Clip

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

The VTCR is single-sided valley truss clip that provides a positive connection between the valley truss and the supporting framing below. Installed on top of the roof sheathing, it eliminates the need to add a support wedge under the valley truss or to bevel the bottom chord to match the roof pitch.

- Single-sided for new construction or retrofit applications can be installed after the valley truss is set in place
- Accommodates pitches from 0/12 to 12/12
- · Can be installed with either beveled or non-beveled bottom chords
- Installs with nails or Simpson Strong-Tie® Strong-Drive® SD Connector screws

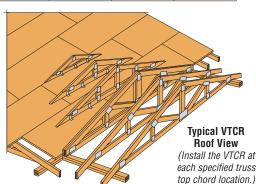
MATERIAL: 18 gauge FINISH: Galvanized INSTALLATION:

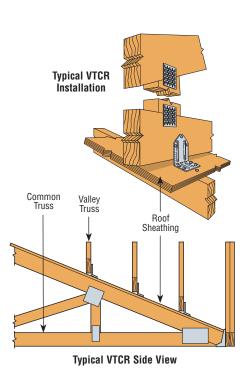


- The dome holes assist in installing the fasteners into the supporting framing at approximately 45°.
- · Install VTCR at all valley truss/common truss intersections.
- VTCR must be installed directly over roof sheathing between 7/16" and 5/8" thick.

	Faste	eners		Factored F	Resistance	
			D.F	ir-L	S-I	P-F
Model	0	W-11	Uplift	Normal	Uplift	Normal
No.	Supporting Framing	Valley Truss	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$
	Training	IIIUSS	lbs	lbs	lbs	lbs
			kN	kN	kN	kN
	4-10d	3-10dx1½	220	595	160	595
VTCR	4-10u	3-100X172	0.98	2.65	0.71	2.65
VIUN	4-SD#9x2½	3-SD#9x1½	575	595	405	595
	4-3D#9XZ/2	3-3D# 9X 1 /2	2.56	2.65	1.80	2.65

- 1. Factored uplift resistances have been increased 15% for wind loads. No further increase is permitted.
- 2. Factored normal resistance assume continuous bearing of the valley truss bottom chord along the roof sheathing. For applications where the supporting framing is less than 24" o/c, the tabulated normal resistances shall be linearly reduced.
- 3. NAILS: 10d = 0.148" dia. x 3" long, $10dx1\frac{1}{2} = 0.148$ " dia. x $1\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.
- 4. SCREWS: SD #9x11/2" (model SD9112) = 0.131" dia. x 1½" long, SD #9x2½" (model SD9212) = 0.131" dia. x 2½" long.





DSC Drag Strut Connector

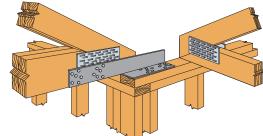
The DSC Drag Strut Connector transfers diaphragm shear forces to the shearwalls. The DSC2 is a smaller, lighter version that installs with fewer screws.

MATERIAL: DSC2-7 gauge, DSC5-3 gauge FINISH: DSC2—Galvanized

DSC5—Simpson Strong-Tie® gray paint

INSTALLATION:

- · Use all specified fasteners; see General Notes.
- . Strong-Drive® SDS Heavy-Duty Connector screws are provided.



Typical DSC5R-SDS3 Installation (DSC2 similar)

000 000	33 TY
	DSC5R/L-SDS3

VTCR

U.S. Patent 6,840,020

(DSC2 similar) (Right hand DSC shown; specify right or left hand when ordering) U.S. Patent 6,655,096

			Fa	actored Resis	tance (K _D =1.15)	
88-4-1	١.		D.Fi	r-L	S-P-F	
Model No.	(in)	Fasteners	Compression	Tension	Compression	Tension
140.	('''')		lbs	lbs	lbs	lbs
			kN	kN	kN	kN
DSC2R/L-SDS3	16	20-1/4"x3" SDS	3740	6530	2695	4700
D302R/L-3D33	10	20-74 X3 SDS	16.66	29.09	12.00	20.94
DCCED/L CDC2	01	21 10 21 000	6495	10630	4675	7655
DSC5R/L-SDS3	21	24-74 X3 3D3	28 93	47.35	20.82	34 10

- 1. Factored resistances have been increased 15% for earthquake and wind loading with no further increase allowed. Lag screws will not generate the tabulated factored resistances.
- Strong-Drive® SDS Heavy-Duty Connector screws minimum penetration is 2¾", minimum end distance is 2½" and minimum edge distance is 5/8" for full load values
- Installation of Strong-Drive SDS Heavy-Duty Connector through truss plates must be approved by the truss engineer. Pe-drilling is required.

GBC Gable Brace Connector

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

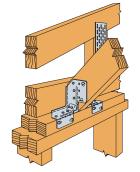
The GBC provides improved anchorage of gable bracing to the exterior wall. Installation flexibility for brace angle. GBC has tension and compression capacities.

MATERIAL: 16 gauge FINISH: Galvanized

INSTALLATION:

- Use all specified fasteners. See General Notes.
- The GBC must be installed in pairs to achieve full load capacity.

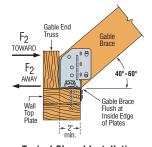
GBC US Patent Pending	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
31	37/8"



SIMPSON

 ${f Strong-Tie}$

Typical GBC Installation

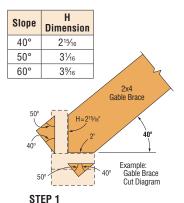


Typical Sloped Installation

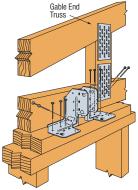
		Faster	ners		Perpend	dicular to Er	ndwall (F ₂)	Factored Ro	esistance(K	(D=1.15)	
		pei	r		D.F	ir-L			S-I	P-F	
	۵.	Conne	ctor	Toward	Anchors	Away fron	n Anchors	Toward	Anchors	Away fron	n Anchors
Model No.	Qty Rea'd			Gable Bra	ace Angle	Gable Bra	ace Angle	Gable Bra	ace Angle	Gable Bra	ace Angle
110.	iioq u	Gable	Тор	40°-45°	46°-60°	40°-45°	46°-60°	40°-45°	46°-60°	40°-45°	46°-60°
		Brace	Plates	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs
				kN	kN	kN	kN	kN	kN	kN	kN
GBC	2	5-8dx1½	7-8d	945	830	695	610	665	580	490	425
GDU	4	J-0UX 1 72	/-ou	4.21	3.70	3.10	2.72	2.96	2.58	2.18	1.89

- 1. For 1% x 3% (or larger) LVL gable brace, the factored resistance at 40° to 45° is 945 lbs (4.21 kN) towards the anchors and 970 lbs (4.32 kN) away from the anchors
- 2. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed. Reduce where other loads govern.
- 3. Use a minimum 2x4 gable brace.
- 4. **NAILS:** 8d = 0.131" dia. x $2\frac{1}{2}$ " long, $8dx1\frac{1}{2} = 0.131$ " dia. x $1\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.

GBC INSTALLATION SEQUENCE

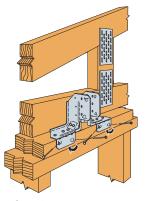


Double angle cut the gable brace to sit flat on the wall double top plate and flush against the gable end truss for 2x4 top plate. The double angle cuts should form a 90° angle on the end of the gable brace.



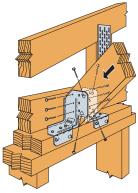
STEP 2

Set each GBC on top of the double top plate so that the bend line slots are flush with the inside edge of the double top plate. Install fasteners into the top of the double top plate.



STEP 3

Bend GBC legs (one time only) over the inside of the double top plate and install fasteners.



STEP 4

Install fasteners into the gable brace.

NOTE: Attach the other end of the gable brace to blocking at the roof diaphragm as directed by the Designer.

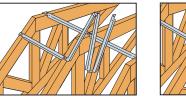
TSF Truss Spacer

The TSF is a fast and accurate method for spacing trusses that eliminates layout marking of top plates and can be left in place under the sheathing. Accuracy is improved, spacing errors are minimized, and it is easy to use.

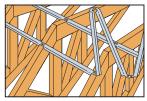
MATERIAL: 24 gauge FINISH: Galvanized INSTALLATION:

- See Installation Sequence below.
- TSF Truss Spacers do not provide bracing of any kind and are not structural members. The TSF is for spacing only. Refer to instructions from architect, engineer, truss manufacturer or other for bracing and installation information

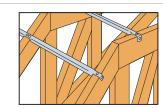
	Dimensio	ns
W	O.C. Spacing	Total Length
1½	16	8'
1½	24	10'
	1½	1½ 16



STEP 1 Nail starting notch to first member.



STEP 2 As each successive member is positioned, unfold TSF to next notch. The notch tee horip nember and align it for nating.



STEP 3

TSF

If spacer does not align with end truss, break spacer off at notch. Then hammer spacer flat, fold it under and nail.

CP CRUSH PLATES Bearing Enhancers

SIMPSON
Strong-Tie

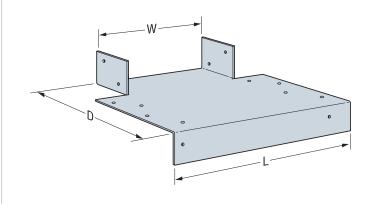
The CP transfers load from the truss or girder to plates for bearing limited conditions. Replaces nail-on scabs or in some cases, an additional ply when needed for bearing.

MATERIAL: See table FINISH: Galvanized DESIGN:

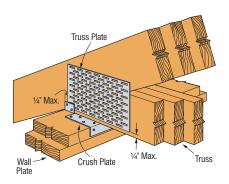
• Factored resistances are in accordance with CSA 086-14 assuming Ω_{Γ}/A_b and $\Omega_{\Gamma}'/A_b'=812$ psi for D.Fir-L and 615 psi for S-P-F. See sections 6.5.4 and 7.5.9 TPIC 2014 when compression loads are applied to both sides of truss chord members at bearing locations.

INSTALLATION: • Use all specified fasteners.

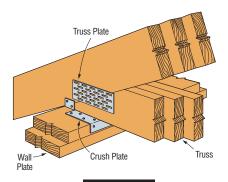
• For Case 1, truss plates must be located a maximum of ¼" from the underside of the truss chord and a maximum of ¼" from the edge of the wall plates in accordance with the reinforcing requirements of 7.5.9 TPIC 2014.



$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	F Bearing K _D =1.00) lbs kN 4515 20.11 9030 40.22 13545 60.33 18065 80.47 7095
Model No. Ga Wall Plate W all Plate Wall Plate Wall Plate Wall Plate Truss Uplift (K _D =1.00) Bearing (K _D =1.00) Uplift (K _D =1.15) Bearing (K _D =1.00) Uplift (K _D =1.00) Bearing (K _D =1.00) Rearing (K _D =1.00) Uplift (K _D =1.00) Bearing (K _D =1.00) Chance CP1-4 16 3½ 4½ 6-10d 4-10dx1½ 225 5965 225 100 225 11390 225 11390 225 11390 225 11390 225 11390 225 11390 225 117895 225 117895 225 117895 225 117895 225 1100 106.28 1.00 106.28 1.00 <th>Bearing Kp=1.00) lbs kN 4515 20.11 9030 40.22 13545 60.33 18065 80.47</th>	Bearing Kp=1.00) lbs kN 4515 20.11 9030 40.22 13545 60.33 18065 80.47
No. Ga Plate W D L Wall Plate Truss (KD=1.15) (KD=1.00) (KD=1.15) (K	Kp=1.00) Ibs kN 4515 20.11 9030 40.22 13545 60.33 18065 80.47
CP1-4 20 CP2-4 16 CP3-6 16 CP3	lbs kN 4515 20.11 9030 40.22 13545 60.33 18065 80.47
CP1-4 20	4515 20.11 9030 40.22 13545 60.33 18065 80.47
CP1-4 20	4515 20.11 9030 40.22 13545 60.33 18065 80.47
CP1-4 20 1% 3½ 4½ 6-10d 4-10dx1½ 225 5965 225 CP2-4 16 3¼ 3½ 5¾ 6-10d 4-10dx1½ 225 11390 225 CP3-4 16 4¾ 3½ 7½ 6-10d 4-10dx1½ 225 17895 225 CP4-4 12 6½ 3½ 9½ 6-10d 4-10dx1½ 225 23860 225 CP1-6 20 1½ 5½ 4½ 10-10d 4-10dx1½ 225 9370 225 CP2-6 16 3¼ 5½ 5¾ 10-10d 4-10dx1½ 225 18740 225 CP3-6 16 2x6 4¾ 5½ 7½ 10-10d 4-10dx1½ 225 28110 225 1.00 125.21 1.00 125.21 1.00 10.00 125.21 1.00	20.11 9030 40.22 13545 60.33 18065 80.47
CP1-4 20 CP2-4 16 CP3-4 16 CP3-4 16 CP3-4 16 CP3-4 16 CP4-4 12 6½ 3½ 9½ 6-10d 4-10dx1½ 1.00 53.14 1.00 53.14 1.00 53.14 1.00 79.71 1.00 79.71 1.00 79.71 1.00 106.28 1.00 106.28 1.00 106.28 1.00 106.28 1.00 106.28 1.00 4-10dx1½ 225 9370 225 18740 225 1.00 3½ 5½ 5¾ 10-10d 4-10dx1½ 225 1.00 83.47 1.00 225 1.00 83.47 1.00 225 28110 <	20.11 9030 40.22 13545 60.33 18065 80.47
CP2-4 16 CP3-4 16 CP3-4 16 CP4-4 16 CP4-4 12 6½ 3½ 9½ 6-10d 4-10dx1½ 100 53.14 100 53.14 100 79.71 1.00 79.71 1.00 79.71 1.00 225 1.00 106.28 1.00 106.28 1.00 106.28 1.00 106.28 1.00 106.28 1.00 41.47 1.00 41.47 1.00 41.47 1.00 41.47 1.00 83.47 1.00 83.47 1.00 125.21 1.00 125.21 1.00 125.21 1.00 125.21 1.00 125.21 1.00 125.21 1.00 125.21 1.00 125.21 1.00 125.21 1.00 125.21 1.00 125.21	9030 40.22 13545 60.33 18065 80.47
CP2-4 16 CP3-4 16 CP3-4 16 CP4-4 12 CP4-4 12 CP4-6 16 CP2-6 16 CP3-6 10-10d 4-10dx1½ CP3-10dx1½ CP3-	40.22 13545 60.33 18065 80.47
CP3-4 16 2x4 4¾ 3½ 7½ 6-10d 4-10dx1½ 225 17895 225 1.00 79.71 1.00 79.71 1.00 225 23860 225 1.00 106.28 1.0	13545 60.33 18065 80.47
CP3-4 16	60.33 18065 80.47
CP4-4 12 6½ 3½ 9½ 6-10d 4-10dx1½ 225 23860 225 CP1-6 20 1½ 5½ 4½ 10-10d 4-10dx1½ 225 9370 225 CP2-6 16 CP3-6 16 2x6 2x6 2x6 2x6 2x6 3½ 5½ 10-10d 4-10dx1½ 225 18740 225 1.00 83.47 1.00 4¾ 5½ 7½ 10-10d 4-10dx1½ 225 28110 225 1.00 125.21 1.00 225 37495 225	18065 80.47
CP4-4 12 6½ 3½ 9½ 6-10d 4-10dx1½ 1.00 106.28 1.00 CP1-6 20 1½ 5½ 4½ 10-10d 4-10dx1½ 225 9370 225 CP2-6 16 2x6 2x6 2x6 2x6 2x6 2x6 2x6 2x6 2x6 2x	80.47
CP1-6 20 CP2-6 16 CP3-6 16 2x6 1% 5½ 4½ 10-10d 4-10dx1½ 225 9370 1.00 41.47 1.00 225 18740 225 1.00 83.47 1.00 83.47 1.00 83.47 1.00 225 1.00 125.21 1.00 125.21 1.00 125.21 1.00 125.21 1.00 125.21 225 37495 225	
CP2-6 16 CP3-6	
CP2-6 16 2x6 2x6 2x6 2x6 2x6 2x6 2x6 2x6 2x6 2x	31.60
CP3-6 16 2x6 434 5½ 7½ 10-10d 4-10dx1½ 225 28110 225 1.00 125.21 1.00 225 275 27495 225 275 275 275 275 275 275 275 275 27	14190
CP3-6 16 4¾ 5½ 7½ 10-10d 4-10dx1½ 225 28110 225 1.00 125.21 1.00 225 275 275 275 275 275 275 275 275 275	63.21
1.00 125.21 1.00 225 37495 225	21285
CD4 6 12 61/ 51/ 01/ 10 10d 4 10dv11/ 225 37495 225	94.81
1.E4-D 1.Z 1 D/2 D/2 M/2 HI-HII 4-HIIX //2	28390
372 372 10 10d 4 10dA172 1.00 167.02 1.00	126.46
CASE 2 (No Reinforcement)	
CP1-4 20 1% 3½ 4½ 6-10d 4-10dx1½ 225 4685 225	3550
178 372 472 0-10d 4-10dx172 1.00 20.87 1.00	15.81
CP2-4 16 3½ 3½ 5¾ 6-10d 4-10dx1½ 225 9370 225	7100
2x4 2x4 1.00 41.74 1.00	31.63
CP3-4 16 4¾ 3½ 7½ 6-10d 4-10dx1½ 225 14055 225	10650
1.00 62.61 1.00	47.44
CP4-4 12 6½ 3½ 9½ 6-10d 4-10dx1½	14195
	63.23
CP1-6 20 1% 5½ 4½ 10-10d 4-10dx1½ 225 7365 225	5575
	24.83
CP2-6 16 3½ 5½ 5¾ 10-10d 4-10dx1½	11150
2x6	49.67
CP3-6 16 44 55 7 10-10d 4-10dx15	16725 74.50
	22305
CP4-6 12 6½ 5½ 9½ 10-10d 4-10dx1½ 223 23400 223 1.00	77.500



Case 1



Case 2

^{1.} Factored bearing resistances assume wall plate and truss are the same species. For a mixed species system use S-P-F values.

^{2.} NAILS: 10d = 0.148" dia. x 31 long, 10dx1½ = 0.148" dia. x 1½ long. See pages 22-23 for other nail sizes and information RICONTRUSS.CA

TBE Truss Bearing Enhancers

One size works with any number of girder plys. The TBE transfers load from the truss or girder to plates for bearing-limited conditions, and provides exceptional uplift capacity. Replaces nail-on scabs that provide lower load

The table lists factored resistances for TBE4 used on 2x4 and TBE6 used on 2x6 top plates. The tables give the different resistances calculated for TBE with and without wood bearing. See page 177 for Alternate Installation.

transfer, or in some cases, an additional ply when needed for bearing.

MATERIAL: 18 gauge

FINISH: Galvanized. See Corrosion Information, pages 14-17.

INSTALLATION: • Use all specified fasteners. See General Notes.

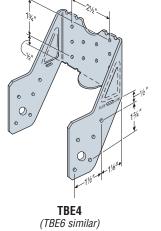
- TBE must be installed in pairs.
- Top plate size is 2x4 for TBE4, 2x6 for TBE6. Use alternate installation for TBE4 and TBE6 on larger plates or pre-sheathed walls.
- Do not use TBEs in end-grain-bearing applications.

TBE FASTENER SCHEDULE

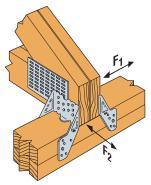
Model	Truce Dive	Fasteners p	er each TBE
No.	Truss Plys	Rafter	Plate
TBE4	1	10-10dx1½	10-10dx1½
I DE4	2 or more	10-10d	10-10d
TDEC	1	10-10dx1½	10-10dx1½
TBE6	2 or more	10-10d	10-10d

NAILS: 10d = 0.148" dia. x 3" long, 10dx1½ = 0.148" dia. x 1½" long. See pages 22-23 for other nail

sizes and information.



(TBE6 similar)
U.S. Patent 5,109,646
Canada Patent 2,044,440



Two TBE installed with two ply girder truss

		Faste	eners	TE	BE Only Facto	red Resistan	ce	Combined	TBE and
	No. of			Uplift	Normal	Late (KD=		Wood B Factored R	
Model No.	Truss Plies	Truss	Plate	(K _D =1.15)	$(K_D = 1.00)$	F1	F2	Normal (K _D =1.00)	TBL ⁶
				lbs	lbs	lbs	lbs	lbs	in
				kN	kN	kN	kN	kN	""
				Ū).Fir-L				
	1	00.104511/	20-10dx1½	1605	3540	655	1415	7800	C 41
	'	20-10dx1½	20-100X1½	7.14	15.75	2.91	6.29	34.70	6.41
	2	00 104	20-10d	1605	3660	655	1415	12180	F 00
TBE4	2	20-10d	20-100	7.14	16.28	2.91	6.29	54.18	5.00
IDE4	3	20-10d	20-10d	1605	3660	655	1415	16445	4.50
	3	20-100	20-100	7.14	16.28	2.91	6.29	73.15	4.50
	4	00.104	20-10d	1605	3660	655	1415	20705	4.05
	4	20-10d	20-100	7.14	16.28	2.91	6.29	92.10	4.25
	4	00.404541/	00 10dv11/	1760	3540	490	1745	10235	0.41
TDF 0	1	20-10dx1½	20-10dx1½	7.83	15.75	2.18	7.76	45.53	8.41
		2 20-10d	20-10d	1760	3860	490	1745	17250	7.09
	2 20-100	20-10d	20-100	7.83	17.17	2.18	7.76	76.73	
TBE6		3 20-10d	00.40.1	1760	3860	490	1745	23945	6.56
	3		20-10d	7.83	17.17	2.18	7.76	106.52	
	4	00 104	00.104	1760	3860	490	1745	30640	0.00
	4	20-10d	20-10d	7.83	17.17	2.18	7.76	136.30	6.29
					S-P-F				
				1605	3220	615	1415	6445	
	1	20-10dx1½	20-10dx1½	7.14	14.32	2.74	6.29	28.67	6.99
				1605	3440	615	1415	9890	
	2	20-10d	20-10d	7.14	15.30	2.74	6.29	43.99	5.37
TBE4				1605	3440	615	1415	13120	
	3	20-10d	20-10d	7.14	15.30	2.74	6.29	58.36	4.74
		00.40.1	00.40.1	1605	3440	615	1415	16345	4.40
	4	20-10d	20-10d	7.14	15.30	2.74	6.29	72.71	4.43
		00.40.1	00.40.1.447	1760	3220	490	1585	8290	0.00
	1	20-10dx1½	20-10dx1½	7.83	14.32	2.18	7.05	36.88	8.99
		00.40.1	00.40.1	1760	3540	490	1585	13680	7.40
TDEO	2	20-10d	20-10d	7.83	15.75	2.18	7.05	60.85	7.42
TBE6	0	00.40.4	00.40.4	1760	3540	490	1585	18750	0.70
	3	20-10d	20-10d	7.83	15.75	2.18	7.05	83.41	6.78
		00.40:	00.40.1	1760	3540	490	1585	23820	0.40
	4	20-10d	20-10d	7.83	15.75	2.18	7.05	105.96	6.46
			A / \ A	/ \ A /		100			100

- Factored resistances are for two TBE's only. Wood factored bearing resistance may be added as shown in the table.
- 2. Factored bearing resistances shown assume Q_T/A_b and $Q_T/A_b' = 812$ psi (5.60 MPa) for D.Fir-L and 614 psi (4.24 MPa) for S-P-F. See section 6.5.4 TPIC 2014 for required bearing reinforcement when compression loads are applied to both sides of truss member.
- Factored uplift resistances have been increased 15% for short term load duration with no further increase allowed; reduce resistances by 15% for standard term load duration.
- Factored resistances are determined by nail shear calculations or tests of the metal connectors. The attached wood members must be designed to withstand the loads imposed by the nail.
- withstand the loads imposed by the nails.

 5. Use lower of top plate or wood truss species.
- 6. Total bearing length, TBL, equals the plate width plus simulated bearing length provided by the TBE. TBE4 = 3½" plate width; TBE6 = 5½" plate width.

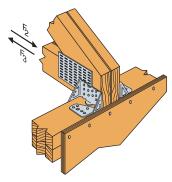
TBE Truss Bearing Enhancers

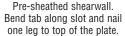
SIMPSON Strong-Tie

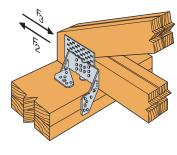
ALTERNATE INSTALLATION (See illustrations at right)

Model	Alternate Installation Factored Resistance										
			ir-L 1.15)		S-P-F (K _D =1.15)						
No.	Uplift	F ₁	F ₂	F ₃	Uplift	F ₁	F ₂	F ₃			
	lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs			
	kN	kN	kN	kN	kN	kN	kN	kN			
TBE4	1605	490	1415	490	1280	370	1005	350			
	7.14	2.18	6.29	2.18	5.69	1.65	4.47	1.56			
TDEO	1760	490	1415	490	1280	370	1005	350			
TBE6	7.83	2.18	6.29	2.18	5.69	1.65	4.47	1.56			

- Alternate Installation Factored Normal Resistances are 0.60 of the TBE only tabulated resistances on page 176.
- 2. TBL values do not apply to Alternate Installation.
- 3. See table footnotes on opposite page.







TBE6 Installed on Double 2x8 Top Plate

TC Truss Connectors

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to $1\frac{1}{4}$ ". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the Designer.

MATERIAL: 16 gauge FINISH: Galvanized INSTALLATION: • Use all specified fasteners. See General Notes.

- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss and clinch on back side).
 Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.
- After installation of roofing materials, nails may be required to be fully seated into the truss. (As required by the Designer or Truss Designer).

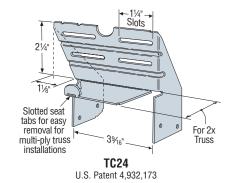
Optional TC Installation

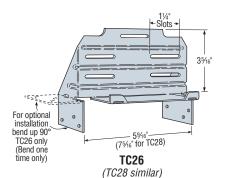
 Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall.
 See optional load tables and installation details.

	Fasteners		Factored Uplift Resistance (K _D =1.15		
Model		W-11	D.Fir-L	S-P-F	
	Truss	Wall Plates	lbs	lbs	
		1 lates	kN	kN	
TC24	4-10d	4-10d	605	430	
1024	4-10u	4-10u	2.69	1.91	
TC26	5-10d	6-10d	1015	720	
1020	5-10u	0-10u	4.51	3.20	
TC28	5-10d	6-10d	1015	720	
1020	J-10u	0-10u	4.51	3.20	

OPTIONAL TC INSTALLATION TABLE

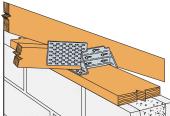
Model	Faste	eners	Factored Uplift Resistance (K _D =1.1		
	347-11		D.Fir-L		
	Truss	Wall Plates	lbs	lbs	
		1 lates	kN	kN	
	5-10dx1½	6-10dx1½	810	660	
TC26		0-10ux 1 72	3.60	2.94	
1020		6 1 0d	930	660	
	5-10d	6-10d	4.14	2.94	
		W W	/ W/ W/ W		



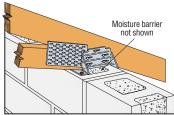




Typical TC24 Installation



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer (8", 10", 12" Wall Installation similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

- Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
- 2. Grout strength is 15 MPa minimum.
- 3. Nail values based on single 2x truss. 10d joist nails must be clinched.
- 4. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
- TC26 fastened to grouted concrete block with 6 ¾6" x 2¼" Titen® screws has a factored uplift resistance of 275 lbs (1.22 kN).

NAILS: 10d = 0.148" dia. x 3" long, 10dx1½ = 0.148" dia. x 1½" long See pages 22-23 for other nail sizes and information. loads are applied.

HTC Heavy Truss Clips

For alignment control between a roof truss and nonbearing walls; the 2½" slot permits vertical truss chord movement when

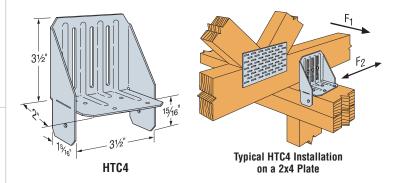
MATERIAL: 18 gauge FINISH: Galvanized

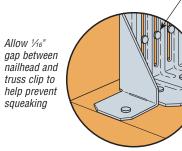
INSTALLATION: • Use all specified fasteners; see General Notes.

- The HTC has a 21/2" slot to accommodate truss movement
- · This connector has high lateral capacity.
- The S/HTC is available for steel truss applications.

	Dimensions	Faste	eners	Factor	ed Resist	ance (K	=1.15)		
Madal			Slot	Withou	ut Gap²	With 11/4" Gap3			
Model No.	Ton Bloto	Base		F ₁	F ₂	F ₁	F ₂		
NO.	Top Plate			lbs	lbs	lbs	lbs		
				kN	kN	kN	kN		
	D.Fir-L								
	2x4 Plate	6-10d	3-10d	735	445	145	470		
HTC4				3.27	1.98	0.65	2.09		
11104	2x6 Plate	6-10d	3-10d	910	465	265	460		
	2.01 late			4.05	2.07	1.18	2.05		
	S-P-F								
	2x4 Plate	6-10d	3-10d	530	315	105	340		
HTC4	ZX4 Flate	0-10u		2.36	1.40	0.47	1.51		
11104	2x6 Plate	6-10d	3-10d	650	330	190	330		
	2xo Piale		J-10u	2.90	1.47	0.85	1.47		

- Factored resistances have been increased 15% for wind or earthquake loading with no further increase allowed. Reduce where other load durations govern.
- Truss or rafter must be bearing on top plate to achieve factored resistances under "Without Gap."
- 3. Installed with maximum 1¼" space between rafter or truss and top plate, use values under "With 1¼" Gap." Where resistances are not required, space is not limited to 1¼".
- 4. NAILS: 10d = 0.148" dia. x 3" long. See pages 22-23 for other nail sizes and information





the connector, to allow vertical truss movement.

Nails should not be driven completely flush against

Typical HTC4 Installation on a 2x6 or larger Plate

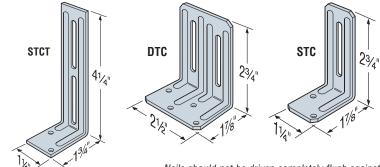
STC/STCT/DTC Roof Truss Clips

For alignment control between a roof truss and nonbearing walls; the $1\frac{1}{2}$ " slot permits vertical truss chord movement when loads are applied.

MATERIAL: 18 gauge. FINISH: Galvanized

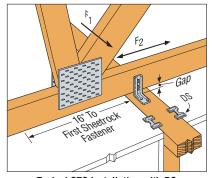
INSTALLATION: • Use all specified fasteners; see General Notes.

- Use STC or DTC depending on required resistances. STC, installed with Drywall Stop (DS), helps prevent fasteners tearing through the ceiling sheetrock (see illustration).
- Use STCT where truss or rafter is separated from the top plate of the nonbearing wall.
- Install slot nails in the middle of the slot.
- Not intended for floor applications.



Nails should not be driven completely flush against the connector, to allow vertical truss movement.

	Dimensions (in)		Fasteners		Factored Resistance (K _D =1.15)																
					D.Fir-L			S-P-F													
Model				ise Slot	Without Gap ²		1/4" Max Gap		Without Gap ²		1/4" Max Gap										
No.	No. Plate Vertical Base Leg	Vertical	Base		F ₁	F ₂	F ₁	F ₂	F ₁	F ₂	F ₁	F ₂									
		Leg	Баѕе		lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs									
					kN	kN	kN	kN	kN	kN	kN	kN									
STC	41/ y 47/	11/ y 93/	11/4 x 23/4 2-8d 1-80	1 04	155	85	70	60	110	60	50	45									
316	11/4 x 17/8 11/4 x 23/	174 X Z74		2-ou 1-ou	0.69	0.38	0.31	0.27	0.49	0.27	0.22	0.20									
STCT	11/4 x 13/4	1¼ x 4¼ 2-8d	11/ y 11/ 0	41/ 5/41/	11/ v /11/	11/ v /11/	41/ v 41/	11/ v /11/	41/ 5/41/	41/ 41/	41/ 41/	0.04	1-8d	_	_	_	_	_	_	_	_
3101	5161 1¼ X 1¾		2-ou 1-	1-0U	_	_		_	_	_	_	_									
DTC	DTC 2½ x 1% 2	214 v 23/	4-8d	4-8d 2-8d	240	395	155	250	170	280	110	175									
סוט		2 1/2 X 29/4	272 X 294	Z1/2 X Z9/4	21/2 X 29/4	21/2 X 29/4	Z1/2 X Z9/4	2½ X 2¾	2½ X 2¾	2½ x 2¾	2½ X 2¾	21/2 X 29/4 4-6	4-80	2-0u	1.07	1.76	0.69	1.11	0.76	1.25	0.49



Typical STC Installation with DS Allow 1/16" gap between nailhead and truss clip to help prevent squeaking

Factored resistances may not be increased for short-term loading.
 Truss or rafter must be bearing on top place to achieve the factore resistances under "Without Gap."

^{3.} Installed with maximum 1/4" space between rafter or truss and top plate under
W th 1/4 Gap !" Where resistances are not required, space is not mitted to 4".

4. No ILS: 8d = 0.131" dialy 21/2 long. See pages 22-23 for other nail sizes and information.

CHC Component Hoist Clip



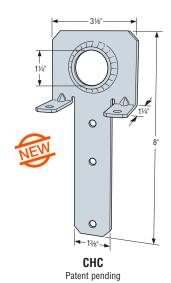
The CHC component hoist clip provides a tested, load-rated solution for the safe lifting and placement of assembled wood components. The CHC is load-rated with Strong-Drive® SDS Heavy-Duty Connector screws for easy installation and removal, and superior shear and withdrawal strength during lifting.

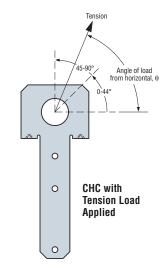
FEATURES:

- · Attaches easily to wood members using Strong-Drive SDS Heavy-Duty Connector screws (sold separately)
- May be used alone or in pairs for increased load
- · Tested in multiple load directions for versatility

MATERIAL: 12 gauge FINISH: Galvanized INSTALLATION:

- Use all specified fasteners. See General Notes.
- Fasteners require full penetration into the framing members.
- Use one time only.
- Lifting devices should be connected to the CHC with a closed-loop attachment of sufficient strength to carry the allowable load.





Single Part Safe Working Loads

Model	Faste	ners4	Angle from	D.Fir-L/ S-P-F	
No.	Тор	Face	Horizontal, θ		
CHC	2-1/4"x3" SDS	3-1/4"x3" SDS	0-44	610	
ОПО	2-74 X3 3D3	3-74 X3 3D3	45-90	975	

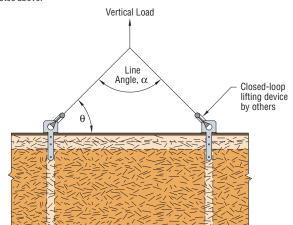
- 1. Safe working loads are based on the lowest ultimate test load of 3 test specimens, or the average of 6 specimens, divided by 5.
- 2. No load duration increase allowed.
- 3. Safe working loads are based on installation over sheathing on stud walls with double 2x top plates and max. %" sheathing.

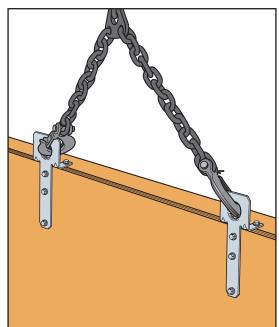
 4. Fasteners require full penetration into the framing members.
- 5. All lifting devices and spreader bars that are used in conjunction with the CHC shall be of sufficient strength to carry the required load. Spreader bars must also have sufficient rigidity to resist bending of the lifted component.

Safe Working Loads for Two Parts

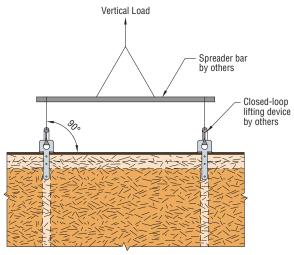
Model No.	Type of Connection	Angle from Horizontal, θ	Line Angle, α	D.Fir-L/ S-P-F
	1	30		610
CHC		45	90	1380
CHC		60	60	1690
	2	90	_	1950

See footnotes above.





Typical CHC Installation Using Two Parts



Plated Truss Connectors

Simpson Strong-Tie® hurricane ties provide a positive connection between truss/rafter and the wall of the structure to resist wind and seismic forces. New additions to the line provide even more options.

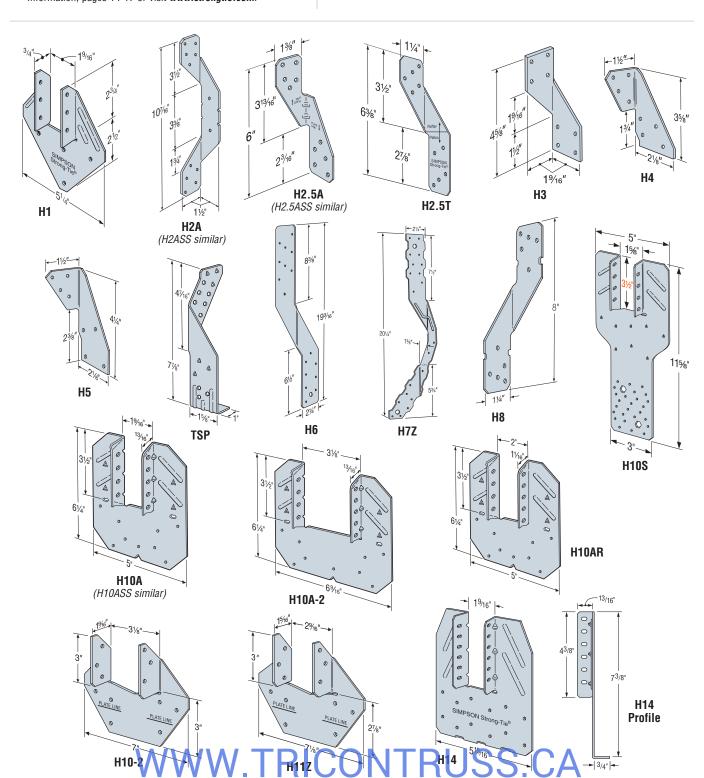
- H10AR The heavy-duty design of the H10A available with a 2" wide throat to accommodate rough lumber
- H10A-2 The H10A design with a 3" throat for double 2x members
- H2ASS, H2.5ASS and H10ASS Popular ties now available in stainless steel.

MATERIAL: See table.

FINISH: Galvanized. H7Z and H11Z—ZMAX® coating. Some models available in stainless steel or ZMAX; see Corrosion Information, pages 14-17 or visit www.strongtie.com.

INSTALLATION: • Use all specified fasteners. See General Notes.

- H1 can be installed with flanges facing inward (reverse of H1 installation drawing; number 1).
- H2.5T, H3, H4, H5 and H6 ties are shipped in equal quantities of right and left versions (right versions shown).
- · Hurricane ties do not replace solid blocking.
- When installing ties on plated trusses *(on the side opposite the truss plate)* do not fasten through the truss plate from behind. This can force the truss plate off of the truss and compromise truss performance.
- . H10A optional nailing to connect shear blocking, use 8d nails. Slots allow maximum field bending up to a pitch of 6:12, use H10A sloped loads for field bent installation.

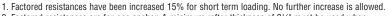


Catalogue C-C-CAN2015 © 2015 SIMPSON STRONG-TIE COMPANY INC.

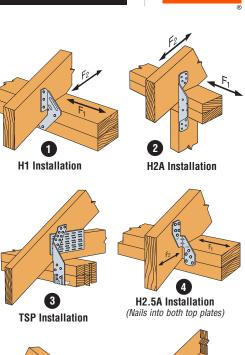


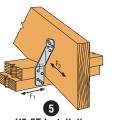
- These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.
- These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

			_	aatanara			Factore	d Resist	ance (K	D = 1.15)		
				asteners			D.Fir-L			S-P-F		
	Model	Ga				IImlif4	Late	eral	IImli44	Late	eral	
	No.	ua	To Pottoro/	To	To	Uplift	F ₁	F ₂	Uplift	F ₁	F ₂	
			Rafters/ Truss	Plates	Studs	lbs	lbs	lbs	lbs	lbs	lbs	
						kN	kN	kN	kN	kN	kN	
	114	10	C 0dv41/	4.04		740	685	300	680	485	215	
	H1	18	6-8dx1½	4-8d	_	3.29	3.05	1.33	3.02	2.16	0.96	
	H2A	18	E 0dv11/	0 0dv11/	5 0dv11/	830	220	75	590	155	55	
	IIZA	10	5-8dx1½	2-8dx1½	5-8dx1½	3.69	0.98	0.33	2.62	0.69	0.24	
	H2.5A	18	5-8d	5-8d		805	160	160	755	160	160	
	11Z.JA	10	J-6u	J-0u		3.58	0.71	0.71	3.36	0.71	0.71	
	H2.5T	18	5-8d	5-8d		835	175	210	740	160	210	
	112.01	10	3 00	J 00		3.71	0.78	0.93	3.29	0.71	0.93	
	НЗ	18	4-8d	4-8d	_	740	180	265	615	125	190	
	110	10	4-00	4-0u		3.29	0.80	1.18	2.74	0.56	0.85	
	H4	20	4-8d	4-8d	_	510	180	235	440	130	165	
	117	20	4 00	7 00		2.27	0.80	1.05	1.96	0.58	0.73	
	H5	18	4-8d	4-8d	_	685	180	305	500	130	215	
	110	10	7 00	7 00		3.05	0.80	1.36	2.22	0.58	0.96	
	Н6	16	_	8-8d	8-8d	1585	1085		1125	770	_	
	110	10		0-0u		7.05	4.83	_	5.00	3.43	_	
	H7Z	16	4-8d	2-8d	8-8d	1390	670		990	475	_	
			. 60	2 00	0 00	6.18	2.98	_	4.40	2.11	_	
	H8 ³	18	5-10dx1½	5-10dx1½	_	1120			1025		_	
			0 100/11/2	J-100X172			4.98	_	_	4.56	_	
	H10A ⁹	18	9-10dx1½	9-10dx1½	9-10dx1½	_	1735	795	410	1505	565	290
						7.72	3.54	1.82	6.69	2.51	1.29	
靈	H10AR	18	9-10d x 1½	9-10d x 1½	_	1485	690	430	1220	570	305	
9						6.61	3.07	1.91	5.43	2.54	1.36	
愈	H10A-2	18	9-10d x 1½	9-10d x 1½	_	1835	1275	430	1645	880	305	
						8.16	5.67	1.91	7.32	3.91	1.36	
	H10S ^{7,8}	18	8-8dx1½	8-8dx1½	8-8d	1465	795	315	1040	565	225	
						6.52	3.54	1.40	4.63	2.51	1.00	
	H10-2	18	6-10d	6-10d	_	1070	760	555	760	540	395	
						4.76	3.38	2.47	3.38	2.40	1.76	
	H11Z	18	6-16dx2½	6-16dx2½	_	1095	920	545	780	655	390	
						4.87	4.09	2.42	3.47	2.91	1.73	
		1 12-8dx1½	13-8d	_	2390	855	320	1805	610	230		
	H14				10.63	3.80	1.42	8.03	2.71	1.02		
		2 12-8dx1½	15-8d	_	2390	855	320	1805	610	230		
			[Z] 1Z-80X1½ 15-80		10.63	3.80	1.42	8.03	2.71	1.02		
			9-10dx1½	½ 6-10dx1½	_	1295	440		920	310		
	TSP	16				5.76	1.96		4.09	1.38	_	
			9-10dx1½	6-10d	_	1560	440		1105	310		
	9-10dx				6.94	1.96	_	4.92	1.38	_		



- 2. Factored resistances are for one anchor. A minimum rafter thickness of $2\,\%$ must be used when framing anchors are installed on the same side of the plate (exception: H2.5A).
- 3. H8 factored uplift resistances for stud-to-bottom plate installations are 595 lbs. (2.65 kN) for D.Fir-L and 390 lbs. (1.74 kN) for S-P-F.
- 4. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
- 5. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable (see General Instuctions for the Installer notes on pages 20-21). For a continuous load path, connections at the top and bottom of the wall must be on the same side of the wall (see technical bulletin T-HTIECONPATH).
- 6. Factored resistances in the F₁ direction are not intended to replace diaphragm boundary members or prevent cross grain bending of the truss or rafter members. Additional shear transfer elements shall be considered where there may be effects of cross grain bending or tension.
- 7. H10S can have the stud offset a maximum of 1" from the rafter (centre to centre) for a reduced uplift of 1435 lbs (6.38 kN) D.Fir-L and 1015 lbs (4.51 kN) S-P-F.
- 8. H10S nails to plates are optional for uplift but required for lateral loads
- 10. **NAILS:** 16dx2½ = 0.162" 8d = 0.131" dia. x $2\frac{1}{2}$ " long, $8dx1\frac{1}{2}$ = 0.131" dia. x $1\frac{1}{2}$ " long. See pages 22-23 for other



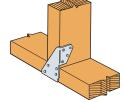


6 H2.5T Installation

H2.5T Installation (Nails into both top plates)



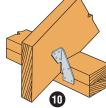




H3 Installation (Nails into upper top plate)

8 H4 Installation





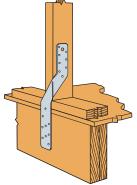
H4 Installation (Nails into upper top plate)

H5 Installation (Nails into both top plates)

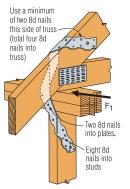
H/TSP Seismic & Hurricane Ties



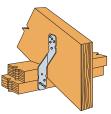
H6 Stud to Top Plate Installation



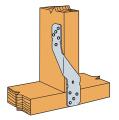
H6 Stud to Band Joist Installation



13 H7Z Installation



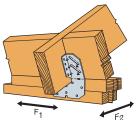
H8 attaching rafter to double top plates



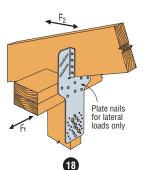
H8 attaching stud to sill (4-8d into plate, 5-8d into stud)



H8 attaching I-joist to double top plates

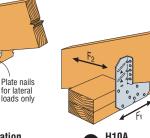


H10A Field-Bent Installation



H10SInstallation

H10S Installation with stud offset

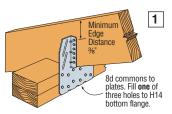


20 H10A Installation

H10A optional positive angle nailing connects shear blocking to rafter. Use 8d common nails. Slot allows maximum field-bending up to a pitch of 6/12, use 75% of the table uplift value; bend one time only.

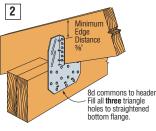


H10-2 Installation
(H11Z similar)

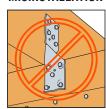


#14 Installation to double top plates





AVOID A MISINSTALLATION



Do not make your own holes or

Considerations for Hurricane Tie Selection

- 1. What is the uplift load?
- 2. What is the parallel-to-plate load?
- 3. What is the perpendicular-to-plate load?
- 4. What is the species of wood used for the rafter and the top plates? (Select the load table based on the lowest performing species of wood.)
- 5. Will the hurricane tie be nailed into both top plates or the upper top plate only?
- 6. What load or loads will the hurricane tie be taking?

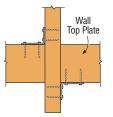
Factored resistances for more than one direction for a single connection cannot be added together. A design load which can be divided into components in the directions given must be evaluated as follows:

Factored Uplift / Uplift Resistance + Factored Parallel to Plate / Parallel to Plate Resistance + Factored Perpendicular to Plate / Perpendicular to Plate Resistance < 1.0.

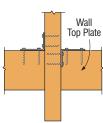
The three terms in the unity equation are due to possible directions that exist to generate force on a hurricane tie. The actual number of terms used in the equation for each condition is dependant on designer's method of calculating wind forces and the utilization of the tie in the structural system.

Hurricane Tie Installations to Achieve Twice the Capacity (Top View)

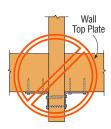
Both connectors shall be same model.



Install diagonally across from each other for minimum 2x truss.



Products can be on the same side of the wall provided they are configured as shown.



Nailing into both sides of a single ply 2x truss may cause the wood to split.

7. Select hurricane tie based on performance, application, installed cost and ease of installation.

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Seismic & Hurricane Ties



The H connector series provides wind and seismic ties for trusses and rafters. The presloped 5:12 seat of the H16 provides for a tight fit and reduced deflection. The strap length provides for various truss height up to a maximum of 131/2" (H16 series). Minimum heel height for H16 series is 4".

The H16-2 series has a presloped seat of 5:12, for double trusses.

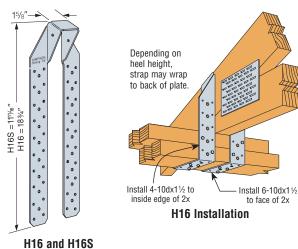
The HGA10 attaches to gable trusses and provides good lateral wind resistance. The HS24 attaches the bottom chord of a truss or rafter at pitches from 0:12 to 4:12 to double 2x4 top plates. Double shear nailing allows for higher lateral resistance.

MATERIAL: See table

FINISH: Galvanized. Some models available in stainless steel or ZMAX®; see Corrosion Information, pages 14-17.

INSTALLATION: • Use all specified fasteners. See General Notes.

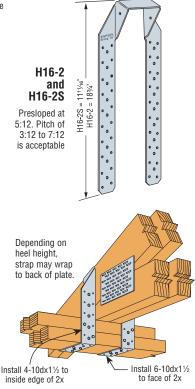
- HS24 requires slant nailing only when bottom chord of truss or rafter has no slope.
- · Hurricane Ties do not replace solid blocking.
- HGA10KT comes with SDS screws provided.

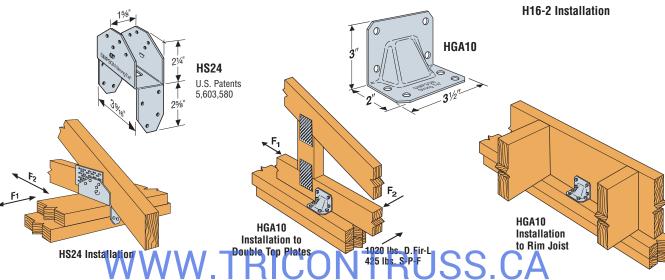


Presloped at 5:12. Truss/rafter pitch of 3:12 to 7:12 is acceptable

		Faster	ners		Factor	ed Resist	ance (K _D	= 1.15)	
					D.Fir-L			S-P-F	
Model	Ga	To	T-	Uplift	Lat	eral	111764	Lateral	
No.	ua	Rafters/	To Plates	Opini	F ₁	F ₂	Uplift	F ₁	F ₂
		Truss	1 10103	lbs	lbs	lbs	lbs	lbs	lbs
				kN	kN	kN	kN	kN	kN
HGA10KT ²	14	4-SDS 1/4"x11/2"	4-SDS 1/4"x3"	750	1604	1615	660	1410	1420
HGATUKT	14	4-5D5 74 X172	4-3D3 74 X3	3.34	7.14	7.19	2.94	6.28	6.32
HS24 ⁴	18	8-8dx1½	8-8d	1145	1210	1600	805	860	1135
П324	10	& 2-8d slant		5.10	5.38	7.12	3.59	3.83	5.05
H16	18	0.404541/	10 10dv11/	1870	_	_	1330	_	_
піо	10	2-10dx1½	10-10dx1½	8.32	_	_	5.92	_	_
H16S	18	0.404541/	10-10dx1½	1870	_	_	1330	_	_
піоъ	10	2-10dx1½	10-100X172	8.32	_	_	5.92	_	_
1146.0	18	0.404541/	10.10dv11/	1870	_	_	1330	_	_
H16-2	10	2-10dx1½	10-10dx1½	8.32	_		5.92		_
H16-2S	18	2-10dx1½	10-10dx1½	1870	_	_	1330	_	_
П10-25	10	Z-10UX 1 72	10-100X172	8.32	_	_	5.92	_	_

- 1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
- 2. Factored F2 resistances shown are for loading applied into the connector. For loading applied away from the connector the factored resistances are 1020 lbs (4.54 kN) for D.Fir-L and 425 lbs (1.89 kN) for S-P-F.
- 3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
- 4. HS24 factored resistances without slant nailing are 885 lbs (3.94 kN) D.Fir-L and 630 lbs (2.80 kN) S-P-F for uplift, 985 lbs (4.38 kN) D.Fir-L 700 lbs (3.11 kN) S-P-F for F₁, 930 lbs (4.14 kN) D.Fir-L and 655 lbs (2.91 kN) S-P-F for F₂.
- 5. **NAILS**: $10dx1\frac{1}{2} = 0.148$ " dia. $x \, 1\frac{1}{2}$ " long, 8d = 0.131" dia. $x \, 2\frac{1}{2}$ " long, $8dx1\frac{1}{2} = 0.131$ " dia. $x \, 1\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.





The LGT, MGT and VGT are girder tiedowns for moderate to high load applications. The LGT and VGT are also suitable for retrofit applications.

LGT connectors provide a low profile connection to the studs for easy installation of drywall. Simple to install and can be installed on the inside or outside of the wall.

The Variable Girder Tiedown (VGT) is a higher capacity alternative to the LGT and MGT for girder trusses. It attaches with Strong-Drive® SDS Heavy-Duty Connector screws to the side of truss and features a predeflected crescent washer that allows it to accommodate top chord pitches up to 8:12. The VGT is also available with one flange concealed for attachment to trusses with no overhang.

MATERIAL: VGT-7 gauge, LGT2-14 gauge, MGT, LGT3, LGT4-12 gauge FINISH: Galvanized

INSTALLATION: • Before installing fasteners, ensure LGT3-SDS2.5 makes complete contact with bottom of truss.

- Strong-Drive SDS Heavy-Duty Connector screws included with LGT3, LGT4 and VGT series.
- Strong-Drive SDS Heavy-Duty Connector screws driven through truss plates must be approved by the Truss Designer. Pre-drilling using a 5/32" bit is required.
- VGT—Can be installed on roof pitches up to 8/12 or on a bottom chord designed to transfer the load.
- VGT—Screw holes are configured to allow for double installation on a two-ply (minimum) truss.
- VGT—The product can be installed in a single application or in pairs
- to achieve a higher uplift capacity.

 VGT—When installed on trusses with no overhangs, specify VGTR/L.
- VGT—Install washer component (provided) so that top of washer is horizontal as well as parallel with top of wall top plate.

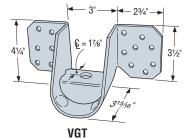
 • LGT3-SDS2.5 and LGT4-SDS3—The four large hexagon holes are
- intended for CMU and concrete applications.
- · See page 204 for masonry applications.

OPTIONS: LGT3 and LGT4 are available with reduced widths of W = 413/16" and W = 63/8" — order as LGT3N-SDS2.5 and LGT4N-SDS3.

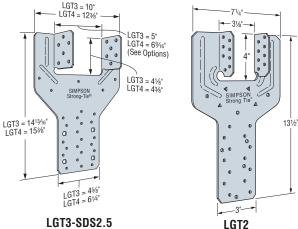
	No of		Fa	steners	Factored Resistance (K _D = 1.15)		
Model No.	Qty.	No. of Plies		0.1	D.Fir-L	S-P-F	
NU.		LIIES	Studs or Anchor	Girder Truss	lbs	lbs	
			Allollol	11455	kN	kN	
LGT2	1	2 ply 14-10d 16-10d		3670	2605		
LG12	G12 1 1		14-10d	10-100	16.33	11.59	
LGT3-SDS2.5	1	3 ply	26-10d	12-1/4"x21/2" SDS	6415	4930	
LG13-3D32.3	'	S ply	20-10u	12-74 X272 3D3	28.54	21.93	
LGT4-SDS3	1	4 ply	30-10d	16-1/4"x3" SDS	6030	3980	
LG14-3D33	'	4 piy	30-10u	10-74 X3 3D3	26.82	17.70	
MGT	1	2 ply min.	1_5/-" Dia	-%" Dia. 22-10d	5610	3985	
IVIGT	'	Z pry min.	1-78 Dia.	22-10u	24.96	17.73	
	1	2 ply min.	1-%" Dia.	16-1/4"x3" SDS	8600	6195	
VGT	'	Z pry min.	1-78 Dia.	10-74 X0 303	38.26	27.56	
VGT	2	2 ply min.	2-%" Dia.	32-1/4"x3" SDS	11690	8420	
		Z piy iiiii.	2-78 Dia.	32-74 X3 3D3	52.00	37.46	
	1	2 ply min	1.5/ " Dio	16-1/4"x3" SDS	3475	2505	
VGTR/L	'	2 ply min. 1-5/8" Dia.		10-74 X3 3D3	15.46	11.14	
VOIN/L	2	2 ply min.	2-%" Dia.	32-1/4"x3" SDS	6950	5010	
		Z PIY IIIIII.	2-78 Did.	32-74 83 303	30.92	22.29	

- 1. Attached members must be designed to resist the factored loads.
- Factored resistances have been increased 15% for uplift with no further increase allowed. Reduce where other loads govern.

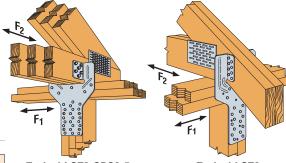
 Additional analysis and account of the second of the se
- 3. Additional anchorage products to be designed by others.4. MGT can be installed with straps vertical for full
- table load provided 26-10d nails are installed to either a solid header or minimum double 2x6 web.
- 5. LGT3-SDS2.5-F₁ factored resistances are 1335 lbs (5.94 kN) for D.Fir-L and 945 lbs (4.20 kN) for S-P-F. F₂ factored resistances are 670 lbs (2.98 kN) for D.Fir-L and 475 lbs (2.11 kN) for S-P-F.
- 6. LGT2—F₁ factored resistances are 1170 lbs (5.20 kN) for D.Fir-L and 830 lbs (3.69 kN) for S-P-F. F₂ factored resistances are 285 lbs (1.27 kN) for D.Fir-L and 200 lbs (0.89 kN) for S-P-F.
- 7. NAILS: 10d = 0.148" dia. x 3" long. See pages 22-23 for other nail sizes and



Factored Decistores



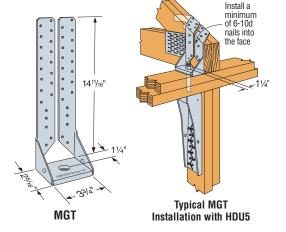
(LGT4-SDS3 similar)

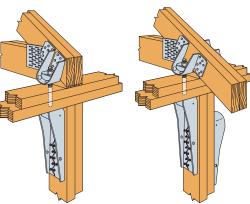


Typical LGT3-SDS2.5 Installation

Typical LGT2 Ínstallation

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Typical VCTR Single Installation with HDU4 Typical VGT Double Installation with HDU5s

HGT Heavy Girder Tiedowns

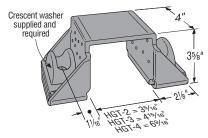
The HGT - Heavy Girder Tie-Down offers the highest uplift capacity for girders and can be installed on trusses and beams with top chord slopes from 3:12 to 8:12.

MATERIAL: 7 gauge

FINISH: Simpson Strong-Tie® gray paint

INSTALLATION:

- Install two LBP% washers on top of each crescent washer. LBP5% washers are not included with HGT and must be ordered separately. Crescent washers come with the HGT.
- Anchorage from HGT to holdown below shall be with %" diameter ASTM A307 Grade A bolts or threaded rod.
- · See page 205 for masonry or concrete installations.



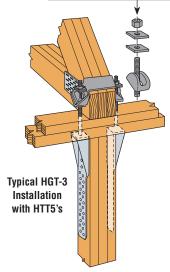
Install two LBP%" washers on top of each crescent washer (total four 5/8" washers) for wood installation. All washers and crescent washers are required. Crescent washers are supplied.

HGT-2 (HGT-3, HGT-4 similar)

		No. of		O.C. Dimension	Faste	eners	Factored Uplift Resistance $(K_D = 1.15)$		
Model No.	Qty.	No. of Plies	Between			D.Fir-L	S-P-F		
NU.		LIIES	Anchors (in)	Anchor Bolts	Girder Truss	lbs	lbs		
			(111)	Duits IIuss		kN	kN		
HGT-2	4	2 ply	511/16	0 6/11 1	16-10d	12140	9280		
пит-2	ı	2 ply	3.716	2- ⁵ / ₈ " φ	10-100	54.00	41.28		
HGT-3	1	2 ply	75/16	0.5/11.1	16-10d	12140	9280		
пит-з	'	3 ply	1 916	2-⅓" φ	10-100	54.00	41.28		
HGT-4	4	4 ply	9	0 6/11 1	16 104	12140	9280		
ПС 1-4		4 ply	9	2-5/8" φ 16-10d		54.00	41.28		



- 2. Attached members must be designed to resist the applied loads.
- 3. Anchorage must be designed by others.
- 4. NAILS: 10d = 0.148" dia. x 3" long. See pages 22-23 for other nail sizes and information.



HTSQ Twist Strap

The new HTSQ twist straps provide a tension connection between two wood members and are designed to resist uplift for decks, boardwalks and beams economically. The HTSQ is quicker, easier and more economical to install when compared to bolted straps. HTSQ provides a strong connection with fewer fasteners than nailed HTS straps when incorporating Simpson Strong-Tie® Strong-Drive® SDS Heavy-Duty Connector screws. Strong-Drive SDS Heavy-Duty Connector screws with a double-barrier coating finish are included with HTSQ straps in a ZMAX® coating. For stainless-steel HTSQ straps, stainless-steel Strong-Drive SDS Heavy-Duty Connector screws are provided.

FEATURES:

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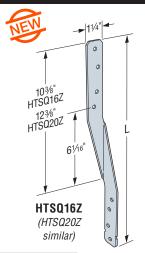
- · Quicker and more consistent installation than bolts
- · Better fastener withdrawal resistance than nails
- · Corrosion resistance finish options

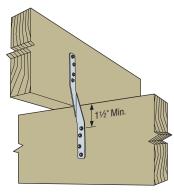
MATERIAL: 14 gauge

FINISH: ZMAX® coating or stainless steel.

INSTALLATION:

· Use all specified fasteners.





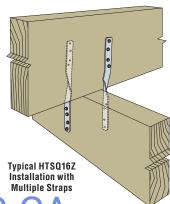
Typical HTSQ16Z Installation

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

				Factored Resistance						
				D.F	ir-L	S-P-F				
	Model No.	(in.)	Fasteners Total	(K _D =1.00)	(K _D =1.15)	$(K_D=1.00)$	(K _D =1.15)			
	NU.	(111.)	Iutai	lbs	lbs	lbs	lbs			
				kN	kN	kN	kN			
	HTSQ16Z-SDS	16	8-1/4"x11/2" SDS	1500	1725	1215	1395			
-4	H190102-3D3	10	0-74 X 172 3D3	1.36	1.56	0.96	1.09			
	HTSQ20Z-SDS	20	8-1/4"x11/2" SDS	1500	1725	1370	1575			
	11134202-303	20	0-74 X 172 3D3	2.36	2.71	2.16	2.47			

- 1. Factored resistances have been increased 15% for short term load duration. No further increase is permitted.
- 2. Install four fasteners into each member to achieve full capacity. HTSQ20 has two (2) extra holes per side to allow for installation flexibility





TS/LTS/MTS/HTS Twist Straps

SIMPSON

Twist straps provide a tension connection between two wood members. They resist uplift at the heel of a truss economically. The 3" bend section eliminates interference at the transition points between wood members. TS twist straps come with an equal number of left and right hand units in each carton.

MATERIAL: LTS-18 gauge; MTS-16 gauge; HTS-14 gauge; TS-16 gauge FINISH: Galvanized. Some products available in stainless steel and ZMAX®; see Corrosion Information, pages 14-17.

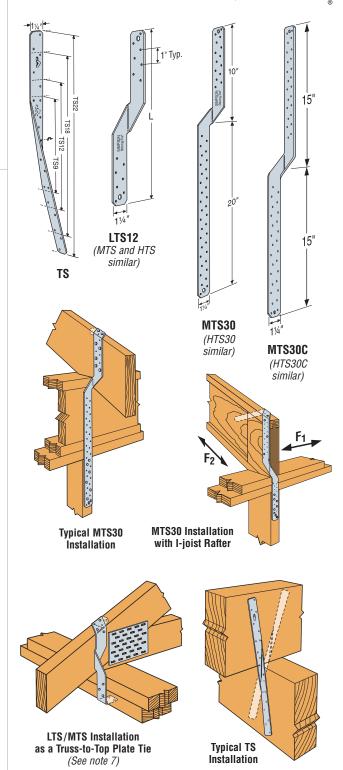
INSTALLATION: Use all specified fasteners. See General Notes.

- TS should be installed in pairs to reduce eccentricity.
- When LTS/MTS is installed as Truss-to-Top Plate tie, install 3-10dx11/2" nails to the u/s of the plate and 3-10dx1½" nails into the edge of the double top plate.
- These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.
- These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

Factored Resistance

 $(K_D = 1.15)$

Madel		Factoners	(KD =	1.15)
Model No.	L (in)	Fasteners (Total)	D.Fir-L	S-P-F
140.	("")	(Total)	lbs	lbs
			kN	kN
TS9	9	0.164	1125	1040
159	9	8-16d	5.00	4.63
TS12	11%	10-16d	1410	1300
1012	1178	10-100	6.27	5.78
TS18	17¾	14-16d	1970	1820
1010	17.74	14 100	8.76	8.10
TS22	21%	18-16d	2125	2125
1022	2170	10 100	9.45	9.45
LTS12	12	12-10dx1½	1015	720
		12 10ux172	4.52	3.20
LTS16	16	12-10dx1½	1015	720
			4.52	3.20
LTS18	18	12-10dx1½	1015	720
			4.52	3.20
LTS20	20	12-10dx1½	1015	720
		1 1 1 2 2 1 1 1 1 1	4.52	3.20
MTS12	12	14-10dx1½	1570	1180
			6.98	5.25
MTS16	16	14-10dx1½	1570	1180
			6.98	5.25
MTS18	18	14-10dx1½	1570	1180
			6.98 1570	5.25 1180
MTS20	20	14-10dx1½	6.98	5.25
			1570	1180
MTS30	30	14-10dx1½	6.98	5.25
			1570	1180
MTS24C	24	14-10dx1½	6.98	5.25
			1570	1180
MTS30C	30	14-10dx1½	6.98	5.25
			2050	1455
HTS16	16	16-10dx1½	9.12	6.47
LITOCO	00	0440144	2050	1455
HTS20	20	24-10dx1½	9.12	6.47
117004	0.4	04.40.4.41/	2050	1455
HTS24	24	24-10dx1½	9.12	6.47
HTS28	28	24-10dx1½	2050	1455
171020		24-10UX172	9.12	6.47
HTS30	S30 30	24-10dx1½	2050	1455
111000	30	24-10UX1/2	9.12	6.47
HTS30C	30	24-10dx1½	2050	1455
1110000	00	17777	9.12	6.47
		vv vv	V V _ 1	いしん



- 1. LTS12 thru LTS20, MTS16 through MTS30, HTS24 through HTS30C (except HTS30) have additional nail holes.
- 2. Install half of the fasteners on each end of strap to achieve maximum factored resistance.
- 3. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed: reduce where other loads govern.
- 4. All straps except the MTS30 and HTS30 have the twist in the centre of the strap.
- Twist straps do not have to be wrapped over the truss to achieve the load.
- 6. Optional nail holes are provided on some straps.
- 7. When used as a truss-to-top plate tie multiply the tabulated values by 0.95 for LTS and 0.74 for MTS. HTS cannot be used in this application.
- NAILS: 16d=0.162 dia. x 3% long. 10dx1%=0.148 dia. x 1% long. See pages 22-23 for other noil sizes and information

CS/CMST/CMSTC Coiled Straps



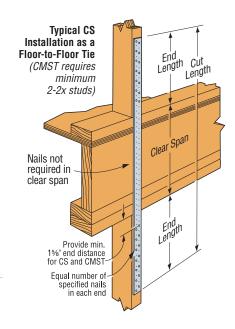
CMSTC provides nail slots for easy installation and coined edges for safe handling. CS are continuous utility straps which can be cut to length on the job site. Packaged in lightweight (about 40 pounds) cartons.

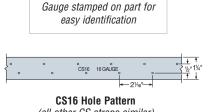
FINISH: Galvanized. Some products available in ZMAX® coating; see Corrosion Information, pages 14-17.

INSTALLATION:

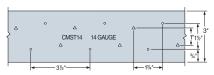
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- Use all specified fasteners. See General Notes.
- · Wood shrinkage after strap installation across horizontal wood members may cause strap to buckle outward.
- Refer to the applicable code for minimum nail penetration and minimum wood edge and end distances.
- . The table shows the maximum factored resistances and the nails required to obtain them. Fewer nails may be used; reduce the factored resistance as shown in footnotes.
- · CMST only-Use every other triangle hole if the wood tends to split. Use round and triangle holes for comparable MST loads, providing wood does not tend to split.
- For lap slice and alternate nailing information, request to technical bulletin T-CMST.

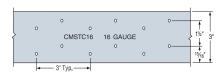








CMST14 Hole Pattern (CMST12 similar)



CMSTC16 Hole Pattern

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

			Dimensio				Factored Tens	ile Resistance	
			Dillielisio	JIIS		D.F	ir-L	S-I	P-F
Model No.	Ga	Total Coil	End	Cut	Fasteners (Total)	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$
		Length Length (in) (in)		(Total)	lbs	lbs	lbs	lbs	
		(tt)	(in)	(in)		kN	kN	kN	kN
CS22	22	300	10	clear span + 20	16-8d	1140	1140	1075	1140
0322	22	300	10	Ciear Spair + 20	10-0u	5.07	5.07	4.78	5.07
CS20	20	250	12	clear span + 24	18-8d	1390	1390	1295	1390
0320	20	230	12	Cieai Spail + 24	10-0u	6.18	6.18	5.76	6.18
CS18	18	200	12	clear span + 24	20-8d	1745	1850	1620	1850
0310	10	200	12	Cieai Spail + 24	20-0u	7.76	8.23	7.21	8.23
CS16		150	14	clear span + 28	24-8d	2305	2305	2155	2305
0310	16	130	14	Ciear Spair + 20	24-0u	10.25	10.25	9.59	10.25
CMSTC16	10	54	20	clear span + 40	46-10d	5685	5845	5195	5845
CIVISTOTO		34	20	Ciear Spair + 40	40-10u	25.29	26.00	23.11	26.00
CS14		100	22	clear span + 44	34-8d	3360	3360	3090	3360
0314	14	100	22	Cicai Spail + 44	34-0u	14.95	14.95	13.75	14.95
CMST14	14	52½	31	clear span + 62	66-10d	8430	8430	7455	8430
GIVIST 14		J2 //2	ان	Gidai Spail + 02	00-100	37.50	37.50	33.16	37.50
CMST12	12	40	43	clear span + 86	94-10d	11995	11995	10615	11995
GIVIO 1 12	12	40	40	Glear Spair + 00	34-10u	53.36	53.36	47.22	53.36

- 1. Factored resistances shown are the lesser of the steel tensile strength (T_r) or the lateral nail value (N_r).
- 2. Use half of the required nails in each member being connected to achieve the listed resistances.
- No. of Nails Used x Table Value 3. Calculate the connector value for a reduced number of nails as follows: Factored Resistance No. of Nails in Table

Example: CS14 on D.Fir-L with 30 nails total. (Half of the nails in each member being connected)

30 Nails (Used) Factored Resistance = 34 Nails (Table)

x 3360 lbs = 2965 lbs

4. NAILS: 10d = 0.148" dia. >

SIMPSON
Strong-Tie

HRS, a heavy 12 gauge strap tie, provides greater support in construction and repair of home projects. Straight lines and chamfered edges for better appearance. The MSTC series has countersunk nail slots for a lower nailing profile. Coined edges ensure safer handling.

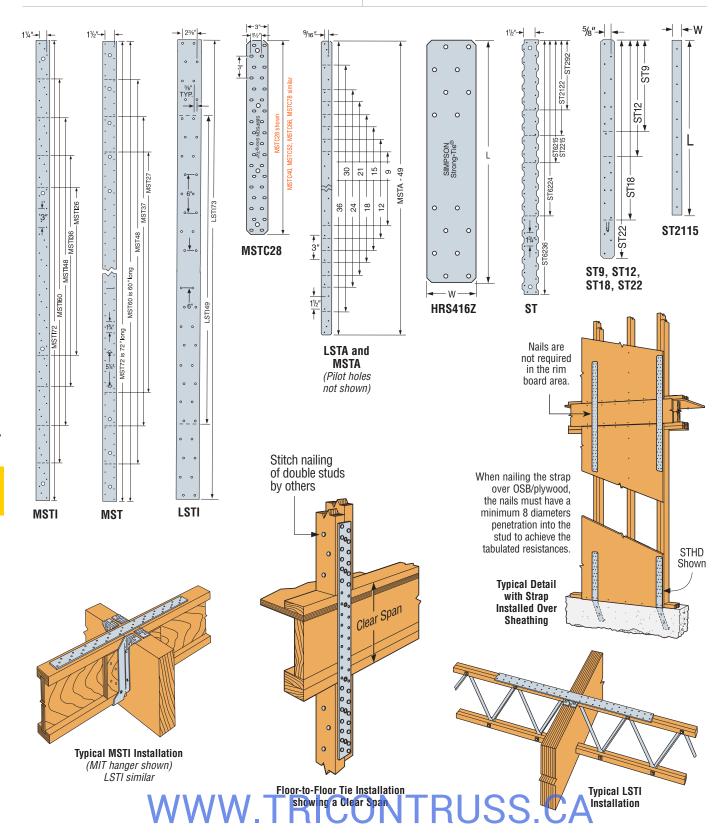
Install strap ties where wall plates are cut, at wall intersections, and as ridge ties. LSTA and MSTA straps are engineered for use on 1½" members. The 3" centre-to-centre nail spacing reduces the possibility of splitting. The LSTI light strap ties are suitable where gun-nailing is necessary through

diaphragm decking and wood chord open web trusses.

FINISH: HST—Simpson Strong-Tie® gray paint; PS—HDG;
all others—galvanized. Some products are available in stainless
steel or ZMAX® coating; see Corrosion Information, pages 14-17.

INSTALLATION: Use all specified fasteners. See General Notes.

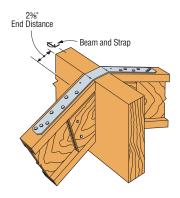
OPTIONS: Special sizes can be made to order.
Contact Simpson Strong-Tie for longer lengths.



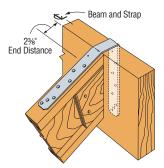


- These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.
- These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

Model	Co		(111)	Fasteners				P-F
No.	ua	w	L	(Total)	(ND = 1.00)	(KD = 1.10)	(K _D = 1.00)	(K _D = 1.15)
		VV	L		kN	kN	kN	kN
					600	690	555	635
LSTA9		11/4	9	6-10d	2.67	3.07	2.47	2.82
1.07140		447	40	0.40.1	800	920	735	845
LSTA12		11/4	12	8-10d	3.56	4.09	3.27	3.76
LSTA15	1	11/4	15	10-10d	1000	1150	920	1060
LOTATO		1 /4	10	10-100	4.45	5.12	4.09	4.72
LSTA18		11/4	18	12-10d	1200	1380	1105	1270
					5.34	6.14	4.92	5.65
LSTA21		11/4	21	14-10d	1400 6.23	1610 7.16	1290 5.74	1485 6.61
	20				1600	1840	1475	1695
LSTA24		11/4	24	16-10d	7.12	8.19	6.56	7.54
ОТООО		01/	05/	0.04	585	675	535	615
ST292		21/16	95/16	8-8d	2.60	3.00	2.38	2.74
ST2122		21/16	1213/16	12-8d	940	1085	865	995
012122		£/1b	16 / lb	12 UU	4.18	4.83	3.85	4.43
ST2115		3/4	165/16	8-8d	670	770	615	710
					2.98 1335	3.43 1540	2.74 1235	3.16 1420
ST2215		21/16	165/16	16-8d	5.94	6.85	5.49	6.32
					2235	2465	2075	2385
LSTA30		11/4	30	20-10d	9.94	10.97	9.23	10.61
LOTAGO		417	00	04.40.4	2465	2465	2465	2465
LSTA36		11/4	36	24-10d	10.97	10.97	10.97	10.97
LSTI49		3¾	49	32-10dx1½	3115	3580	2852	3280
L01143		J /4	43	32-10ux172	13.86	15.93	12.69	14.59
LSTI73		3¾	73	48-10dx1½	4670	5370	4280	4920
					20.77	23.89	19.04	21.89
MSTA9	18	11/4	9	6-10d	670 2.98	770 3.43	625 2.78	715 3.18
		18				895	1030	830
MSTA12		11/4	12	8-10d	3.98	4.58	3.69	4.25
MOTALE		41/	15	10 104	1120	1285	1040	1195
MSTA15		11/4	15	10-10d	4.98	5.72	4.63	5.32
MSTA18		11/4	18	12-10d	1340	1545	1245	1430
- 100 17110		174	10	12 100	5.96	6.87	5.54	6.36
MSTA21		11/4	21	14-10d	1565	1800	1455 6.47	1670 7.43
					6.96 1790	8.01 2060	1660	1910
MSTA24		11/4	24	16-10d	7.96	9.16	7.38	8.50
MOTAGO		417	0.0	00.404	2470	2840	2260	2595
MSTA30		11/4	30	20-10d	10.99	12.63	10.05	11.54
MSTA36		11/4	36	24-10d	2965	3070	2710	3070
WIGTAGO		1/4	00	∠¬ IVU	13.19	13.66	12.06	13.66
MSTA49		11/4	49	28-8d	2725	2725	2545	2725
					12.12	12.12	11.32	12.12
ST6215		21/16	165/16	16-8d	1405 6.25	1615 7.18	1300 5.78	1500 6.67
07225		611	6051	64.6:	2305	2650	2155	2475
ST6224	16	21/16	235/16	24-8d	10.25	11.79	9.59	11.01
ST9		11/4	9	6-8d	525	605	490	560
פוט		1 74	J	u-ou	2.34	2.69	2.18	2.49
ST12		11/4	11%	8-8d	700	805	650	750
					3.11	3.58	2.89	3.34
ST18		11/4	17¾	12-8d	1050 4.67	1210 5.38	975 4.34	1125 5.00
	-				1580	1790	1465	1685
ST22		11/4	21%	18-8d	7 3	796	6.52	7.50



Typical LSTA Installation (hanger not shown) Bend strap one time only



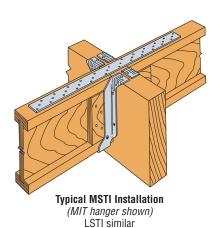
Typical LSTA Installation (hanger not shown) Bend strap one time only

- Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
- Use half of the nails in each member being connected to achieve the listed resistances.
 For overlap splice details, refer to T-CMST.
- 4. NAILS: 10d = 0.148" dia. x 3" long, 10dx1½ = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long. See pages 22-23 for other nail sizes and information.



- These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.
- These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

		Dime	nsions		F	actored Tens	ile Resistano	e
			n)		D.F	ir-L	S-I	P-F
Model No.	Ga			Fasteners	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$
NU.		w	L	(Total)	lbs	lbs	lbs	lbs
					kN	kN	kN	kN
MOTODO			001/	00.40.1	3955	4545	3615	4155
MSTC28		3	281/4	32-10d	17.59	20.22	16.08	18.48
MCTC40	16	3	401/4	48-10d	5930	6820	5420	6235
MSTC40	16	3	401/4	48-100	26.38	30.34	24.11	27.74
MSTC52	1	3	521/4	54-10d	6670	6940	6100	6940
W151652		3	3274	54-100	29.67	30.87	27.14	30.87
MSTC66		3	65¾	66-10d	8515	8565	7455	8565
IVISTUOO		٥	0574	00-10u	37.88	38.10	33.16	38.10
MSTC78	14	3	77¾	66-10d	8515	8565	7455	8565
IVIOTOTO	14		1174	00-10u	37.88	38.10	33.16	38.10
ST6236		21/16	3313/16	36-8d	3735	4295	3270	3760
310230		2710 00 716		30-0u	16.61	19.11	14.55	16.73
MSTI26		21/16	26	22-10dx1½	2825	3250	2475	2850
10101120	<u>'</u>	2/16	20	22 100X172	12.57	14.46	11.01	12.68
MSTI36		21/16	36	32-10dx1½	4110	4725	3600	4140
WIOTIOO		2710	00	02 100X172	18.28	21.02	16.01	18.42
MSTI48		21/16	48	44-10dx1½	5650	6500	4955	5695
WIGHTIG		2710	10	111000172	25.13	28.91	22.04	25.33
MSTI60		21/16	60	56-10dx1½	7195	7360	6305	7250
					32.01	32.74	28.05	32.25
MSTI72	12	21/16	72	68-10dx1½	7360	7360	7240	7360
					32.74	32.74	32.21	32.74
MST27		21/16	27	26-8d	2685	3090	2355	2710
					11.94	13.75	10.48	12.06
MST37		21/16	37½	38-8d	3930	4515	3440	3960
_					17.48	20.08	15.30	17.62
MST48		21/16	48	50-8d	5170	5945	4530	5210
	18				23.00	26.45	20.15	23.18
HRS416Z		31/4	16	16-SDS 1/4"x11/2"	2400	2760	2120	2440
					10.68	12.28	9.43	10.85
MST60		21/16	60	64-8d	6620	7610	5800	6670
	10				29.45	33.85	25.80	29.67
MST72		21/16	72	78-8d	8065	9135	7065	8125
					35.88	40.64	31.43	36.14

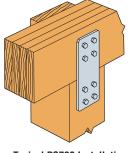


1. Factored resistances have been increased 15% for earthquake or wind loading with

- no further increase allowed.

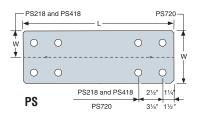
 2. Use half of the nails in each member being connected to achieve the listed resistances.
- 3. For overlap splice details, refer to T-CMST.
- 4. NAILS: 10d = 0.148" dia. x 3" long, 10dx1½ = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long. See pages 22-23 for other nail sizes and information.

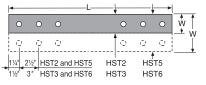
			Dimensi	ions (in)		T _r ¹
	Model No.	Ga	w	L	Fasteners (Total)	lbs
	110.		VV	_	(Total)	kN
	PS218		2	18	4-¾" MB	8315
	F 32 10			10	4-74 IVID	36.99
	PS418		4	18	4-¾" MB	21325
	F3410		4	10	4-74 IVID	94.86
	PS720	7	63/4	20	8-1⁄2" MB	35985
	F3720	′	074	20	0-72 IVID	160.08
	HST2		21/2	211/4	6-5%" MB	12670
	11012		Z 72	2174	0-78 IVID	56.36
	HST5		5	211/4	12-5/8" MB	25375
4	11010		J	21/4	12-78 IVID	112.88
	HST3		3	251/4	6-¾" MB	20520
	11013	3	3	2374	U-74 IVID	91.28
	HST6	3	6	9514	_ 10.3/" MD_	41035
	потб		0	251/4	12-3⁄4" MB	182 54



Typical PS720 Installation

 T_r is the factored tensile resistance of the strap in accordance with CSA S136-12. The capacity of the strap, used in a connection, must be verified by the Designer using the lower of the strap capacity or the factorer range it may the applicable.



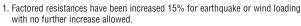




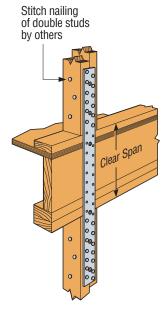
These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

Floor-to-Floor Clear Span Table

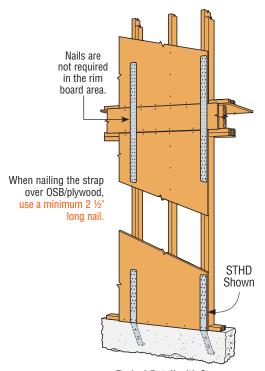
				Factored Tens	ile Resistance	
	Clear	F4	D.F	ir-L	S-I	P-F
Model No.	Span	Fasteners (Total)	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$	(K _D = 1.15
	(in)	(Total)	lbs	lbs	lbs	lbs
			kN	kN	kN	kN
	10	20.04	2725	2725	2725	2725
MSTA49	16	38-8d	12.12	12.12	12.12	12.12
IVISTA49	18	36-8d	2725	2725	2725	2725
	10	30-ou	12.12	12.12	12.12	12.12
	16	12-10d	1480	1705	1355	1560
MSTC28	10	12-100	6.58	7.58	6.03	6.94
10101020	18	8-10d	990	1135	905	1040
	10	0-100	4.40	5.05	4.03	4.63
•	16	28-10d	3460	3980	3160	3635
MCTCAO	10	20-10u	15.39	17.70	14.06	16.17
MSTC40	18	04.104	2965	3410	2710	3115
	10	24-10d	13.19	15.17	12.06	13.86
	16	44.104	5435	6250	4970	5715
MCTCEO	16	44-10d	24.18	27.80	22.11	25.42
MSTC52	10	40.404	4940	5685	4515	5195
	18	40-10d	21.98	25.29	20.08	23.11
	10	60 104	7740	8565	6775	7790
MCTCCC	16	60-10d	34.43	38.10	30.14	34.65
MSTC66	10	00 104	7740	8565	6775	7790
	18	60-10d	34.43	38.10	30.14	34.65
	16	66 104	8515	8565	7455	8565
MCTCZO	10	66-10d	37.88	38.10	33.16	38.10
10101010	18	66-10d	8515	8565	7455	8565
	10	00-100	37.88	38.10	33.16	38.10
	10	00.04	2065	2375	1810	2085
MCTOT	16	20-8d	9.19	10.56	8.05	9.27
IVI 5 1 3 /	18	18-8d	1860	2140	1630	1875
	10	10-ou	8.27	9.52	7.25	8.34
	10	20.04	3310	3805	2900	3335
MCTAO	16	32-8d	14.72	16.93	12.90	14.84
IVIS 140	10	3U 04	3100	3570	2720	3125
	18	30-8d	13.79	15.88	12.10	13.90
	10	16 04	4755	5470	4170	4795
MCTEO	16	46-8d	21.15	24.33	18.55	21.33
UOIGIVI	10	44.04	4550	5235	3985	4585
	18	44-8d	20.24	23.29	17.73	20.40
	10	60.04	6205	6520	5435	6250
MCT70	16	60-8d	27.60	29.00	24.18	27.80
IVIO I / Z	10	E0 04	6000	6520	5255	6045
MSTC78 MST37 MST48 MST60	18	58-8d	26.69	29.00	23.38	26.89



^{2.} Use half of the required nails in each member being connected to achieve the listed resistances.



Floor-to-Floor Tie Installation showing a Clear Span



Typical Detail with Strap Installed Over Sheathing



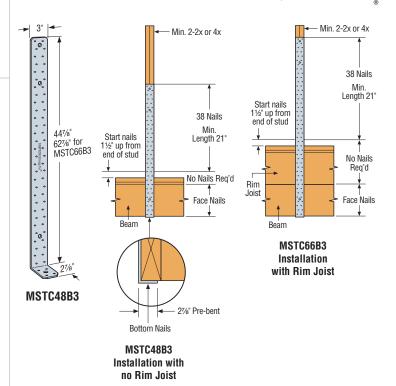
^{3.} When nailing the strap over OSB/plywood, use a minimum 2 1/2" long nail.

^{4.} NAILS: 10d=0.148" dia. x 3" long, $10dx1\frac{1}{2}=0.148"$ dia. x $1\frac{1}{2}"$ long. See pages 22-23 for other nail sizes and information.

MATERIAL: 14 gauge FINISH: Galvanized

Model	Minimum Beam Size (in)		F	astene	rs	Factored Tensile Resistance (K _D = 1.15)		
No.			Beam			D.Fir-L	S-P-F	
	Width	Depth	Face Bottom		Studs/ Post	lbs	lbs	
			гасе	DULLUIII		kN	kN	
MCTC40D0	3	91/4	10 104	4 104	38-10d	5440	3860	
MSTC48B3	٥	974	12-100	12-10d 4-10d		24.20	17.17	
MSTC66B3	31/2	111/4	14-10d	4-10d	38-10d	5230	3715	
IVIOIODDS	3 7/2	1174	14-100	4-100	30-100	23.27	16.53	

- Factored resistances have been increased 15% for earthquake or wind loading. No further increase is permitted. Reduce where other load durations govern.
- Nails in studs/post shall be installed symmetrically. Nails may be installed over the entire length of the strap.
- 3. The 3" wide beam may be double 2x members.
- 4. Straps installed over sheathing up to $\frac{1}{2}$ " thick can achieve 85% of the tabulated values.
- NAILS: 10d = 0.148" dia. x 3" long. See pages 22-23 for other nail sizes and information.



PWF24 Strap Tie

The PWF24 is a galvanized metal strap manufactured specifically for connecting preservative-treated wood foundation walls to the floor system. This strap exceeds the prescriptive requirements of CSA S406-14 "Construction of Preserved Wood Foundations."

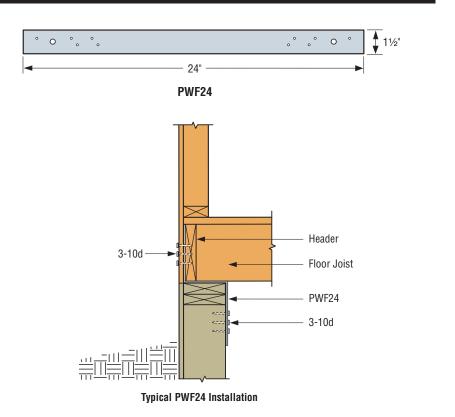
MATERIAL: 20 gauge FINISH: Galvanized

INSTALLATION: • All fasteners shall be hot dipped galvanized.

- See CSA S406-14.
- For installations in interior-dry applications with CCA-treated lumber only.

Model	Dimens	ions (in)	Total
No.	W	L	Fasteners
PWF24	1½	24	6-10d

- 1. Install 3 nails into the stud and 3 nails into the rim board.
- NAILS: 10d = 0.148" dia. x 3" long.
 See pages 22-23 for other nail sizes and information.



A Angles and Z Clips

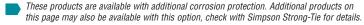
Z2 clips secure 2x4 flat blocking between joists or trusses

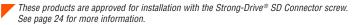
to support sheathing. MATERIAL: Z clips—see table. A21 and A23—18 ga.; all other A angles-12 ga.

FINISH: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pages 14-17.

INSTALLATION: • Use all specified fasteners. See General Notes.

- Z clips do not provide lateral stability. Do not walk on stiffeners or apply load until diaphragm is installed and nailed to stiffeners.
- F₁ direction is loading into the part.





		Din	nensi	ons		Faste	nore		Factore	d Resist	ance (K	= 1.15)
	Madal		(in)			1 4310	,11013		D.F	ir-L	S-P-F	
	Model No.					Base		Post	F ₁	F ₂	F ₁	F ₂
		W ₁	W ₂	L	Bolts	Nails	Bolts	Nails	lbs	lbs	lbs	lbs
					DUILS	Maiis	סווט	Maiis	kN	kN	kN	kN
	A21	2	1½	1%		2-10dx11/2		2-10dx1½	405	260	335	185
	AZI		1 72	178		Z-100X172		Z-100X172	1.80	1.16	1.49	0.82
	A23	2	1½	23/4		4.40dv41/		4-10dx1½	815	715	725	510
	AZS	2	1 72	274	_	4-10dx1½	_	4-10ux 1 72	3.63	3.18	3.23	2.27
	A 2.2	2	3	1½		4 104	I-10d —	4-10d	1175	570	930	405
	A33 3	3	3	1 72		4-100	_	4-100	5.23	2.54	4.14	1.80
	A44	44 497 43	4% 1½		4 104		4 104	1175	485	930	345	
	A44	4%16	478	1 72		4-10d		4-10d	5.23	2.16	4.14	1.53
	A66	5%	5%	1½	2-3/8"	3-10d	2-3/8"	3-10d	_	_	_	_
	AUU	378	378	1 //2	MB	3-10u	MB	3-10u	_	_	_	_
Ì	A88	8	8	2	3-3/8"	4-10d	3-3/8"	4-10d	_	_	_	_
	Aoo	0	0	4	MB	4-10u	MB	4-10u	_	_	_	_
	A24	3%	2	2½	1-1/2"		1-1/2"	2-10d	_	_	_	_
	A24	378		Z 7/2	MB		MB	Z-100	_			
	A311	11	35/8	2	1-1/2"		1-1/2"	4-10d				
	ASII	11	3%8	2	MB	_	MB	4-100	_	_	_	_

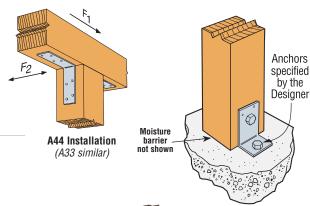
- 1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed; reduce where other loads govern.
- 2. **NAILS:** $10dx1\frac{1}{2} = 0.148$ " dia. x $1\frac{1}{2}$ " long, 10d = 0.148" dia. x 3" long. See pages 22-23 for other nail sizes and information.

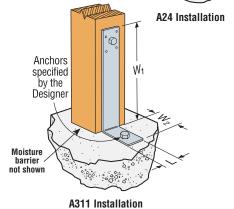
Catalogue C-C-CAN2015 © 2015 SIMPSON STRONG-TIE COMPANY INC.

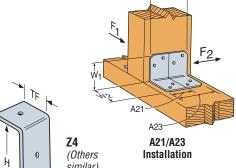
				nsions n)				Resistance 1.00)
Model No.	Ga					Fasteners ¹ Total	D.Fir-L	S-P-F
NO.		W ₁	Н	В	TF	iotai	lbs	lbs
							kN	kN
Z2	20	2 5⁄16	1½	1%	1%	4-10dx1½	740	525
	20	∠ 716	1 72	178	178	4-10ux 1 72	3.29	2.34
Z4	12	1½	3½	21/8	13/4	2-16d	765	545
	12	1 72	372	278	174	2-10u	3.40	2.42
Z6	12	1½	5%	2	1%	2-16d	790	560
20	12	1 /2	J78		1 78	2-10u	3.51	2.49
Z28	28	25/16	1½	1%	1%	10dx1½	-	_
220	20	Z716	1 72	178	178	100X172	-	_
Z38	28	25/16	2½	1%	1%	10dx1½		_
230	20	∠716	2 /2	1 78	1 78	IUUXI/2	_	_
Z44	12	2½	3½	2	1%	4-16d	1420	1010
244	12	4 72	J72		1 78	4-10u	6.32	4.49

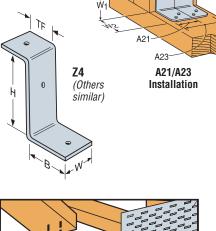
- 1. Z28 and Z38 do not have nail holes. Fastener quantity and type shall be per Designer.
- 2. Z4 and Z6 resistances apply with a nail into the top and a nail into the seat.
- 3. Factored resistances for Z clips cannot be increased for short term loading.
- Typical 72 Installation 4. **NAILS:** 16d = 0.162" dia x 3½" long See pages 22-23 for other nail sizes

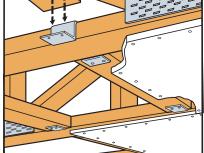












SIMPSON Strong-Tie

The larger LTP5 spans subfloor at the top of the blocking or rim joist. The embossments enhance performance and the min/max nailing option allows for design flexibility.

The LTP4 Lateral Tie Plate transfers shear forces for top plate-to-rim joist or blocking connections. Nail holes are spaced to prevent wood splitting for single and double top plate applications. May be installed over plywood sheathing.

The A35 anchor's exclusive bending slot allows instant, accurate field bends for all two- and three-way ties. Balanced, completely reversible design permits the A35 to secure a great variety of connections.

MATERIAL: LTP4/LTP5—20 gauge; all others—18 gauge FINISH: Galvanized. Some products available in stainless steel or ZMAX®; see Corrosion Information, pages 14-17.

INSTALLATION: • Use all specified fasteners. See General Notes.

Fasteners

Total

8-8dx1½

9-8dx1½

12-8dx11/2

12-8dx1½

12-8dx11/2

12-8dx1½

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

These products are approved for installation with the Strong-Drive® SD Connector screw.

Direction

of Load

 F_2

A₁, E

C₁

 A_2

 C_2

D

 F_1

 F_2

G

Н

G

3. LTP4 can be installed over ½" plywood sheathing with no reduction in capacity.

Factored Resistance

 $(K_D = 1.00) | (K_D = 1.15) | (K_D = 1.00) | (K_D = 1.15)$

lbs

kΝ

475

2 11

455

2.02

430

1.91

205

0.91

375

1.67

355

1 58

225

1.00

675

3.00

650

2.89

580

2.58

595

2.65

620

2.76

615

2.74

lbs

kΝ

665

2.96

640

2.85

545

2.42

290

1.29

545

2.42

505

2.25

365

1.62

955

4.25

920

4.09

815

3.63

835

3.71

875

3.89

865

3.85

D.Fir-L

lbs

kΝ

630

2.80

630

2.80

475

2.11

290

1.29

475

2.11

475

2.11

315

1.40

950

4.23

920

4.09

815

3.63

835

3.71

875

3.89

865

3.85

2. Some illustrations show connections that could cause cross-grain tension or bending of the wood during loading if not reinforced sufficiently. In this case, mechanical reinforcement should be considered.

4. LTP5 can be installed over 1/2" plywood sheathing and achieve 0.89 of the tabulated values for loads

S-P-F

lbs

kΝ

475

2.11

455

2.02

440

1.96

205

0.91

430

1.91

355

1.58

260

1.16

675

3.00

650

2.89

580

2.58

595

2.65

620

2.76

615

2.74

• A35-Bend one time only.

See page 24 for more information.

Type of

Connection

1

2

3

4

5

6

the minimum joist thickness is 3".

Model

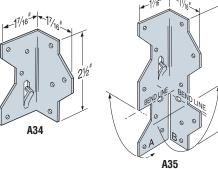
A34

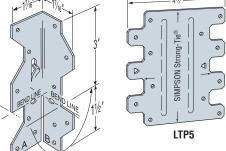
A35

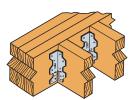
LTP4

LTP5

Straps & Ties

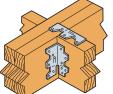






LTP4 41/4

Ceiling Joists to Beam

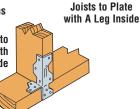


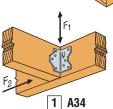


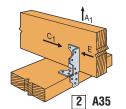
LTP4

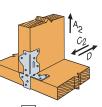
Joists to Beams

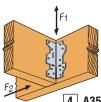
Studs to Plate with B Leg Outside



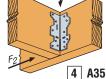








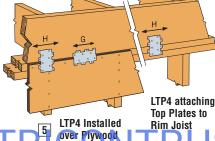
	2 C2 D
3 A35	



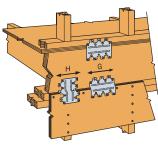


1. Factored resistances are for one anchor. When anchors are installed on each side of the joist,





Sheathing



L/LS/GA Reinforcing and Skewable Angles

L–Staggered nail pattern reduces the possibility for splitting. LS–Field-adjustable 0° to 135° angles.

The GA Gusset Angles' embossed bend section provides added strength.

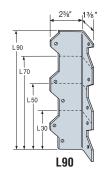
MATERIAL: L-16 gauge; GA and LS-18 gauge

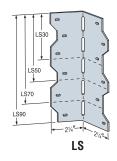
FINISH: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pages 14-17.

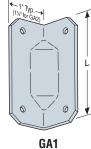
INSTALLATION:

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- Use all specified fasteners; see General Notes.
- LS-field skewable; bend one time only.
- Joist must be constrained against rotation (for example, with solid blocking) when using a single LS per connection.
- Nail the L angle's wider leg into the joist to ensure table values and allow correct nailing.



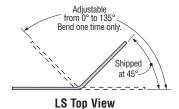


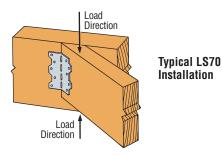


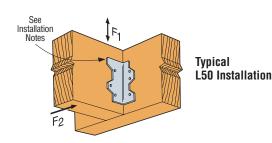
(GA2 similar)

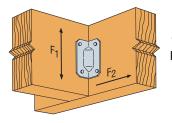
These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

					Resistance	D F
Model	L	Fasteners		ir-L	-	P-F
No.	(in)	Total	$(K_D = 1.00)$	(K _D = 1.15)	$(K_D = 1.00)$	(K _D = 1.15)
			lbs	lbs	lbs	lbs
			kN	kN	kN	kN
GA1	23/4	4-10dx1½	305	350	215	245
G/11	2/4	1 100/172	1.36	1.56	0.96	1.09
GA2	31/4	6-10dx1½	530	610	485	555
G/12	074	0 100X172	2.36	2.71	2.16	2.47
		4-10dx1½	360	360	275	315
L30	3	4 100X172	1.60	1.60	1.22	1.40
LOU	0	4-10d	420	480	390	445
		4-100	1.87	2.14	1.73	1.98
		6-10d	625	720	580	670
		0 100	2.78	3.20	2.58	2.98
L50	5	6-SD#9x1½"	685	785	585	675
LJU	J	U-3D#3X172	3.05	3.49	2.60	3.00
	6-SD#9		830	830	585	675
		0-3D#3X272	3.69	3.69	2.60	3.00
		8-10d	835	960	775	890
	7	0-10u	3.71	4.27	3.45	3.96
1.70	7	0.00//0.41/#	910	1050	835	960
L70		8-SD#9x1½"	4.05	4.67	3.71	4.27
		0.00.00//	1290	1480	1115	1280
		8-SD#9x2½"	5.74	6.58	4.96	5.69
		40.40.1	1045	1200	970	1115
		10-100	4.65	5.34	4.31	4.96
1.00	0	40. CD //041/#	1140	1310	1045	1200
L90	9	10-SD#9X1½	5.07	5.83	4.65	5.34
		40.00.01/1	1610	1850	1450	1670
		10-SD#9XZ½	7.16	8.23	6.45	7.43
		0.404.41/	475	475	365	415
1 000	00/	8-SD#9x2½" 10-10d 10-SD#9x1½" 10-SD#9x2½" 6-10dx1½ 6-10d	2.11	2.11	1.62	1.85
LS30	3%	6 101	540	555	385	415
		6-100	2.40	2.47	1.71	1.85
		0.404.447	720	800	625	720
1050	47/	8-10dx1½	3.20	3.56	2.78	3.20
LS50	41//8	0.404	770	890	670	720
		8-10d	3.43	3.96	2.98	3.20
		40.401.417	900	1035	700	805
1.070	02/	10-10dx1½	4.00	4.60	3.11	3.58
LS70	6%	40.40.1	965	1090	775	805
		10-10d	4.29	4.85	3.45	3.58
		40.40 41/	1080	1240	980	1125
1.000	7-/	12-10dx1½	4.80	5.52	4.36	5.00
LS90	71/8	40	1160	1330	1010	1125
		12-10d	5.16	5.92	4.49	5.00
		W	$\Delta\Delta\Delta$	V _	RIC	









Typical GA Installation

- 1. GA resistances are for both F1 or F2 direction. L and LS resistances are for F1 direction only.
- 2. Factored resistances shown are for one part.
- 3. L50, L70 and L90 may be installed using 10dx1½" nails. Multiply the tablulated 10d resistances x 0.92.
- 4. NAILS: 10d = 0.148" dia. x 3" long, $10dx1\frac{1}{2} = 0.148$ " x $1\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.
- 5. **SCREWS:** SD#9x1½" = 0.131" dia. x 1½ long (SD9112)

SD#9x2½" = 0.131" dia. x 2½" long (SD9212)

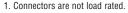
Versatile angle gussets and heavy angles promote standardization and construction economy, and are compatible with Strong-Tie structural hardware.

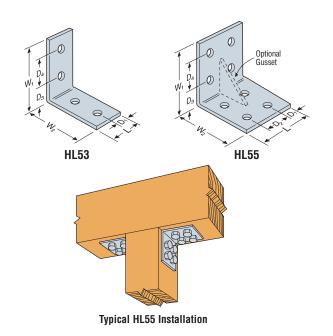
FINISH: HL33, 35, 53, 55—Galvanized; others Simpson Strong-Tie® gray paint (including all parts with gussets). May be ordered in HDG.

OPTIONS: Gussets may be added to HL models when $L \ge 5$ " (specify G after model number, as in HL46G).

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

Model	Ga		Dimensions (in)									
No.		W ₁ & W ₂	L	D ₁	D ₂	D ₃	D ₄	Qty.	Dia.			
HL33	7	31/4	2½	11/4	_	2	_	2	1/2			
HL35	7	31/4	5	11/4	2½	2	_	4	1/2			
HL53	7	5¾	21/2	11/4	_	2	21/2	4	1/2			
HL55	7	5¾	5	11/4	21/2	2	21/2	8	1/2			
HL43	3	41/4	3	1½	_	23/4	_	2	3/4			
HL46	3	41/4	6	1½	3	23/4	_	4	3/4			
HL73	3	71/4	3	11/2	_	23/4	3	4	3/4			
HL76	3	71/4	6	1½	3	23/4	3	8	3/4			





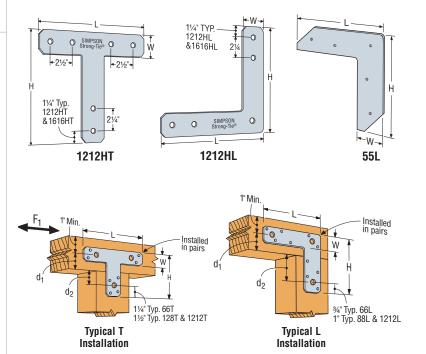
T and L Strap Ties

FINISH: Galvanized. See Corrosion Information, pages 14-17.

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

		Dime	nsions	s (in)	Fas	steners	
Model No.	Ga	L	н	w	Nails	Во	Its
110.		_	-	VV	Naiis	Qty.	Dia.
55L	16	43/4	43/4	11/4	5-10d	_	_
66L	14	6	6	1½	10-16d	3	3/8
88L	14	8	8	2	12-16d	3	1/2
1212L	14	12	12	2	14-16d	3	1/2
1212HL	7	12	12	21/2	_	4	5/8
1616HL	7	16	16	2½		4	5/8
66T	14	6	5	11/2	8-16d	3	3/8
128T	14	12	8	2	12-16d	3	1/2
1212T	14	12	12	2	12-16d	3	1/2
1212HT 7	7	12	12	2½	_	6	5/8
1616HT	7	16	16	2½		6	5/8

- 1. Connectors are not load-rated.
- 2. **NAILS:** 16d = 0.162" dia. x 3½" long, 10d = 0.148" dia. x 3" long. See pages 22-23 for other nail sizes and information.



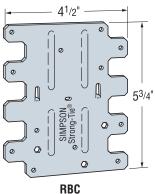
RBC Roof Boundary Clip

SIMPSON
Strong-Tie

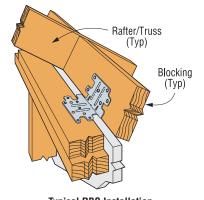
The RBC Roof Boundary Clip is designed to aid installation and transfer shear loads between the roof diaphragm and wall. The locator tabs make proper location of the clip easy. The RBC can be used on wood or masonry walls and will handle roof pitches from 0/12 to 12/12.

MATERIAL: 20 gauge FINISH: Galvanized INSTALLATION:

- Use all specified fasteners. See General Notes.
- Field bend to desired angle one time only.
- See flier F-RBC for more information on installation.
- Titen® screws are not recommended for exposed exterior applications or wet service conditions.
- See page 32 for more information on Titen



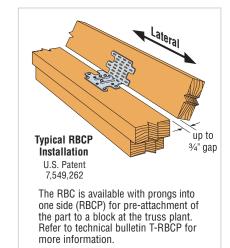
U.S. Patent 7,293,390

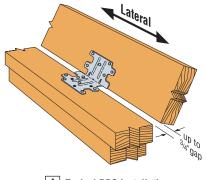


Typical RBC Installation Over 1" Foamboard⁵

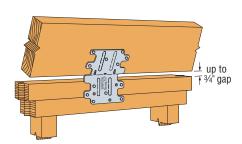
			Faste	eners	Factored Resistance (K _D = 1.15)		
Model No.	Type of Connection	Bending			D.Fir-L	S-P-F	
NU.	Connection	Angle	To Wall	To Blocking	lbs	lbs	
					kN	kN	
	1	45° to 90°	0.40441/	6-10dx1½	660	465	
		45 10 90	6-10dx1½	0-10ux172	2.94	2.07	
		< 30°	6-10dx1½	6-10dx1½	645	460	
RBC	2		0-100X172	0-100X172	2.87	2.05	
NDC		30° to 45°	6-10dx1½	6-10dx1½	685	485	
		30 10 45	0-100X172	0-10ux172	3.05	2.16	
	2	0° to 45°	3-1/4x21/4 Titen	6-10dx1½	575	410	
	3	0 10 43	3-74X274 IIIeII	0-10ux172	2.56	1.82	

- 1. Factored resistances are for one anchor attached to blocking minimum $1\frac{1}{2}$ " thick.
- 2. RBC can be installed with up to 3/4" gap and achieve 100% of the listed value.
- Factored resistances have been increased 15% for wind or earthquake loading with no further increase allowed. Reduce where other loads govern.
- 4. When attaching to concrete use 3-1/4x13/4" Titen® screws.
- RBC installed over 1" foamboard has a factored resistance of 650 lbs. (2.89 kN) in a parallel to wall load direction for D.Fir-L. For S-P-F, the value is 460 lbs (2.05 kN).
- 6. RBC may be installed over $\frac{1}{2}$ " structural sheathing using $10dx1\frac{1}{2}$ nails with no reduction in capacity.
- 7. **NAILS:** $10dx1\frac{1}{2} = 0.148$ " dia. $x \frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.

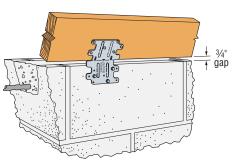




1 Typical RBC Installation



2 Typical RBC Installation



3 Typical RBC Installation to CMU Block

MATERIAL: See table

- Can be used on a single 2x stud.
- Threaded rod, washers and nuts are not supplied with the FSC.

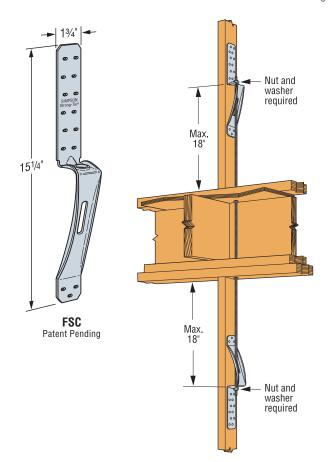
As an alternative to coil strap, our new FSC-Floor Span Connector, connects upper floors to lower floors from the inside of the wall. The

convenient obround holes make installation in narrow wall cavities easy. Installs with a ¾" all thread rod, nut and washer *(not included)*.

- \bullet Use %" threaded rod grade A307 or better, with matching nuts and cut washers.
- FSC may be installed a maximum of 18" from the sill or top plates.
- Drill ½" to ¾" diameter hole through the plates for threaded rod access, hole should be located approximately 1½" away from the face of stud used for FSC attachment.
- Nails can be installed up to 30 degree angle with no reduction in capacity.

		Fasten	ers		ile Resistance 1.15)			
Model No.	Ga			D.Fir-L S-P-F				
NU.		Stud	Anchor	lbs	lbs			
				kN	kN			
FSC	12	15-10dx1½	3%" ATR	2520	1790			
F50	12	13-100X172	% AIK	11.22	7.96			

- The factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed; reduce where other loads govern.
- 2. Resistances are based on a minimum lumber thickness of 1½".
- 3. Standard cut washer is required with the %" all thread rod.
- 4. FSC's shall be offset no more than 3" horizontally from each other.
- 5. NAILS: $10dx1\frac{1}{2} = 0.148$ " dia. x $1\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.



Typical FSC Installation

DSP/SSP/SP/SPH/RSP4/TSP Stud Plate Ties



This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

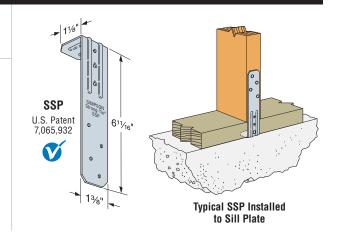
The Stud Plate Tie series offers various solutions for connecting the stud to the top and bottom plates. All models can be used to make a connection to either the top or bottom plate, and several are suitable for double top plates and studs.

MATERIAL: DSP/SSP/SPH—18 gauge; TSP—16 gauge; all others—20 gauge **FINISH**: Galvanized. Some products available in ZMAX® coating;

see Corrosion Information, pages 14-17.

INSTALLATION: • Use all specified fasteners; see General Notes.

- TSP/DSP/SSP—sill plate installation-fill all round holes.
- TSP/DSP/SSP—top plate installation-fill all round and triangle holes
- SP1/SP2—one of the 10d common stud nails is driven at a 45° angle through the stud into the plate.

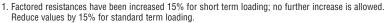


DSP/SSP/SP/SPH/RSP4/TSP Stud Plate Ties

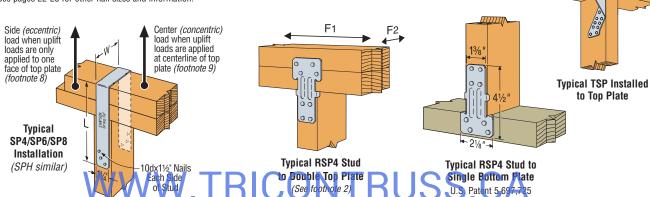
SIMPSON

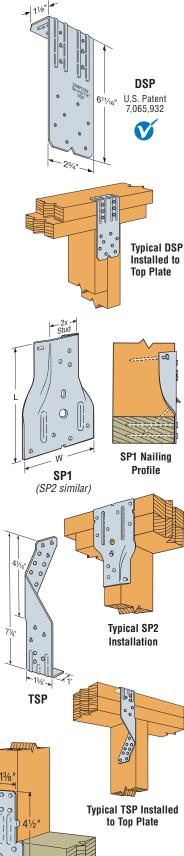
- These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.
- These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

				Dimer		Fasteners			Facto	red Resist	ance (K _D =	:1.15)
				(i	n)		1 431011013		D.F	ir-L	S-I	P-F
	Model No.	Stud	Plate Width	W	L	Studs	Double	Single	Double Top Plate	Single Sill Plate	Double Top Plate	Single Sill Plate
				VV	L .	Stuus	Top Plate	Sill Plate	lbs	lbs	lbs	lbs
									kN	kN	kN	kN
							Connector	Type				
	RSP4	2x		21/8	41/2	4-8dx1½	4-8dx1½	4-8dx1½	670	595	600	535
	nor4	_ ZX		278	4 72	4-0UX172	4-0UX 1 72	4-0UX 1 72	2.98	2.65	2.67	2.38
						4-10dx1½	3-10dx1½	1-10dx1½	570	535	570	535
	SSP	2x		13/8	611/16	4-10ux1/2	3-10ux 1 /2	1-10ux 1 /2	2.54	2.38	2.54	2.38
	001	^^		178	0 /16	4-10d	3-10d	1-10d	710	690	710	690
ļ						+ 10u	0 10u	1 100	3.16	3.07	3.16	3.07
	SP1	2x	_	3½	51/16	6-10d	_	4-10d	_	810	_	740
	01 1			072	3 710	0 100		4 10u	_	3.60	_	3.29
	SP2	2x	_	3½	6%	6-10d	6-10d	_	1220	_	1110	_
	01 2			072	070	0 100	0 100		5.43	_	4.94	_
						8-10dx1½	6-10dx1½	2-10dx1½	1270	890	1270	890
	DSP	2-2x	_	23/4	611/16	0-10UX172	0 100X172	2 10UX172	5.65	3.96	5.65	3.96
	DOI	L L X		L/4	0 710	8-10d	6-10d	2-10d	1550	985	1550	985
						8-10d	0 100	2 100	6.90	4.38	6.90	4.38
						6-10dx1½	_	3-10dx1½		765		685
						0 100X172		0 100X172		3.40	_	3.05
	TSP	_	_	1½	71/8	9-10dx1½	6-10dx1½	_	1325		940	_
	101			172	'''	0 1000/172	0 100X172		5.89		4.18	_
						9-10dx1½	6-10d	_	1455		1030	_
						0 1000/172	0 100		6.47	_	4.58	_
	SP4	2x	4x	3%16	71/4	6-10dx1½	_	_	1135		915	_
			٠,٠	0716	1 /4	0 100X172			5.05	_	4.07	_
	SPH4	2x	4x	3%16	83/4	12-10dx1½	_	_	2450	2010	1815	1430
	01111		-1/	0710	074	12 100X172			10.90	8.94	8.07	6.36
	SP6	2x	6x	5%16	73/4	6-10dx1½	_	_	1135		915	_
	01 0		UA .	0710	1 /4	0 100X172			5.05	_	4.07	_
	SPH6	2x	6x	5%16	91/4	12-10dx1½	_	_	2450	2010	1815	1430
	31 110		- OA	3710	0,4	100/17/2			10.90	8.94	8.07	6.36
	SP8	2x	8x	75/16	85/16	6-10dx1½		_	1135		915	
		_^		10	2710	3 . 5 d/x 1 / 2			5.05		4.07	_
	SPH8	2x	8x	75/16	8%	12-10dx1½	_	_	2450	2010	1815	1430
			- O/A		.				10.90	8.94	8.07	6.36



- RSP4 factored lateral resistance is 345 bs (1.53 kN) D-Fir-L and 245 lbs (1.09 kN) S-P-F for F₁ direction. The factored resistance in the F₂ direction is 175 lbs (0.78 kN) D.Fir-L and 125 lbs (0.51 kN) S-P-F. These values apply to both single and double plate applications.
- 3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
- 4. Tabulated values for SP4, SPH4, SP6, SPH6, SP8 and SPH8 assume loads are applied through the centre of the stud or plates (concentric loading). For applications where the load is applied to the connector through one side of the stud or plates (eccentric loading) multiply the tabulated values by 0.50.
- 5. **NAILS:** 10d = 0.148" dia. x 3" long, $10dx1\frac{1}{2} = 0.148$ " dia. x $1\frac{1}{2}$ " long, $8dx1\frac{1}{2} = 0.131$ " dia. x $1\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.





HU/HUC Hangers

HU and HUC products are heavy duty face mount joist hangers.

MATERIAL: 14 gauge

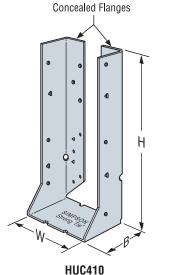
FINISH: Galvanized, ZMAX® and stainless steel available

INSTALLATION: • These hangers are attached to the concrete or grout-filled CMU walls using 1/4" hex head Titen® screws. Titen screws (Model No. TTN25234H) are not provided with the hangers.

- Drill the 3/16" diameter hole to the specified embedment depth plus 1/2".
- \bullet Alternatively, drill the $\% {\rm 16}"$ diameter hole to the specified embedment depth and blow it clean using compressed air.
- · Caution: Oversized holes in the base material will reduce or eliminate the mechanical interlock of the threads with the base material and will reduce the anchor's load capacity.
- · The hangers should be installed such that a minimum end and edge distance of 11/2" is maintained.
- · Stainless steel HU/HUC hangers and Titen screws are available for exterior applications.
- See page 32 for more information on Titen® screw.

OPTIONS: • The HUC is a concealed flange version of the HU. Concealed flange hangers have the face flanges turned in.

- HU is available with A flanges concealed, provided the W dimension is 25/16" or greater, at 100% of the table value.
- · HU is available with one flange concealed when the W dimension is less than 25/16" at 100% of the table value.
- · Skewed HU/HUC hangers attached to masonry have not been evaluated.

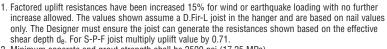




Hex Head

										(5)	
		Dimer	nsions			Fasteners		Factored F	Resistance		1½" min. J
		(i	n)			rastellers		Uplift	Normal		ביוווא
Model No.								$(K_D = 1.15)$	$(K_D = 1.00)$		
	W	Н	В	de	СМИ	Concrete	Joist	lbs	lbs		
								kN	kN		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
HU26	1%6	31/16	21/4	211/16	4-1/4x23/4 Titen	4-1/4x13/4 Titen	2-10dx1½	490	2265		L
11020	1 716	3 / 16	Z /4	Z /16	4-748274 111611	4-74X174 IIIGII	Z-100X172	2.18	10.08		HU4
HU28	1%6	51/4	21/4	47/8	6-1/4x23/4 Titen	6-1/4x13/4 Titen	4-10dx1½	975	3590		Masor
11020	1 716	J74	Z 74	478	0-74XZ74 III.eli	0-74X174 III.	4-10ux 172	4.34	15.97		

Model No.								(K _D = 1.15)	$(K_D = 1.00)$
110.	W	Н	В	de	СМИ	Concrete	Joist	lbs	lbs
								kN	kN
111100	40/	01/	01/	0117	4 1/03/ Titan	4 1/43/ Titon	0.40441/	490	2265
HU26	1%16	31/16	21/4	211/16	4-1/4x23/4 Titen	4-1/4x13/4 Titen	2-10dx1½	2.18	10.08
HU28	1%16	51/4	21/4	47/8	6-1/4x23/4 Titen	6-1/4x13/4 Titen	4-10dx1½	975	3590
пого	I 7/16	374	274	478	0-74XZ94 TILEII	0-74X 1 74 TILEII	4-10ux 1 /2	4.34	15.97
HU210	1%16	71/8	21/4	63/4	8-1/4x23/4 Titen	8-1/4x13/4 Titen	4-10dx1½	975	3590
110210	1716	1 78	Z 74	074	0-74X274 III.	0-74X174 IIIGII	4-10ux 1 72	4.34	15.97
HU212	1%16	9	21/4	85%	10-1/y23/ Titon	10-1/4x13/4 Titen	6-10dv11/a	1465	4015
110212	1 716	9	Z /4	0 78	10-748274 111611	10-74X174 TILGII	0-10ux 1 /2	6.52	17.86
HU26-2	31/8	5%	2½	5	19-1/y93/ Titen	12-1/4x13/4 Titen	6-10d	1575	5430
11020-2	J /8	J/8	2/2	J	12-74X274 TILGII	12-74X174 IIIGII	0-100	7.01	24.15
HU28-2	31/8	7	2½	65/8	14-1/y23/ Titen	14-1/4x13/4 Titen	6-10d	1575	5780
11020 2	078	,	L /2	078	14 74X274 IIIOII	14 /4/(1/4 11(0))	0 100	7.01	25.71
HU210-2	31/8	813/16	2½	87/16	18-1/4x23/4 Titen	18-1/x13/ Titen	10-10d	2620	5780
1102102	078	0 710		0710	10 74X274 THOI	10 74/174 111011	10 100	11.65	25.71
HU212-2	31/8	10%6	2½	103/46	22-1/4x23/4 Titen	22-1/y13/ Titen	10-10d	2620	5780
110212 2	070	10716	2/2	10716	22 /4X2/4 IIIOII	ZZ /4X1/4 11ttil	10 100	11.65	25.71
HU46	3%16	53/16	2½	413/16	12-1/4x23/4 Titen	19-1/y13/ Titen	6-10d	1575	5430
11040	0710	0710	L /2	7 710	12 74X274 THOI	12 /4X1/4 11toll	0 100	7.01	24.15
HU48	3%16	613/16	2½	67/16	14-1/4x23/4 Titen	14-1/x13/ Titen	6-10d	1575	5780
	0710	0 710	L /2	0710	74,72,74 111011	74,7174 116011	0 100	7.01	25.71
HU410	3%16	85/8	2½	81/4	18-1/4x23/4 Titen	18-1/4x13/4 Titen	10-10d	2620	5780
	3710	270		0,4	10 / 1112/4 / 1110//	10 7.50.74 11011	.5 100	11.65	25.71
HU412	3%16	105/16	2½	915/16	22-1/4x23/4 Titen	22-1/4x13/4 Titen	10-10d	2620	5780
110412	0/10	10/18		3 / 10		LL /4/1/4 11t011	10 100	11.65	25.71

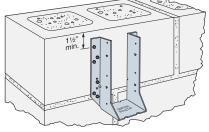


2. Minimum concrete and grout strength shall be 2500 psi (17.25 MPa).

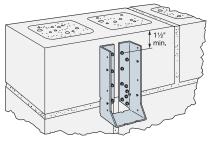
3. The Designer must ensure the joist can generate the factored normal resistances shown.

de is the dimension from the bearing seat to the top joist nail.

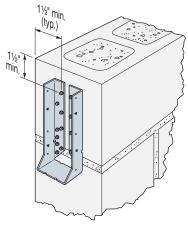
5. **NAILS:** 10d = 0.148" dia. x 3" long, 10dx1 $\sqrt{2}$ CONTRUSS. See pages 22-23 for other nail sizes



410 Installed on nry Block Sidewall



HUC410 Installed on **Masonry Block Sidewall**



HUC410 Installed on Masonly Block End Wall

Masonry Connectors



High-capacity girder hangers for masonry applications. Installation is made easier using Strong-Drive® SDS Heavy-Duty Connector screws into the wood member and Titen HD® anchors into the masonry.

MATERIAL: See table FINISH: Galvanized

INSTALLATION: Use all specified fasteners (included).

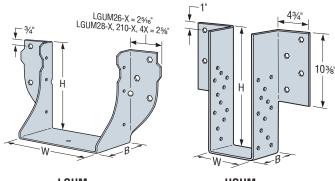
Titen HD:

- Drill holes using drill bits equal in diameter to the specified Titen HD anchor.
- Holes shall be drilled ½" deeper than the specified Titen HD anchor length (i.e. 4½" for a 4" long Titen HD anchor).
- Caution: Oversized holes in the base material will reduce or eliminate the mechanical interlock of the threads with the base material and will reduce the anchor's load capacity.
- Titen HD is not recommended for exposed exterior applications or wet service conditions.
- See page 31 for more information on Titen HD anchors.

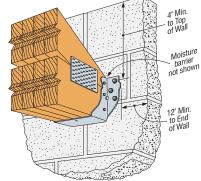
OPTIONS:

- For HGUM only Other seat widths available. Order as "X" version.
- · HGUM available with one flange concealed.
- LGUM/HGUM available with skews up to 45°. See hanger options, page 238.

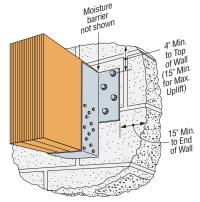
		Dir	nensi	ons	Eac	tonore	Fac	tored Resista	ance
			(in)		Fasteners		Uplift		mal
Model	Ga				CMU/	MU/ Icrete Joist (F	Орин	CMU	Concrete
No.	ua	w	н	В	Concrete		$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.00)$
		"			Titen	SDS	lbs	lbs	lbs
					HD	Screws	kN	kN	kN
					Double 2	x Sizes			
LGUM26-2-SDS	12	35/16	57/16	4	4-3/8"x4"	4-1/4"x2 1/2"	2640	8125	8125
Eddined E obo		0710	0710	'	1 70 X1	1 74 NE 72	11.76	36.19	36.19
LGUM28-2-SDS	12	35/16	73/16	4	6-3/8"x4"	6-1/4"x2 1/2"	4070	10110	10110
		•			•		18.13	45.03	45.03
LGUM210-2-SDS	12	35/16	93/16	4	8-3/8"x4"	8-1/4"x2 1/2"	5430	11585	11585
2002.10 2 050		07.10	0710	·		•	24.19	51.60	51.60
					Triple 2	x Sizes			
LGUM26-3-SDS 12 4 ¹⁵ / ₁₆ 5½ 4 4-3/8"x4" 4-1/4"x2½"	2640	8125	8125						
Eddivizo 5 000	12	7 /10	072	7	70 77	7 /4 / 2 /2	11.76	36.19	36.19
LGUM28-3-SDS	12	415/16	71/4	4	6-3/8"x4"	6-1/4"x21/2"	4070	10110	10110
Eddivizo 5 500	12	7 /10	174	7	0 /8 /4	0 /4 XZ/2	18.13	45.03	45.03
LGUM210-3-SDS	12	415/16	91/4	4	8-3%"y4"	8-1/4"x21/2"	5430	11585	11585
EddWiz 10 0 0B0	12	7 710	374	,	0 78 X4		24.19	51.60	51.60
				C	Quadruple	2x Sizes			
LGUM26-4-SDS	12	6%16	57/16	4	4-3/8"x4"	4-1/4"x21/2"	2640	8125	8125
Eddivi20-4-3D3	12	0716	J/16	7	4-78 A4	4-74 AZ 72	11.76	36.19	36.19
LGUM28-4-SDS	12	6%16	73/16	4	6-3/8"x4"	6-1/4"x21/2"	4070	10110	10110
Eddivizo 4 000	12	0716	1 / 10	7	0 /6 /4	0 /4 XZ/2	18.13	45.03	45.03
LGUM210-4-SDS	12	6%	93/16	4	8-3/8"x4"	8-1/4"x21/2"	5430	11585	11585
EddWiz 10 4 0D0	12	0716	3716	7	0 /8 /4	0 /4 XZ/2	24.19	51.60	51.60
					4x Si	zes			
LGUM46-SDS	12	35/8	55/16	4	4-%"x4"	4-1/4"x21/2"	2640	8125	8125
LGUW40-3D3	12	378	J 716	4	4-78 84	4-/4 XZ/2	11.76	36.19	36.19
LGUM48-SDS	12	35/8	75/16	4	6-3/8"x4"	6-1/4"x21/2"	4070	10110	10110
EddW40-3D3	12	378	1716	7	0-78 74	U-74 XZ72	18.13	45.03	45.03
LGUM410-SDS	12	35/8	95/16	4	8-3/8"x4"	8-1/4"x21/2"	5430	11585	11585
EddW410 0D0	12	078	3716	7	0 /8 /4	U 74 AZ 72	24.19	51.60	51.60
Enț	ginee	red W	ood &	Struc	tural Com	posite Lumb	oer Sizes (He	avy Duty)	
HGUM5.25-SDS	7	51/4		51/4	8-%"x5"	24-1/4"x21/5"	8045	15310	22615
11001013.23-303	<i>'</i>	J74		J74	0-78 XJ	Z4-74 XZ72	35.84	68.20	100.73
HGUM5.50-SDS	7	51/2		51/4	8-5%"x5"	24-1/4"x21/2"	8045	15310	22615
1100W03.30-3D3	1	J /2	,,	51/4 8-	0-78 XJ	८≒ ⁻/4 ⊼८ /2	35.84	68.20	100.73
HGUM7.00-SDS	7	7	11 to	51/4	5½ 8-%"x5" 2	24-1/4"x21/2"	8045	15310	22615
1100W17.00-0D3		<u> </u>	30	J /4		<u>∟</u> ¬ /4 ∧∠ /2	35.84	68.20	100.73
HGUM7.25-SDS	7	71/4		51/4	8-%"x5"	24-1/4"x21/4"	8045	15310	22615
11d0lv17.20-0D3	′	1 /4	_	0 /4	U /8 AU	24-1/4"x21/2"	35.84	68.20	100.73
HGUM9.00-SDS	7	9	۸/	51)	8-% V5"	24-1/4"x21/2"	8045	15310	22615
11001013.00-003	'	9	V	VY		LT-/4 AL/2	35.84	68.20	100.73



LGUM HGUM



Typical LGUM Installation



Typical HGUM Installation

- Factored uplift values have been increased 15% for wind or earthquake loading with no further increase allowed. Reduce where other load durations govern.
- Factored uplift values assume D.Fir-L joist (SG=0.49).For S-P-F joist, multiply the tabulated uplift values by 0.72.
- 3. Factored resistances assume Type S mortar with f'm = 1087 psi (7.5 MPa) for 15 MPa concrete block masonry as per Table 4 CSA S304-14. For values of f'm < 1085 psi (7.5 MPa) multiply the tabulated values by $(f'm / 1085)^{0.5}$.
- 4. Factored resistances assume a 28 day concrete compressive strength of f'_C = 2500 psi (17.25 MPa). For values of f'_C < 2500 psi (17.25 MPa) multiply the tabulated values by $(f'_C/2500)^{0.5}$.
- Factored resistances for concrete block masonry assumes minimum 8" (190 mm) block grouted solid as per CSA A179-14.
 Designer to design block wall reinforcing as per CSA S304-14 to carry the applied load.
- Factored resistances for concrete assumes minimum 8" (203 mm) concrete wall. Designer to design concrete wall reinforcing as per CSA A23.3-14 to carry the applied load.
- Factored normal resistances assume D.Fir-L joist. For other joist materials, the Designer must ensure that the bearing capacity of the joist does not govern.
- HGUM tabulated factored uplift resistance require a minimum loaded edge distance of 15". For loaded edge distances less han 15" to a minimum of 4, the factored uplift resistance is 5030 lbs (22 38 kN).

Catalogue C-C-CAN2015 © 2015 SIMPSON STRONG-TIE COMPANY INC

WM/WMI/WMU Hangers

See pages 126-140 for sizes, fasteners and load information. WMs are designed for use on standard 8" grouted masonry block wall construction.

MATERIAL: See tables on pages 126-140; WM, WMI, WMU—12 ga. top flange and stirrup

FINISH: Simpson Strong-Tie® gray paint; hot-dip galvanized available: specify HDG.

FACTORED RESISTANCES: For hanger heights exceeding the joist height, the factored resistance is 0.50 of the table load.

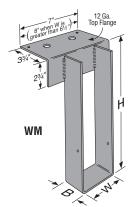
INSTALLATION: • Use all specified fasteners.

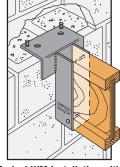
- Mid-Wall—two 16d duplex nails must be installed into the top flange and embedded into the grouted wall. Embed WM into block with a minimum of one course above and one course below the top flange with one 15M vertical rebar minimum 24" long in each cell. Minimum grout strength is 2000 psi (13.8 MPa).
- · When installed on top of masonry wall, use 2-Titen® 1/4x13/4" masonry screws after pre-drilling into minimum 2000 psi (13.8 MPa) grout.
- See page 32 for more information on Titen screws.

OPTIONS: • See Hanger Options, page 231 for hanger modifications and associated load reductions.

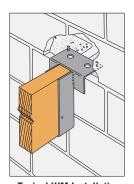
	Joist S	ize (in)		Fastener	s	Factored F	Resistance			
80-4-1						Uplift	Normal			
Model No.	Width	Hoimbt	Ton	Face	Joist	$(K_D = 1.15)$	$(K_D = 1.00)$			
NU.	wiutii	Height	Тор	гасе	Juist	lbs	lbs			
						kN	kN			
Mid-Wall Installation										
WM/WMI	1½ to 7½	3½ to 30	2-16d		2-10dv11/6	_	6060			
VV IVI/ VV IVII	1 /2 10 1 /2	372 10 30	duplex	— 2-10dx1½	_	26.96				
WMU	1½ to 7½	9 to 28	2-16d	4-1/4x13/4	6-10dx11/2	860	6060			
VVIVIO	1 /2 10 1 /2	3 10 20	duplex	Titen	0-10ux172	3.83	26.96			
			Top-of-W	all Installa	ation					
WM/WMI	1½ to 7½	3½ to 30	2-1/4x13/4		2-10dx1½	_	5300			
VVIVI/ VVIVII	172 10 7 72	372 10 30	Titen			_	23.58			
WMU	1½ to 7½	9 to 28	2-1/4x13/4	4-1/4x13/4	6-10dx1½	745	5300			
VVIVIO	1 /2 10 1 /2	3 10 20	Titen	Titen	0-100X172	3.31	23.58			

- 1. Factored uplift resistances shown are for D.Fir-L joist. Multiply table value x 0.71 for S-P-F values.
- 2. WM/WMI/WMU hangers are limited based on joist bearing capacity for the specific wood species, up to the maximum test value of 6060 lbs (26.99 kN). All headers are grouted masonry block.
- 3. Titen® 1/4x13/4" installed on top of wall after grout has cured.
- 4. Products shall be installed such that Titen screws are not exposed to weather.
- 5. **NAILS:** 16d duplex = 0.162" dia. $x3\frac{1}{2}$ " long, $10dx1\frac{1}{2} = 0.148$ " dia. $x1\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.

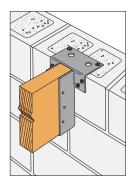




Typical WM Installation with Alternate Nailing Pattern (ANP)



Typical WM Installation at Mid-Wall



Typical WMU Installation at Top-of-Wall

LTA2 Lateral Truss Anchor

The new LTA2 is an embedded truss anchor for grouted CMU and concrete walls that develops high loads with shallow embedment. Designed for 2x4 minimum truss chords, the LTA2 resists uplift and lateral loads parallel and perpendicular to the wall with a minimum heel height requirement.

FEATURES: • Simplified design of the embedded portion allows for easy positioning close to rebar

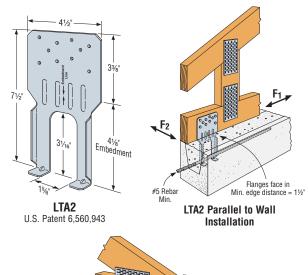
- Ideal for anchoring trusses running perpendicular or parallel to the wall
- Embedment line stamped on part simplifies installation and helps avoid installation errors

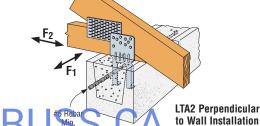
MATERIAL: 18 gauge FINISH: Galvanized; see Corrosion Information, pages 14-17. INSTALLATION: • Use all specified fasteners. See General Notes.

- Whether in grouted CMU or concrete, the LTA2 must be embedded to the depth of the embedment line stamped on the part.
- A minimum of one horizontal 15M rebar is required at top of concrete or in the top course of grouted CMU.
- For parallel-to-wall applications, install the LTA2 with flanges facing the center of the wall. Minimum edge distance of 1½" required.

				Factored Resistance ($K_D = 1.15$)								
	Model No.				D.Fir-L		S-P-F					
		Fasteners	Installation	Uplift	F ₁	F ₂	Uplift	F ₁	F ₂			
				lbs	lbs	lbs	lbs	lbs	lbs			
			l	kN	kN	kN	kN	kN	kN			
			Perpendicular	1845	495	1330	1310	350	945			
	LTA2	10-10dx11/2	to Wall	8.21	2.20	5.92	5.83	1.56	4.20			
		10-10ux 1 72	Parallel	1825	1305	370	1295	930	265			
			to Wall	8.12	5.81	1.65	5.76	4.14	1.18			

- 1. Factored resistances are based on a minimum grout strength of 2500 psi (17.25 MPa) with one 15M horizontal rebar in the shear cone
- 2. Factored uplift resistances have been increased 15% for wind loading with no further increase allowed
- 3. **NAILS:** $10dx1\frac{1}{2} = 0.148$ " dia. x 1 23 for other nall sizes





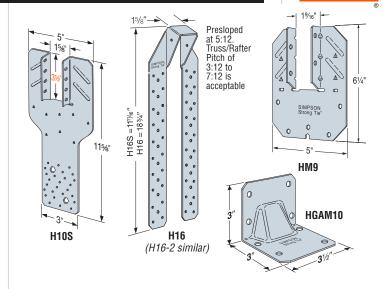
The H10S provides a high capacity connection from truss or rafter to stud. A flexible nailing pattern allows installation where the stud is offset from the rafter up to 1". Suitable for wood-to-wood and wood-to-CMU/concrete application.

The presloped 5:12 seat of the H16 provides for a tight fit and reduced deflection. The strap length provides for various truss height up to a maximum of $13\frac{1}{2}$ ". Minimum heel height for H16 series is 4". **MATERIAL**: See table.

FINISH: Galvanized; other models available in stainless steel or ZMAX®; see Corrosion Information, pages 14-17.

INSTALLATION: • Use all specified fasteners. See General Notes.

- HGAM10 can be installed into grouted concrete block. Screws are provided.
- The HM9KT and the HGAM10KTA are sold with Strong-Drive[®] SDS Heavy-Duty Connector screws and Titen[®] screws.
- · Hurricane ties do not replace solid blocking.
- Attach to grouted concrete block with a minimum one 15M rebar horizontal in the top lintel block.
- Titen® and Titen HD® are not recommended for exposed exterior applications or wet service conditions.
- See pages 31-32 for more information on Titen screw and Titen HD anchor.



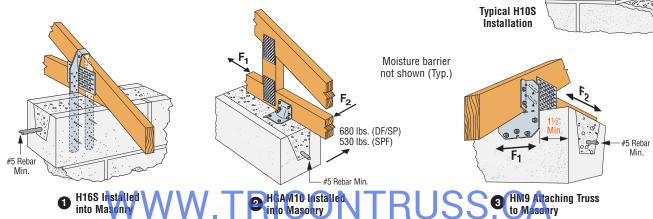
51/8"

		Dimei	nsions		Fasteners			Fact	ored Resist	ance (K _D = 1	1.15)		
		(i	n)		rastellers		D.Fir-L			S-P-F			
Model	Ga						Uplift	Lateral		Uplift	Lat	Lateral	
No.	ua	w	L	Rafters/	СМО	Concrete	Opini	F ₁	F ₂	Opini	F ₁	F ₂	
		VV		Truss	CIVIO	Contracte	lbs	lbs	lbs	lbs	lbs	lbs	
							kN	kN	kN	kN	kN	kN	
HM9KT	18	1%16	61/4	4-1/4"x11/2" SDS	5-1/4x21/4 Titen	5-1/4x13/4 Titen	815	580	285	585	580	285	
THVISKI	10	I 716	074	4-74 X172 3D3	J-74X274 IIIGII	J-74X 174 111611	3.63	2.58	1.27	2.60	2.58	1.27	
HGAM10KT	14	_	_	4-1/4"x11/2" SDS	4-1/4x23/4 Titen	4-1/4x13/4 Titen	1470	1305	1495	1060	940	1310	
HUANTOKT	14			4-74 X172 3D3		4-74X174 IIIGII	6.54	5.81	6.66	4.72	4.18	5.83	
H10S	18	15/8	11%	8-10dx1½	2-36v/ Titan HD	%x4 Titen HD 2-%x4 Titen HD	1655	_		1175	_	_	
11103	10	178	1178	0-10ux 172	2-7884 111611110		7.36	_		5.23	_	_	
H16	18	15/8	18¾	2-10dx1½	6-1/4x21/4 Titen	6-1/4x13/4 Titen	2075	_	_	1470	_	_	
1110	10	178	1074	Z-100X172	0-74XZ74 IIIGII	0-74X174 IIIGII	9.23	_		6.54	_	_	
H16S	18	15/8	11 11/16	2-10dx1½	6-1/4x21/4 Titen	6-1/4x13/4 Titen	2075	_		1470	_	_	
11103	10	1 78	11 /16	Z-100X172	0-748274 111611	0-74X174 IIIGII	9.23	_	_	6.54	_	_	
H16-2	18	31/4	18¾	2-10dx1½	6-1/4x21/4 Titen	6-1/4x13/4 Titen	2075	_	_	1470	_	_	
1110-2	10	374	1074	Z-100X172	6-1/4X21/4 Titen	U-74A 174 IILGII	9.23	_		6.54	_	_	
H16-2S	18	31/4	1111/16	2-10dx1½	6-1/4x21/4 Titen	6-1/4x13/4 Titen	2075	_		1470	_	_	
1110-23	10	374	11.716	Z-10UX 1 72	U-74XZ74 IIIEII	U-74X 174 TILEII	9.23	_	_	6.54	_	_	

- 1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
- Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.
- When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
- HGAM10KTA factored F₂ resistances shown are for loading applied into the connector. For loading applied away from the connector, the factored resistances are 960 lbs (4.27 kN) for D.Fir-L and 690 lbs (3.07 kN) for S-P-F.
- 5. Minimum edge distance for Titen screws is 11/2".

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- Factored resistances for CMU assume Type S mortar with f'm = 1087 psi (7.5 MPa) for 15 MPa concrete block as per Table 4, CSA S304-14.
- 7. CMU must be grouted solid with a minimum grout strength of 2500 psi (17.25 MPa).
- Factored resistances for concrete assume a 28 day concrete compressive strength of f'_C = 2500 psi (17.25 MPa).
- Designer to design wall reinforcing to carry the applied loads.
- 10. NAILS: $10dx1\frac{1}{2} = 0.148$ " dia. $x1\frac{1}{2}$ " long, $8dx1\frac{1}{2} = 0.131$ " dia. $x1\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.



LGT/MGT/VGT Heavy Girder Tiedowns

The LGT and VGT products are moderate to high load capacity girder tie-downs for new or retrofit applications.

LGT connectors provide a low profile connection to the wall for easy installation of drywall. Simple to install and can be installed on the inside or outside of the wall.

The Variable Girder Tiedown (VGT) is a higher capacity alternative to the LGT and MGT for girder trusses. It attaches with SDS screws to the side of truss and features a predeflected crescent washer that allows it to accommodate top chord pitches up to 8:12. The VGT is also available with one flange concealed for attachment to trusses with no overhang.

MATERIAL: VGT—7 gauge; LGT2—14 gauge; MGT, LGT3—12 gauge FINISH: Galvanized

INSTALLATION: • Use all specified fasteners. See General Notes.

- Minimum grout or concrete strength f'c = 2500 psi (17.25 MPa).
- To achieve the values listed in the table below, the product shall be attached
 to a grouted and reinforced block wall or a reinforced concrete wall designed
 by others to transfer the high concentrated uplift forces to the foundation.
- SDS screws included with LGT3, LGT4 and VGT series.
- VGT—Screw holes are configured to allow for double installation on a two-ply (minimum) truss.
- VGT—Can be installed on roof pitches up to 8:12 or on a bottom chord designed to transfer the loads.
- VGT—When installed on trusses with no overhangs, specify VGTR/L.
- VGT—Install washer component (provided) so that top of washer is horizontal as well as parallel with top of wall top plates.
- Titen® screws and Titen HD® anchors are not recommended for exposed exterior applications or wet service conditions.
- See pages 31-32 for more information on Titen screw and Titen HD anchor.

OPTIONS: LGT3 is available with reduced widths of $W = 4^{1}\%_{6}$ " — order as LGT3N-SDS2 5



32-1/4"x3" SDS

2-5/8" Dia.

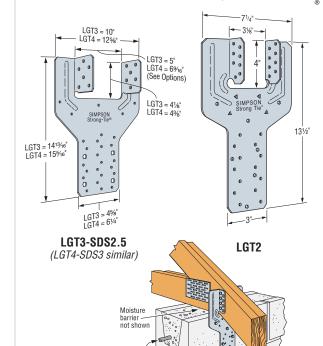
 Factored resistances have been increased 15% for earthquake or wind load. Reduce where other load durations govern.

2 ply min.

- 2. Attached members must be designed to resist the factored loads.
- The MGT can be installed with straps vertical for full capacity provided 26-10d nails are installed to either a solid header or minimum double 2x6 web.
- 4. Products shall be installed such that the Titen® screws and Titen HD® anchors are not exposed to the weather.
- 5. For concrete wall applications use ½x1¾ Titen screws.

2

 NAILS: 10d = 0.148" dia. x 3" long. See pages 22-23 for other nail sizes and information.

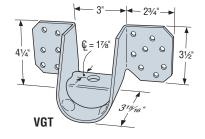


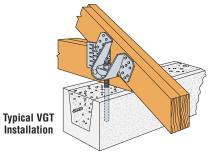
#5 Reba

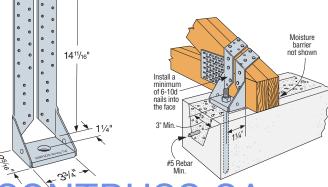
Typical LGT2 Installation into Masonry (LGT3 and LGT4 similar)

SIMPSON

Strong-Tie







VWW.TRICOMGTRUS Typica (MGT Installation

30.92

22.28

HGT Heavy Girder Tiedowns

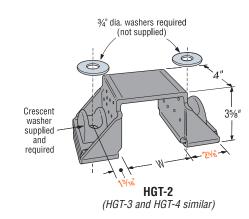
The HGT - Heavy Girder Tiedown offers the highest uplift capacity for girders and can be installed on trusses and beams with top chord slopes from 3:12 to 8:12. The HGT is available in sizes for 2, 3 and 4-ply widths.

MATERIAL: 7 gauge

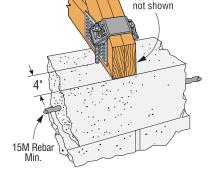
FINISH: Simpson Strong-Tie® gray paint

INSTALLATION: • Use all specified fasteners. See General Notes.

- Minimum grout or concrete strength f'c = 2500 psi (17.25 MPa).
- To achieve the values listed in the table below, the product shall be attached to a grouted and reinforced block wall or a reinforced concrete wall designed by others to transfer the high concentrated uplift forces to the foundation.
- Anchorage from HGT to wall below shall be with 3/4" diameter ASTM A307 Grade A bolts or threaded rod.
- · Standard cut washers (not supplied) are required between the nut and the HGT.
- See page 185 for wood applications.



				Fast	eners	Factored Upli	ift Resistance	
			O.C. Dimension			$(K_D = 1.15)$		
Model No.	Qty.	No. of Plies	Between	Anchor	Girder	D.Fir-L	S-P-F	
			Anchors (in)	Bolts	Truss	lbs	lbs	
			(,			kN	kN	
HGT-2	HGT-2 1 2 plv 5 ¹¹ / ₁₆ 2-¾" φ 16-10d	12140	9280					
1101-2	'	2 ply	J /16	2-¾" ф	10-100	54.00	41.28	
HGT-3	1	3 ply	75/16	0 3/11 1	16-10d	12140	9280	
1101-3	'	3 piy	1 716	2-¾" ф	10-100	54.00	41.28	
HGT-4	1	1 4 ply 9 2-¾" φ 16-10d		12140	9280			
1101-4	'	4 pry	9	2-¾" ф	10-100	54.00	41.28	



Moisture barrier

Typical HGT-2 Installation into Concrete

- 1. Factored resistances have been increased 15% for earthquake or wind load. Reduce where other load durations govern.
- 2. Attached members must be designed to resist the applied loads.
- 3. Anchorage must be designed by others.

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4. NAILS: 10d = 0.148" dia. x 3" long. See pages 22-23 for other nail sizes and information.

MTSM/HTSM Straps Ties

The MTSM and HTSM offer high strength truss to masonry connections.

MATERIAL: MTSM-16 gauge; HTSM-14 gauge

FINISH: Galvanized. Some products available in stainless

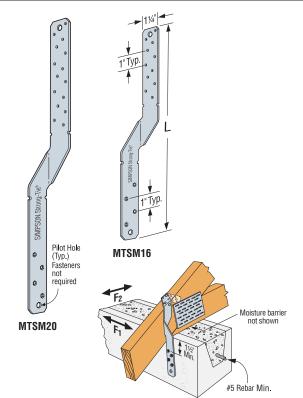
steel or ZMAX®; see Corrosion Information, pages 14-17.

INSTALLATION: • Use all specified fasteners. See General Notes.

- Attach to either side of grouted concrete block with a minimum one 15M rebar horizontal in the lintel block.
- Minimum f'_C = 2500 psi (17.25 MPa) maximum aggregate %".
- Titen® screws are not recommended for exposed exterior applications or wet service conditions.
- See page 32 for more information on Titen screws.

Madal	L (in)		Fasteners	Factored F (K _D =		
Model No.					D.Fir-L	S-P-F
		Truss	CMU	Concrete	lbs	lbs
				kN	kN	
MTSM16	16	7-10d	4-1/4x21/4 Titen	4-1/4x13/4 Titen	1240	880
IVITOIVITO	10	/-10u	4-748274 111611	4-74X194 111611	5.52	3.91
MTSM20	20	7-10d	4-1/4x21/4 Titen	4-1/4x13/4 Titen	1240	880
IVITOIVIZU	20	/-10u	4-748274 111611	4-74X194 111611	5.52	3.91
HTSM16	16	8-10d	4-1/4x21/4 Titen	4-1/4x13/4 Titen	1495	1180
птымпо	10	0-10u	4-74XZ74 IIIUII	4-74X174 IIIEII	6.65	5.25
HTSM20	20	10-10d	4-1/4x21/4 Titen	4-1/4x13/4 Titen	1495	1200
111310120	20	10-10u	4-74A274 IIIGII	4-74A174 IIIGII	6.65	5.34

- 1. Factored resistances have been increased 15% for wind or earthquake loading, no further increase is allowed. Reduce table values where other loads govern as per code.
- 2. Twist straps do not have to be wrapped over the truss to achieve resistances shown.
- 3. Minimum edge distance for Titen® screws is 11/2"
- 4. Products shall be installed such
- the weather. information. 5. NAILS: 10d = 0.148" dia. x 3'



MSTAM/MSTCM Straps Ties

MSTAM and MSTCM models are designed for wood to masonry applications.

The MSTC series has countersunk nail slots for a lower nailing profile. Coined edges ensure safer handling.

FINISH: Galvanized. Some products are

Masonry Application

Ga

18 11/4

16 3

Model

No.

MSTAM24

MSTAM36

MSTCM40

available in stainless steel or ZMAX® coating; see Corrosion Information, pages 14-17.

INSTALLATION:

- · Use all specified fasteners. See General Notes.
- Attaches to grouted concrete block or solid concrete.
- Titen® screws are not recommended for exposed exterior applications or wet service conditions.
- See page 32 for more information on Titen screws.

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

Dimensions

(in)

L

24

36

401/4

Nails

8-10d

12-10d

26-10d

W

11/4

Typical MSTAM36 Installation Clear Span **Factored Tensile Resistance**

lbs

kN

1660

7.38

2685

11.94

5235

23.29

5235

23.29

kΝ

1870

8.32

2685

11.94

5235

23.29

5235

23.29

2" Typ.

11/4

D.Fir-L

lbs

kΝ

1870

8.32

2685

11.94

5235

23.29

5235

23.29

lbs

kN

1790

7.96

2685

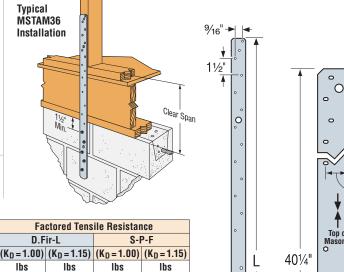
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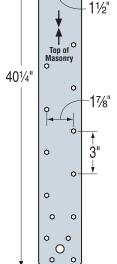
5235

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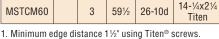
MSTAM36



--3"−

MSTCM40

(Other MSTCM similar)



2. NAILS: 10d = 0.148" dia. x 3" long. See pages 22-23 for other nail sizes and information.

Floor-to-Floor Clear Span Table

		Fasteners			Factored Tensile Resistance				
Madal	Clear		(Total)			ir-L	S-P-F		
Model No.	Span				$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	
NO.	(in)	Nails	CMU	Concrete	lbs	lbs	lbs	lbs	
					kN	kN	kN	kN	
MSTAM36	16 or 18	6-10d	4-1/4x21/4	4-1/4x13/4	1480	1495	1355	1495	
IVISTAIVISU	10 01 10	0-10u	Titen	Titen	6.58	6.65	6.03	6.65	
MSTCM60	22½ 26-10d 14-1		14-1/4x21/4	14-1/4x13/4	5235	5235	5235	5235	
IVISTUIVIOU	2274	20-10u	Titen	Titen	23.29	23.29	23.29	23.29	

Fasteners (Total)

CMU

5-1/4x21/4

Titen

8-1/4x21/4

Titen

14-1/4x21/4

Titen

Concrete

5-1/4x13/4

Titen

8-1/4x13/4

Titen

14-1/4x13/4

Titen

14-1/4x13/4

Titen

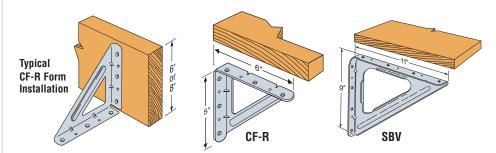
SBV/CF-R Shelf Brackets/Concrete Form Angles

Use the SBV for shelving, counter brackets, window ledge supports, at a very competitive price.

The CF-R is used where a moderate size shelf bracket and reinforcing angle is needed. When used for tilt-up perimeter forming, the nail hole placement ensures substantial re-use.

MATERIAL: 16 gauge FINISH: Galvanized INSTALLATION:

- · Use all specified fasteners. See General Notes.
- SBV-Reversible for nominal 10" or 12" shelves of any thickness.
- CF-R (Retail Pack)—Recommended spacing is 36" for 2x's and 18" for 1x's. Use the 5" leg for 6" lumber and the 6" leg for 8" lumber. Holes are sized for $\frac{1}{4}$ " fasteners or 10d commons



	Fasteners	Factored Resistance ($K_D = 0.65$)				
Model		D.Fir-L	S-P-F			
No.	Stud	lbs	lbs			
		kN	kN			
CF-R	3-SDS 1/4"x2"	140	130			
UI -N	3-3D3 74 XZ	0.62	0.58			
SBV	4-SDS 1/4"x2"	150	140			
SDV	2-303 74 XZ	0.67	0.62			

1. Factored resistances have been decreased for permanent loading. Values can be increased for other load durations as per code.

Masonry Connectors

DTT2Z/DTT2SS Deck Post Connectors

SIMPSON
Strong-Tie

The DTT2 is a safe, cost-effective way to attach deck-railing posts to the deck framing. Because the post is tied back into the deck joists, rather than to the rim joist alone, the connection is stronger than typical through-bolt installations. The DTT2 can be used for laterally tying the deck to the house. Additionally, the versatile DTT2 is load rated as a holdown for light-duty shearwalls and braced wall panel applications. The DTT2 fastens easily to a single 2x joist or stud using Simpson Strong-Tie® Strong-Drive® SDS Heavy-Duty Connector screws (included) and accepts a ½" machine bolt or anchor bolt.

The DTT2SS is made from stainless steel for applications in higher-exposure environments. Whether it's a deck guardrail post application or the lateral-load connection from the deck to the adjacent structure, the stainless-steel DTT2 is the best choice for seaside applications or those calling for more corrosive preservative-treated lumber formulations. It fastens to the framing members with stainless-steel Simpson Strong-Tie Strong-Drive SDS Heavy-Duty Connector screws (included).

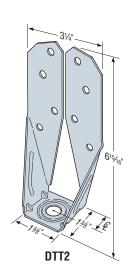
The DTT2Z-SDS2.5 is our standard DTT2Z packaged with $2\frac{1}{2}$ " Simpson Strong-Tie Strong-Drive SDS Heavy-Duty Connector screws instead of the standard $1\frac{1}{2}$ " fasteners. These longer screws allow the DTT2Z to achieve a higher capacity when used as a holdown on double S-P-F studs in a shearwall application. The DTT2Z-SDS2.5 is also suitable in deck applications when double 2x members are used for deck joists or blocking.

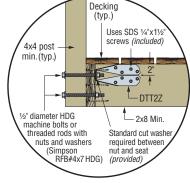
MATERIAL: DTT2Z/DTT2SS—14 gauge

FINISH: DTT2S—ZMAX® coating; DTT2SS—Stainless steel; see Corrosion Information, pages 14-17.

INSTALLATION: Use all specified fasteners. See General Notes.

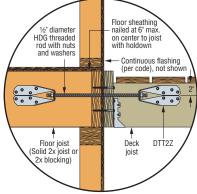
- A standard cut washer (refer to General Notes) must be installed between the nut and the seat.
- Strong-Drive SDS Heavy-Duty Connector screws install best with a low speed high torque drill with a ¾" hex head driver.





DTT2 installed as a lateral connector for a deck guardrail post.

For more information on guardrail post connections, see technical bulletin T-GRDRLPST.



Typical Deck-to-House Lateral Load Connection

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

Model	_	Fasteners		Minimum		Deflection ^{3,4} at Factored	
		Anchor			D.Fir-L	S-P-F	Resistance
	(111)	Bolt Dia.	Fasteners		lbs	lbs	in
		(in)		,	kN	kN	mm
	S 13/16		8-SDS ¼"x1½" - 8-SDS ¼"x2½"	1½	2805	2520	0.250
DTT07/DTT000		1/2			12.48	11.21	6.35
D112Z/D11233					3060	2565	0.250
				ى ا	13.61	11.41	6.35
DTT07 CDC0 E	137.	1/		0	3060	2950	0.250
D112Z-SDS2.5	19/16	72		3	13.61	13.12	6.35
	Model No. DTT2Z/DTT2SS DTT2Z-SDS2.5	No. (in) DTT2Z/DTT2SS 13/46	Model No. (in) Anchor Bolt Dia. (in) DTT2Z/DTT2SS 13/16 1/2	Model No. (in) Anchor Bolt Dia. (in) Fasteners DTT2Z/DTT2SS 13/46 1/2 8-SDS 1/4"x11/2"	Model No. (in) Anchor Bolt Dia. (in) DTT2Z/DTT2SS 13/16 1/2 8-SDS 1/4"x11/2" 3	Model No. Q Anchor Bolt Dia. (in) Fasteners Minimum Wood Thickness (in) D.Fir-L Ibs	Model No. Q (in) Anchor Bolt Dia. (in) Fasteners (in) Wood Thickness (in) D.Fir-L S-P-F Ibs Ibs Ibs No.
- 1. Factored resistances have been increased 15% for short term load duration. Reduce where other load durations govern.
- 2. Tension values are valid for holdowns flush or raised off of the sill plate. 3. Installations shown are for post to joist connections, however these
- Installations shown are for post to joist connections, however these products can be used as a holdown or tension tie for other applications If used as a holdown or tension tie, the following apply:
 - The designer must specify anchor bolt type, length and embedment to ensure adequate anchorage to concrete.

 When using structural compacts lumps across must be a concrete.
 - b. When using structural composite lumber columns, screws must be applied to the wide face of the column.
 - c. Post design shall be by the Designer. Tabulated values are based on
- a minimum wood member thickness in the direction of the fastener penetration. Post may consist of multiple 2x members provided they are designed to act as one unit independently of the holdown fasteners.
- d. Holdowns shall be installed centred along the width of the attached post.
- e. Deflection at Factored Resistance includes fastener slip, holdown elongation and anchor bolt elongation (L = 6"). Additional elongation of anchor bolts shall be accounted for by the Designer when the length of the anchor bolt above the top of the concrete to the attachment at the holdown is longer than 6". Similar consideration for floor to floor connections must be addressed by the Designer.
- Deflection values may be reduced linearly for lesser loads including specified wind loads at h/500.

DPTZ Deck Post Tie

The DPTZ Deck Post Tie products are used to attach 2x4 (DPT5Z) or 4x4 (DPT7Z) vertical posts to the side of stringers, rims or other wood members.

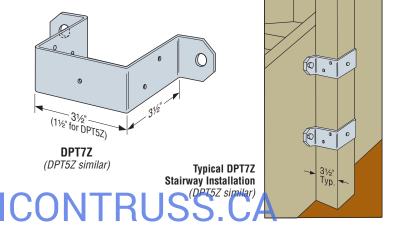
MATERIAL: 14 gauge

FINISH: ZMAX® coating; see Corrosion Information, pages 14-17.

INSTALLATION:

Catalogue C-C-CAN2015 © 2015 SIMPSON STRONG-TIE COMPANY INC

- Use specified HDG fasteners. See General Notes.
- · Install in pairs.
- Install with two %" through bolts into side member and 5-10dx1½ to post for DPT5Z or 5-10d for DPT7Z.



These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

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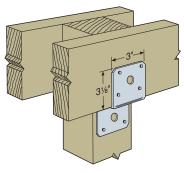
The DJT14Z Deck Joist Tie is designed to attach 2x deck joists to the side of 4x or larger support posts. The DJT14Z can be installed with either nails or bolts.

MATERIAL: 14 gauge

FINISH: ZMAX® coating; see Corrosion Information, pages 14-17. INSTALLATION: • Use specified HDG fasteners. See General Notes.

- · Recommended: install on post first.
- Minimum 2x4 joist and 4x4 post.
- These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.
- These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

	Model No.			Factored Normal Resistance (K _D = 1.00)		
		Ga	Fasteners	D.Fir-L	S-P-F	
				lbs	lbs	
				kN	kN	
			8-16d	1925	1630	
	DJT14Z	14	0-10u	8.56	7.25	
		14	2-5% Dia. MB	2295	1630	
			Z-78 DIA. IVID	10.21	7.25	



Typical DJT14Z Installation

- 1. Resistances assume a dry service condition (K_{SF} = 1.00). Reduce values for other conditions as per 12.2.1.5 CSA 086-14.
- 2. Resistances shown are for one DJT14Z.
- 3. **NAILS:** 16d = 0.162" dia. x $3\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.

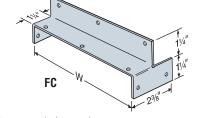
FC Framing Clips

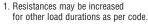
For fast, accurate framing. Three-dimensional nailing pattern results in high-strength joint values. Ideal for fence construction.

MATERIAL: 16 gauge FINISH: Galvanized

INSTALLATION: Use all specified fasteners. See General Notes.

Model No.			F_1 Factored Resistance ($K_D = 1.00$)		
	W (in)	Fasteners	D.Fir-L	S-P-F	
	(111)		lbs	lbs	
			kN	kN	
FC4	3%16	8-16d	1415	1005	
104	3716	0-10u	6.30	4.47	
FC6	51/2	10-16d	1415	1005	
FUO	31/2	10-100	6.30	4.47	

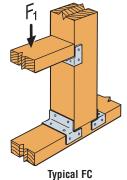




2. Multiply values by 0.67 for wet service conditions ($K_{SF} = 0.67$).

3. A 2½" minimum lumber thickness is required to achieve resistances shown.

NAILS: 16d = 0.162" dia. x $3\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.



Load Installation

ML Angles

The ML angle combines strength and versatility through the use of Simpson Strong-Tie® Strong-Drive® SDS Heavy-Duty Connector screws. Fastener holes are staggered to minimize wood splitting and opposing hole patterns allows for back to back installation without fastener interference.

MATERIAL: 12 gauge

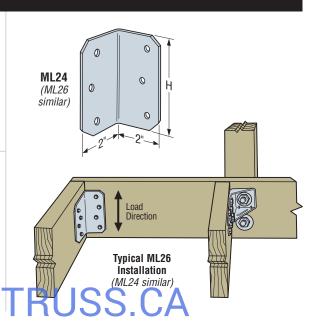
FINISH: ML24Z/ML26Z—ZMAX® coating; ML24SS/ML26SS—stainless steel; see Corrosion Information, pages 14-17.

INSTALLATION: • Use all specified fasteners. See General Notes.

- 1/4"x11/2" Strong-Drive SDS Heavy-Duty Connector screws are not provided with the angle.
- These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

		Factored Resistance			
Model	(in)	Fasteners (Total)	D.Fir-L	S-P-F	
	(,	(10141)	lbs	lbs	
			kN	kN	
ML24	4	6 CDC 1/"v11/"	765	550	
IVILZ4	4	6-SDS 1/4"x11/2"	3.40	2.45	
ML26	6	8-SDS 1/4"x 11/	1360	1160	F
IVILZO	O		6.05	5.16	

1. Factored resistances may be increased 15% for short term load duration. Reduce where other load durations govern



Decks & Fences

The LSC adjustable stair-stringer connector offers a versatile, concealed connection between the stair stringer and the carrying header or rim joist while replacing costly framing. Field slopeable to all common stair stringer pitches, the LSC connector is suitable for either solid or notched stringers.

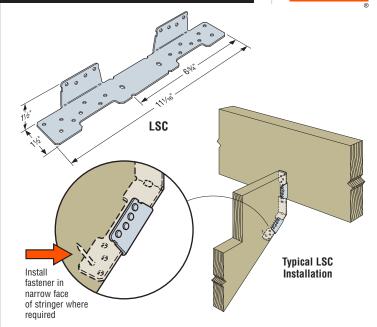
Features:

- · Replaces additional framing and toe-nailing
- Suitable for most installations on 2x10 or 2x12 header/rim joist
- · May be installed flush with the top of the carrying member or lower on the face
- · Interchangeable for left or right applications
- LSCZ features a ZMAX® coating for additional corrosion protection. Suitable for interior and some exterior applications. LSCSS is made from stainless steel for higher exposure

environment. See www.strongtie.com/info for more information MATERIAL: 18 gauge

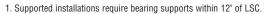
FINISH: LSCZ—ZMAX® coating; LSCSS—Stainless steel **INSTALLATION:**

- · Use all specified fasteners, see table.
- . Before fastening, position the stair stringer with the LSC on the carrying member to verify where the bend should be located.
- Tabs on the LSC must be positioned to the inside of the stairs.
- . The fastener that is installed into the bottom edge of the stringer must go into the second-to-last hole.



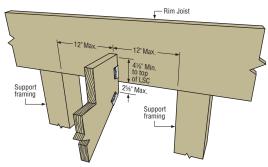
These products feature additional corrosion protection.

			Fasteners		Factored	l Normal		
Model	Rim Joist		Strii	nger	Resistance ($K_D = 1.00$)			
No.	Installation	Rim	Wide	Marrani	D.Fir-L	S-P-F		
		Joist	Wide Face	Narrow Edge	lbs	lbs		
			1 400	Lugo	kN	kN		
		8-10dx1½	8-10dx11//	1-10dx1½	1425	1040		
	Cupported17	0-10UX 1 72	0-10UX172	1-10ux 1 72	6.34	4.63		
	Supported ^{1,7}	8-SD#9x1½"	0 CD#0v11/"		1215	860		
		0-3D#9X172	0-3D#9X172	_	5.40	3.83		
		8-10dx1½	8-10dx11//	1-10dx1½	1165	825		
LSCZ	Standard ²	0-10UX 1 72	0-10UX 172	1-10ux 172	5.18	3.67		
LOUZ	Stanuaru-	9_CD#0v11//"	8-SD#9x1½"	1_CD#0v11/4"	1165	825		
		0-3D# 3X 1 72	0-3D# 3X 1 72	1-3D#3X172	5.18	3.67		
		8-10dx11//	8-10dx1½	1-10dx1½	655	465		
	Cantilevered ^{5,6}	0-100X172	0-10UX172	1-10ux172	2.91	2.07		
	Cantillevereu	8-SD#9x1½"	8-SD#0v11%"		840	600		
		U-3D#3X1/2	U-3D# 3X 1 /2		3.74	2.67		

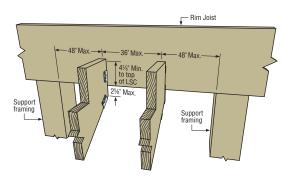


- 2. Standard installations require bearing support within 4 ft. of LSC.
- 3. When cross grain tension forces cannot be avoided in the member, mechanical reinforcement to resist such forces may be considered.
- 4. A minimum distance of 3/4" measured from the lowest rim joist fastener to edge of rim joist is required.

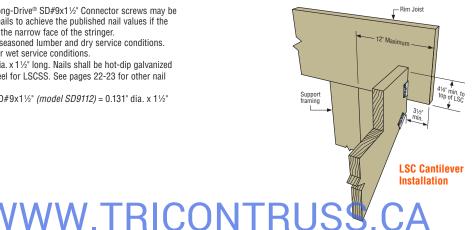
 5. A minimum distance of 3½" measured from the LSC tabs to the
- end of the rim joist is required.
- 6. A maximum rim joist cantilever length of 12" measured from the face of the bearing support to the end of the rim joist is required to achieve the tablulated values
- 7. Simpson Strong-Tie® Strong-Drive® SD#9x1½" Connector screws may be substituted for 10dx11/2" nails to achieve the published nail values if the extra screw is installed in the narrow face of the stringer.
- 8. Tabulated values assume seasoned lumber and dry service conditions. Multiply values by 0.67 for wet service conditions.
- 9. NAIL: $10dx1\frac{1}{2} = 0.148$ " dia. $x 1\frac{1}{2}$ " long. Nails shall be hot-dip galvanized for LSCZ and stainless steel for LSCSS. See pages 22-23 for other nail sizes and information.
- 10. **SCREWS (LSCZ only):** SD#9x1½" (model SD9112) = 0.131" dia. x 1½" long (see page 24).



Supported LSC Installation



Standard LSC Installation



Decks & Fences

TA Staircase Angles

SIMPSON
Strong-Tie

For use in structurally-sound staircase framing. The TA eliminates costly conventional notching.

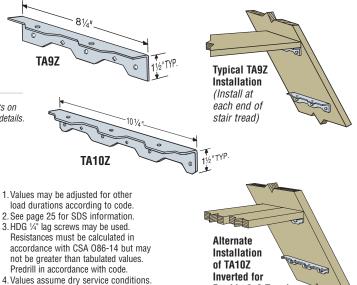
MATERIAL: 12 gauge

FINISH: TA9Z/TA10Z—ZMAX® coating; TA9SS/TA10SS—stainless steel; see Corrosion Information, pages 14-17.

ORDER: May be ordered TA9ZKT and TA10ZKT with two ZMAX TAs and ¼"x1½" Strong-Drive® SDS Heavy-Duty Connector screws.

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

	Faste	ners	Factored Resistance $(K_D = 1.00)$			
Model			(KD = D.Fir-L	1.UU) S-P-F		
No.	Stringer	Tread	lbs	lbs		
			kN	kN		
TA9Z	3-SDS ¼"x1½"	2-SDS 1/4"x11/2"	1025	945		
IA9Z	3-3D3 74 X I 72	2-3D3 74 X I 72	4.56	4.23		
TA10Z	3-SDS 1/4"x11/2"	4-SDS 1/4"x11/2"	1025	1260		
IATUL	3-3D3 /4 X1/2	4-3D3 /4 X1 /2	4.56	5.60		
TA10Z	4-SDS 1/4"x11//"	3-SDS ¼"x1½"	1370	1260		
IAIUZ	4-0D0 74 X 1 72	J-JDJ 74 X I 72	6.10	5.60		



Double 2x6 Treads

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E-Z Base[™]/E-Z Mender[™]/E-Z Spike[™] Fence Products

Replacing an entire fence can be an expensive and difficult task. Simpson Strong-Tie® offers a line of products designed to help make reinforcing fence posts easy and economical. The E-Z Base, E-Z Mender and E-Z Spike offer simple solutions for all types of fence post projects.

E-Z Spike (Model No. FPBS44)

- Allows easy installation of 4x4 wood posts without digging holes or pouring concrete.
- Can be used for a variety of applications where quick-to-install posts are needed.

E-Z Mender (Model No. FPBM44E)

- Allows easy repair of rotted or damaged 4x4 wood posts installed in concrete or dirt.
- Reinforces weakened wood posts without having to replace the post or the concrete.
- Sold individually. Use in pairs.

E-Z Base (Model No. FPBB44)

 Allows easy installation of 4x4 wood posts on existing concrete.

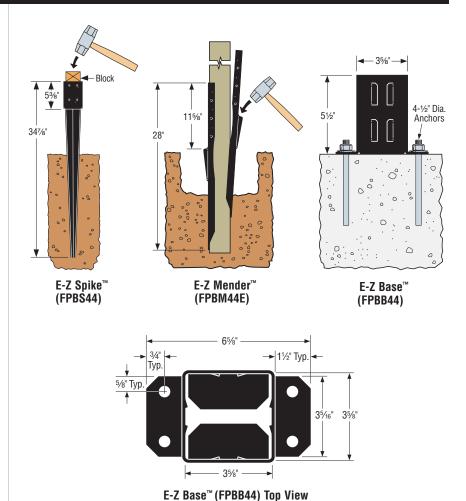
MATERIAL: 12 gauge FINISH: Black powder-coat

INSTALLATION:

- See flier F-EZFPP13.
- Attach post to E-Z Spike or E-Z Base with 8-¼" Strong-Drive® SDS Heavy-Duty Connector screws or ¼" HDG lag screws and attach post to E-Z Mender using 6 HDG nails or screws per part.

NOTE: • Notwithstanding the terms of the Limited Warranty, Simpson Strong-Tie does not guarantee, represent or warrant that this product will perform under, or prevent or reduce damage caused by corrosion, any seismic, wind, atmospheric, or other load-producing event.

These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.



Multiply values by 0.67 for wet service conditions.

Pipe Grip Ties attach wood fence rails to metal fence posts, eliminating rotted and failed wood posts. The PGT is suitable for standard applications as well as corners and splices.

The PGTIC2Z-R is an interior corner pipe grip tie.

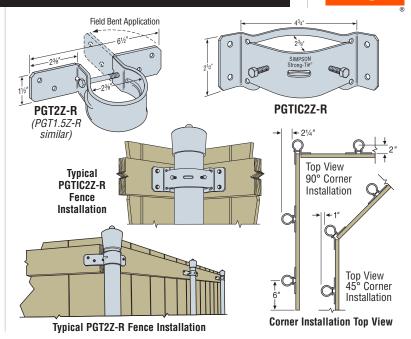
The PGT1.5Z-R is for 1½" pipe (1½" outside diameter), and the PGT2Z-R for 2" pipe (23/8" outside diameter).

MATERIAL: 12 gauge

FINISH: ZMAX® coating, also available in G90.

INSTALLATION: • Use all specified fasteners. See General Notes.

- PGTIC2Z-R to Post Install two set screws (supplied) with 3/8 socket in predrilled holes.
- PGTIC2Z-R to Rails Use Simpson Strong-Tie® 1/4" x 1 1/2" Strong-Drive® SDS Heavy-Duty Connector screws (not supplied).
- · Install on vertical pipes, offsetting corners to allow for the correct rail alignment.
- Use 3 to 4 PGTs per pipe; line up to stringline.
- Fasten PGT with 1/4" hex head bolt (supplied).
- PGT attaches to rails with four 1/4" x 11/2" Strong-Drive SDS Heavy-Duty Connector screws (not supplied). See page 25 for Strong-Drive SDS screw information.
- 1/4" lag bolts may be used. Follow the code requirements for predrilling.
- · Nail fence boards to rails.
- · Field bend PGT flanges to fit corner and angled conditions (bend one time only).



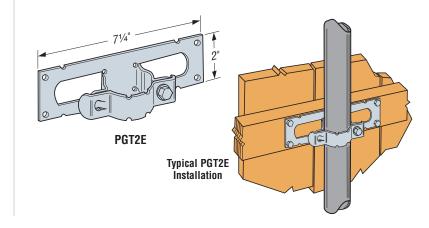
PGT2E Pipe-Grip Tie

Simpson Strong-Tie introduces the latest time-saving solution for building fences with 2" steel posts. The PGT2E pipe-grip tie features a unique two-piece design that installs quickly and provides a solid connection between fence stringer and post. Snap the attachment plate onto the post for easy positioning and secure the strap using one thread-tapping screw (included).

- Faster to install than other two-piece fence-post brackets
- · Safer to use, eliminating protruding carriage bolts and sharp corners
- · Unique locking tab for the strap means only one screw is needed to fasten

MATERIAL: 12 gauge FINISH: Galvanized INSTALLATION:

• Fasten stringers using 1/4" Simpson Strong-Tie® Strong-Drive® SDS Heavy-Duty Connector screws or 1/4" lag screws (follow code requirements for pre-drilling).



FB/FBR Fence Brackets

FB and FBR fence brackets make the connection between fence rails and posts simple and strong. Eliminates the need for toe nailing or screwing. Clean, versatile connections make planning and building fences, deck/porch railings and louvers easier and faster.

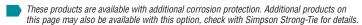
MATERIAL: See table

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FINISH: Galvanized. Some products available in ZMAX® coating; see Corrosion Information, pages 14-17.

INSTALLATION: • Holes are sized for 8dx1½", 8d commons or Strong-Drive® SD9x1½" Connector screws into the supporting member.
• FB24R is sized for 10dx1½" or Strong-Drive SD10x1½" Connector screws.

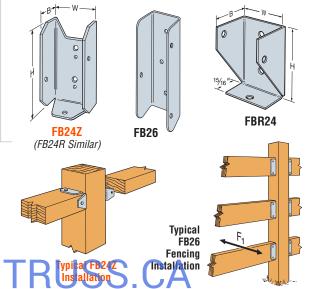
- FB26 is sized for Strong-Drive SD10x11/2" Connector screws.



These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

Model	Ga	Member	Dim	ensions	s (in)	
No.	ua	Size	W	Н	В	١,
FB24Z	20	2x4	1%16	3%	3/4	3
FB24R	20	2x4 RGH	2	3%	3/4	
FBR24	18	2x4	1%	27/16	1 V ₂	
FB26	18	2x6	19/16	5	V1½	

- 1. FB26 has a factored resistance for F₁ of 460 lbs (2.05kN).
- 2. FBR24: R = rail (not rough). 3. **NAILS:** 10dx1½ = 0.148" dia. x 1½' long, 8d = 0.131" dia. x $2\frac{1}{2}$ " long,



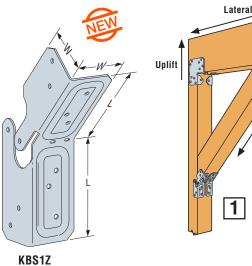
KBS1Z Knee-Brace Stabilizer

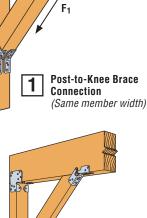
SIMPSON
Strong-Tie

The KBS1Z knee-brace stabilizer makes a structural connection between knee bracing and columns or beams to help stabilize free-standing structures. Factory-formed at a 45° angle and easily installed with nails, the KBS1Z braces 2x, 4x and 6x in line post-to-beam configurations. Check with your local building department for deck bracing requirements.

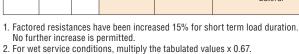
MATERIAL: 16 gauge FINISH: ZMAX® coating INSTALLATION:

- Use all specified fasteners.
- For installations at an angle other than 45°, bend KBS1Z along slots to desired angle. Bend one time only.
- Knee Brace:
- Cut braces at desired angle
- Bend KBS1Z to desired angle if required
- Install fasteners to secure in place
- For equal-width members, install (2) KBS1Z on each end of brace (see connection type 1)
- For 2x knee brace, install single KBS1Z on each end of brace (see connection type 2)
- Beam-to-Post: Install in pairs. See illustrations for quantity and configuration



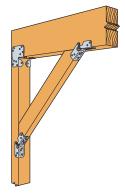


	Dimen (in					F4		Resistance 1.15)
Model			Type of Connection	Connectors per Joint	Direction of Load	Fasteners per	D.Fir-L	S-P-F
	W	L	Connection	per Junit	oi Loau	Connector	lbs	lbs
							kN	kN
					F, - Brace angle = 45°	12-8d	1765	1610
			1	2	1 1 - Drace angle = 45	12-0u	7.85	7.16
				۷	F_1 - Brace angle = 30° or 60°	12-8d	1285	1110
					1 ₁ - Drace arryle = 50 or 60	12-0u	5.72	4.94
					F, - Brace angle = 45°	12-8dx1½	745	685
			2	1	1 1 - Drace arryle = 45	12-00X 172	3.31	3.05
				ļ	F_1 - Brace angle = 30° or 60°	12-8dx1½	735	680
					1 1 - Drace arryle = 30 or 00	12-0ux 1/2	3.27	3.02
		1/2 3		4	Uplift	12-8d	1750	1590
KBS1Z	1½		3		4 ————————————————————————————————————		7.78	7.07
ND31Z	172	3		7	Lateral	12-8d	2415	2205
					Lateral	12-0u	10.74	9.81
					Uplift	12-8d	845	730
				2	Орин	12-0u	3.76	3.25
				۷	Lateral	12-8d	1255	1080
			4		Lateral	12-0u	5.58	4.80
					Uplift	12-8d	1550	1170
				1	οριπι	12*0U	6.90	5.20
				4	Lateral	12-8d	2290	1625
					Lateral	12-0u	10.19	7.23

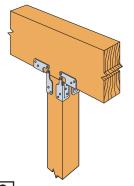


 For all braces installed at intermediate angles between 45° and 30° or 45° and 60°, interpolation between tabulated values may be used.

4. NAILS: 8d = 0.131" dia. x 2½" long, 8dx1½" = 0.131" dia. x 1½" long. See pages 22-23 for other nail sizes and information.

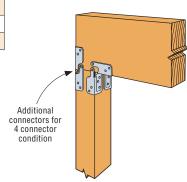


Post-to-2x Knee Brace Connection



(Continuous) Beam-to-Post

Catalogue C-C-CAN2015 © 2015 SIMPSON STRONG-TIE COMPANY INC





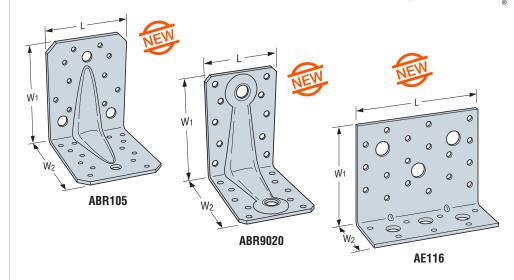
ABR/AE Cross Laminated Timber Connectors

SIMPSON
Strong-Tie

The AE and ABR heavy angles are used to transfer shear forces between CLT wall and floor panels. Both series of angles have been tested using SPF Cross Laminated Timber manufactured to ANSI/APA PRG 320 standard and can be installed using proprietary CNA ring-shank nails or Strong-Drive SD Structural Connector screws.

MATERIAL: See table FINISH: Galvanized INSTALLATION:

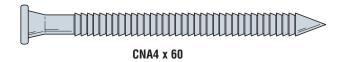
- Use all specified fasteners.
- Installation and fasteners schedule assumes platform framing. Install vertical leg at bottom edge of CLT wall panel and horizontal leg on CLT floor panel with 3%" minimum edge distance.

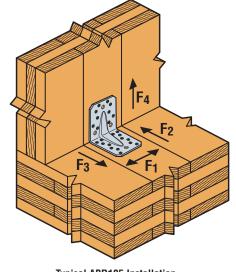


		Dimensio	ons (mm)		Faste	eners	Fa	actored Resist	ance (K _D = 1.1	5)
Model							F ₁	F ₂	F3	F4
No.	t	W ₁	W ₂	L	Horizontal Lea	Horizontal Vertical Leg Leg	lbs	lbs	lbs	lbs
						9	kN	kN	kN	kN
					10-CNA4x60	10-CNA4x60	1525	510	1925	510
ABR9020	2	88	88	65	10-GNA4X00	10-61144800	6.78	2.27	8.56	2.27
ADDSUZU	۷	00	00	05	10-SD#10x2½	10-SD#10x2½	2320	1440	1925	1440
					10-5D#10X272	10-5D#10X2/2	10.31	6.41	8.56	6.41
					14-CNA4x60	10 0004460	1885	500	3330	700
ABR105	3	105	105	90	14-GNA4X00	A4x60 10-CNA4x60	8.38	2.23	14.81	3.12
ABRIUS	3	105	105	90	14 CD#10v01/	10 CD#10v01/	2735	1440	3330	2015
					14-SD#10x2½	10-SD#10x2½	12.16	6.41	14.81	8.97
					7 000 4 4 . 00	10.000.400	2125	900	2125	350
AE440	0	00	40	110	7-CNA4x60	18-CNA4x60	9.45	4.01	9.45	1.56
AE116	3	90	48	116	7 CD#10v01/	10 CD#10v01/	2980	2140	2980	1010
					7-SD#10x2½	18-SD#10x2½	13.25	9.53	13.25	4.49

- Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed. Reduce where other load durations govern.
- 2. Factored resistances are based on Cross Laminated Timber manufactured to ANSI/APA PRG 320 using SPF material.
- 3. **NAILS:** CNA4x60 = 4.1 mm diameter x 60 mm long proprietary ring-shank nail.
- 4. **SCREWS:** SD#10x2½ (model SD9212) = 0.131" dia. x 2½" long.

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Miscellaneous

ICFVL Ledger Connector System



The ICFVL Ledger Connector System is engineered to solve the challenges of mounting wood or steel ledgers to insulated concrete form (ICF) walls. The ICFVL is designed to provide both vertical and lateral, in-plane performance. There are many benefits over traditional anchor bolting, including better on center spacing in most cases, faster installation and no protrusions.

The embedded legs of the ICFVL are embossed for additional stiffness and the hole allows for concrete to flow through and around the connector. The exposed flange on the face of the ICF provides a structural surface for mounting either a wood or steel ledger.

MATERIAL: ICFVL-14 gauge; ICFVL-CW and ICFVL-W-16 gauge

FINISH: Galvanized

INSTALLATION: ICFVL in ICF

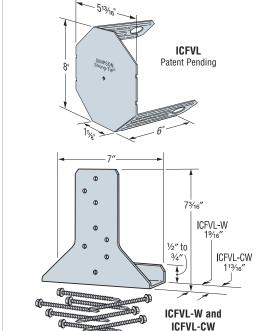
- Snap a chalk line for the bottom of the ledger.
- Mark required on center spacing.
- Use ICFVL to mark kerfs locations.
- · Cut kerfs as marked.
- Insert ICFVL flush to the face of the ICF.
- Pour concrete.

Wood Ledger Attachment - ICFVL-W or -CW

- Slip appropriate ledger connector underneath the ledger.
- Install the eight ICF-D3.62 (included) screws partially into the ledger.
- Position bottom of the ledger level to the chalk line and drive the screws through the wood and into the ICFVL.

Steel Ledger Attachment

- Position bottom of the ledger level to the chalk line and against the ICFVL.
- Attach with four 1/4-14x3/4", #3 drill point screws (not provided).
- ullet All screws should be located at least 1/2" from the edge of the ICFVL.
- · Space screws evenly.



		Factored F	Resistance			
Ledger	Model	Vertical	Lateral			
Type	No.	lbs	lbs			
		kN	kN			
2x D.Fir-L/S-P-F	ICFVL w/ ICFVL-W	w 2820 307				
2X D.FII-L/5-P-F	IGFVL W/ IGFVL-W	kN kN 2820 3075 12.56 13.70 2820 3075 12.56 13.70				
1¾" SCL	ICFVL w/ ICFVL-CW	2820	3075			
1% 36L	IGFVL W/ IGFVL-GVV	12.56	13.70			
Steel	ICFVL	2590	2470			
Steel	IOTVL	11.54	11.00			

- 1. Minimum steel ledger specification is $F_V = 33$ ksi (230 Mpa) and $F_{IJ} = 45$ ksi (310 Mpa) in accordance with CSA S136-12.
- No load duration increase allowed.

WARNING: Industry studies show

that hardened

fasteners can experience

performance problems

in wet environments.

Accordingly, use this

product in dry

environments only.

- 3. Minimum concrete compressive strength, f'c 2500 psi (17.25 Mpa).
- Connector spacing to be determined by the design professional up to a maximum of 4'-0".
- Values shown apply to ICF foam thickness of 3½" or less. Contact factory for values with thicker foam
- When combining vertical and lateral loads designer shall evaluate as follows: Vertical Load/Vertical Resistance + Lateral Load/Lateral Resistance ≤ 1.0.
- The ICFVL must be installed no closer than 4" below the top of the wall to achieve the tabulated resistances shown. For installations where the ICFVL is installed less than 4" from the top of the wall (including flush applications) multiply the factored resistances by 0.94.

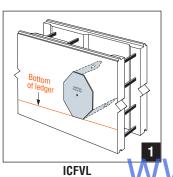
This tables address vertical load applications for ICF foam thickness of $3\frac{1}{4}$ " or less only.

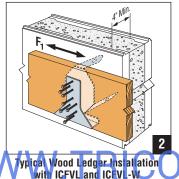
		ICFVL SPACING TO REPLACE ANCHOR BOLTS (in)1,2,3															
Ledger	Connector	½" Dia. Anchors at				5/	%" Dia. Anchors at			(2)-%" Dia. Anchors at			¾" Dia. Anchors at				
Туре	Туре	12" 0.c.	24" 0.c.	36" o.c.	48" 0.c.	12" o.c.	24" 0.c.	36" o.c.	48" 0.c.	12" 0.c.	24" 0.c.	36" o.c.	48" 0.c.	12" 0.c.	24" 0.c.	36" o.c.	48" 0.c.
	WOOD LEDGERS																
2x D.Fir-L/S-P-F	ICFVL w/ ICFVL-W	48	48	48	48	38	48	48	48	19	38	48	48	34	48	48	48
1¾" SCL	ICFVL w/ ICFVL-CW	48	48	48	48	34	48	48	48	17	34	48	48	28	48	48	48
	STEEL LEDGERS																
16 ga (0.060")	ICFVL	20	40	48	48	16	32	48	48	-	_	_	_	_	_	_	_
14 ga (0.057")	ICFVL	16	32	48	48	13	26	39	48	_	_	_	_	_	_	_	_

- 1. The Designer may specify different spacing based on the load requirements. It is recommended to space the components at multiples of the joist spacing to help reduce the chance of interference with joist hangers.
- . Spacings are based on perpendicular to grain capacity of bolt in wood ledger compared to tested value of ICFVL.
- 3. See F-ICFVL flier for additional connection details
- 4. For steel ledgers, the 14 ga spacing is closer than the 16 ga ledger due to the calculated resistance of a bolt being higher when using a thicker piece of steel.

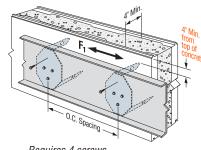
 5. Steel ledger values are based on steel. F_U = 45ksi (310 Mpa).
- The ICFVL must be installed no closer than 4" below the top of the wall
- to achieve the connector spacings shown. For installations where the ICFVL is installed less than 4" from the top of the wall (including flush applications) multiply the connector spacings by 0.94.

Typical Steel Ledger Installation with ICFVL (minimum 16 ga steel ledger)









Requires 4 screws at each location. Table provides on center spacing.

Miscellaneous

The following spacing tables are an alternative to the ICFVL spacing to replace the building code prescribed anchor bolts spacing for vertical loads only. They provide the recommended spacing of the ICFVL Ledger Connectors based on the Factored Vertical Resistance of the connector, the load on the floor, and the span of the joist. The Designer must determine the design load, the ledger design, and the joist design. This table is useful if the Designer already has loads and spans, but not necessarily anchor bolt spacing.

ICFVL Spacing for Wood Ledger (in)

Specified	Load (psf)						Joist S	pan (ft)					
Live	Dead	10	12	14	16	18	20	22	24	26	28	30	32
	10	48	48	48	46	41	37	33	31	28	26	24	23
	15	48	48	48	42	38	34	31	28	26	24	22	21
40	20	48	48	45	39	35	31	28	26	24	22	21	19
	25	48	48	42	37	32	29	26	24	22	21	19	18
	30	48	46	39	34	30	27	25	23	21	19	18	17
	10	48	48	44	38	34	30	28	25	23	22	20	19
50	20	48	45	38	33	30	27	24	22	20	19	18	16
30	30	48	40	34	30	26	24	21	20	18	17	16	15
	40	43	36	30	27	24	21	19	18	16	15	14	13
	10	33	27	23	20	18	16	15	13	12	_	_	_
100	20	30	25	22	19	17	15	14	12				
100	30	28	24	20	18	16	14	13	12		_	_	_
	40	27	22	19	16	15	13	12	_	_		_	_

See notes below.

Values in the cells highlighted in yellow represent the maximum allowable spacing of 48".

ICFVL Spacing for Steel Ledger (in)

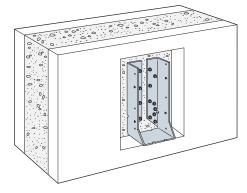
Specified	Load (psf)						Joist S	pan (ft)					
Live	Dead	10	12	14	16	18	20	22	24	26	28	30	32
	10	48	48	48	42	38	34	31	28	26	24	22	21
	15	48	48	45	39	35	31	28	26	24	22	21	19
40	20	48	48	41	36	32	29	26	24	22	20	19	18
	25	48	45	38	34	30	27	24	22	20	19	18	17
	30	48	42	36	31	28	25	23	21	19	18	17	15
	10	48	47	40	35	31	28	25	23	21	20	18	17
50	20	48	41	35	31	27	24	22	20	19	17	16	15
30	30	44	36	31	27	24	22	20	18	17	15	14	13
	40	39	33	28	24	22	19	18	16	15	14	13	12
	10	30	25	21	19	17	15	13	12	_	_	_	_
100	20	28	23	20	17	15	14	12	_				
100	30	26	22	18	16	14	13	12	_	_		_	_
	40	24	20	17	15	13	12	_	_	_	_	_	_

- Values shown are maximum spacing distances (inches) based on 2-span ledger and simple supported joists. It does not consider concentrated loads. The engineer of record can modify the spacing accordingly for other conditions.
- 2. Joist and ledger are to be designed by others.
- Table above address vertical loads only. If connection is designed to resist lateral loads, spacing will decrease. Contact Simpson Strong-Tie for current information.
- 4. The ICFVL must be installed no closer than 4" below the top of wall to achieve the connector spacing.
- The maximum distance between the end of the ledger and the first ICFVL is 12" as per the recommended splicing installation.
- Tables above assume Principal Loads only with Importance Factor = 1.00. For other cases adjust spacing accordingly.

Alternative Retrofit Solution for Direct Attachment of Joist to Wall

Catalogue C-C-CAN2015 © 2015 SIMPSON STRONG-TIE COMPANY INC

The HU and HUC hangers are heavy duty face mount joist hangers made from 14-gauge galvanized steel. These hangers can be directly attached to concrete wall using 1/4"x13/4" Simpson Strong-Tie® Titen® hex head screws. See page 200 for more information on installation and use.



HUC410 Installed on face of concrete in ICF

RTC/FWH Rigid Tie™ Connectors

SIMPSON

The RTC series secures two wood members to a vertical post forming a 90° corner. The RTC42 and RTC44 are heavy-duty structural connectors. See the table for post and joist sizes.

RTB—a bracket for intersecting 2x members.

FWH—4 way connectors for 2x members with bendable flanges.

RTA—connects two 2x wood members at a 90° angle.

RTF—connects two members in a "pass-through" application.

RTR & RTU—a 2x member crosses another.

MATERIAL: RTC44—14 gauge; RTA2—16 gauge;

RTR and RTB—20 gauge; all others—18 gauge

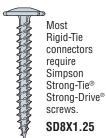
FINISH: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pages 14-17.

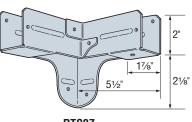
INSTALLATION:

- · Use all specified fasteners. See General Notes.
- · Install vertical members first, then attach horizontal members for easier alignment.
- Seat wood member in bracket with a C-clamp before securing to aid positioning and prevent skewing.
- · Always follow manufacturer's instructions when using power tools and building equipment.
- These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.
- These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

				Fastener	s (Total)		Normal (KD=1.00)
	Model No.	Post Size	Joist Size			D.Fir-L	S-P-F
	NU.	3126	3126	Post	Joist	lbs	lbs
						kN	kN
	FWH2	2x	2x	8-SD8x1.25	8-SDSx1.25	_	_
	ΓVVΠZ	ZX	ZX	0-5D0X1.25	0-9D9X1.20	_	_
	RTA12	1x	1x	4-SD8x1.25	4-SD8x1.25	_	_
	NIAIZ	IX	IX	4-3D0X1.23	4-3D0X1.23		_
	RTA2Z	2x	2x	4-SD8x1.25	4-SD8x1.25	_	_
	ITIAZZ	۷۸	۷۸	4-0D0X1.20	4-0D0X1.20		_
	RTA4	4x	4x	7-SD8x1.25	5-SD8x1.25		
	11174	47	44	7-0D0X1.23	J-0D0X1.23		_
	RTB22	2x	2x	4-SD8x1.25	4-SD8x1.25	_	
	111022			4 0D0X1.20	4 0D0X1.20	_	_
	RTC22Z	2x	2x	5-SD8x1.25	6-SD8x1.25	_	
	TTTOLLL			0 0D0X1.20	0 0D0X1.20	_	_
i 📄	RTC2Z	2x4	2x	6-SD8x1.25	6-SD8x1.25	1080	985
	111022	LAT	LX	0 0D0X1.20	0 0D0X1.20	4.80	4.38
		4x4	2x	14-SD8x1.25	8-SDSx1.25	1905	1750
	RTC42	-7.7		14 ODOX1.20	0 0D0X1.20	8.47	7.78
	111042	4x4	2x	14-10d	8-10dx1½	2700	2480
		TAT		14 10u	0 100/172	12.01	11.03
	RTC44	4x4	4x	14-10d	15-10d	3190	2980
		17(1	174	11 100	10 100	14.19	13.26
	RTF2Z	2x	2x	4-SD8x1.25	8-SDSx1.25		
	111122			4 ODOX1.20	O ODOX1.20	_	_
	RTT22	22 2x 2x 3-SD8x1.25		7-SD8x1.25			
		TEE EX EX O ODOX1.EC		O ODOX1.20	7 ODOX1.20		_
	RTR	2x	2x	2-SD8x1.25	4-SD8x1.25		
				_ 55020	. 55 020		_
	RTU2	2x	2x	2-SD8x1.25	4-SD8x1.25	_	
				_ 55020	. 32020	_	_

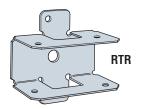
- 1. Factored loads must be equally distributed on both joists.
- 2. Factored resistances may not be increased for short-term loading.
- 3. **NAILS:** 10d = 0.148" dia. x 3" long, $10dx1\frac{1}{2} = 0.148$ " dia. x $1\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.

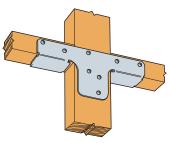


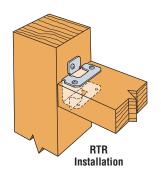


RTC2Z (RTC22 similar)

WARNING: Industry studies show that hardened fasteners can experience performance problems in wet and corrosive environments. Accordingly, use the SD8X1.25 screw in dry, interior, and non-corrosive environments only.

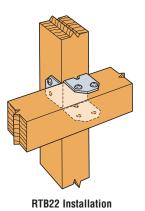


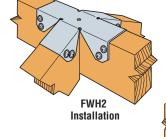


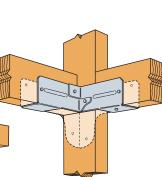












Typical RTC2Z Installation

/WW.TRICONTRISTINGS.CA

Miscellaneous

NS/NSP/PSPNZ Nail Stoppers

Nail Stoppers help prevent nails from piercing water pipes and electrical lines. Installed over utilities that pass through framing members.

MATERIAL: 16 gauge

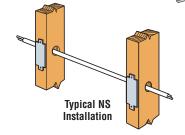
FINISH: Galvanized, PSPN-ZMAX® coating, see Corrosion Information, pages 14-17.

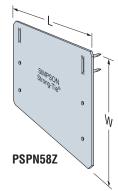
INSTALLATION: • NS/NSP/PSPN58Z—8d commons or prongs.

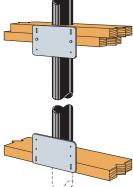
- PSPN516Z—16d commons
- For more information request F-PLUMBING.

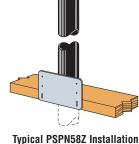
These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

	Model No.	W	L
	NS1	1½	3
	NS2	1½	6
	PSPN58Z	5	8
•	PSPN516Z	5	16





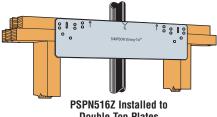




PSPN516Z **Installation to Sill Plate**

-165/16"-

NS



Double Top Plates

HSS/SS Stud Shoes

Stud Shoes reinforce studs notched in construction. They are NOT a total replacement of removed material.

HSS2-3 is designed for triple 2x studs. HSS Stud Shoes provide tension resistances as well as increased compression resistances. Flared flange provides greater strength. Installs over pipe up to 2%" outside diameter.

MATERIAL: 16 gauge FINISH: Galvanized

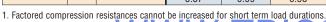
INSTALLATION: Use all specified fasteners. See General Notes.

- HSS: Bend flanges at 90° angle during installation, then bend back and screw into position (screws supplied).
- · Bend flanges one cycle only.

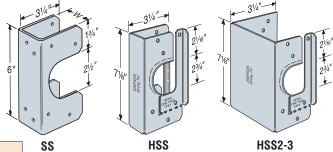
See pages 22-23 for other nail sizes

Catalogue C-C-CAN2015 © 2015 SIMPSON STRONG-TIE COMPANY INC.

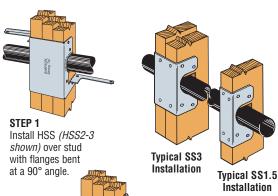
				Factored F	Resistance	
			D.Fi	r-L	S-P	-F
Model	Stud	Fasteners	Compresion	Tension	Compresion	Tension
No.	Size	rasiellers	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$
			lbs	lbs	lbs	lbs
			kN	kN	kN	kN
SS1.5	2x	12-10dx1½	875	_	620	_
331.3	ZX	1Z-1UUX 1 72	3.89	_	2.76	_
SS2.5	3x	12-10dx1½	1170	_	920	_
332.3	3.	12-10ux172	5.20	_	4.09	_
SS3	2-2x	12-10d	1255	_	970	_
333			5.58	_	4.31	_
SS4.5	3-2x	14-10d	1375	_	975	_
334.3	J-ZX	14-10u	6.12	_	4.34	_
HSS2-SDS1.5	2x	12-1/4"x11/2" SDS	1860	1450	1430	1040
11002-0001.0	21	12-74 X 1 1/2 SDS	8.27	6.45	6.36	4.63
HSS2-2-SDS3	2-2x	12-1/4"x3" SDS	1980	1370	1425	990
11002-2-0000	2-21	12-74 X3 3D3	8.81	6.09	6.34	4.40
HSS2-3-SDS3	3-2x	12-1/4"x3" SDS	1635	1370	1175	990
11002-0-0000	U-ZX	12-74 X3 3D3	7.27	6.09	5.23	4.40
HSS4-SDS3	4x	12-1/4"x3" SDS	1995	1370	1435	990
11004-0000	4X	12-1/4 X3 3D3	8.87	6.09	6.38	4.40

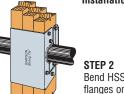


2. **NAILS:** 10d = 0.148 dia. x 3 long, 10dx11 = 0.148 dia



US Patent 6,176,057





Bend HSS (HSS2-3 shown) flanges one time only. crew into position.

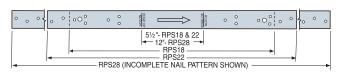
SIMPSON Strong-Tie

The RPS can be used to reinforce notches in wall plates for HVAC and pipe penetrations in walls.

FINISH: Galvanized, some products available in ZMAX® coating. See Corrosion Information, pages 14-17.

INSTALLATION: Use all specified fasteners. See General Notes.

- Use RPS22 or RPS28 (16 gauge) to reinforce top plate.
- Use RPS18Z, RPS22Z or RPS28Z (16 gauge ZMAX) to reinforce sill plate.

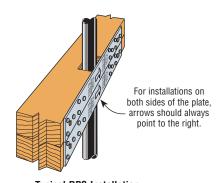


RPS

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

		Dimer	nsions			Factored Tensile Resistance					
Madal		(i	n)	Matak	Fastanava	D.F	ir-L	S-P-F			
Model No.	Ga			Notch Width	Fasteners (Total)	$(K_D=1.00)$	$(K_D = 1.15)$	$(K_D=1.00)$	$(K_D = 1.15)$		
140.		W	L	width (Total)	wittii	with (Total)	(Total)	lbs	lbs	lbs	lbs
					kN	kN	kN	kN			
RPS18		11/2	185/16	≤ 5½"	12-8d	1155	1325	1075	1240		
NF 310		1 72	10716	≥ 372	12-0u	5.14	5.90	4.79	5.52		
RPS22	16	11/2	225/16	≤ 5½"	16-8d	1535	1770	1435	1650		
nr322	10	1 72	22716	≤ 3 72	10-ou	6.84	7.88	6.39	7.35		
RPS28		11/2	285/16	≤ 12"	12-8d	1155	1325	1075	1240		
NF 320		1 72	20716	> 12		5.14	5.90	4.79	5.52		

- Factored resistances have been increased 15% for earthquake or wind loading. No further increase is permitted.
- 2. **NAILS:** 8d = 0.131" dia. $\times 2\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.



Typical RPS Installation (Only one strap may be necessary to meet code requirements)

CTS218 Compression and Tension Straps

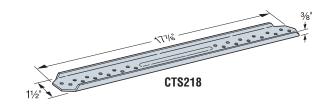
The CTS218 is designed to repair wood members such as top plates, studs and trusses and is our first strap that handles both tension and compression loads. The unique rolled edges of the strap allow it to span gaps as wide as $4\frac{1}{2}$ " and its $1\frac{1}{2}$ " width enables installation on the narrow face of 2x lumber.

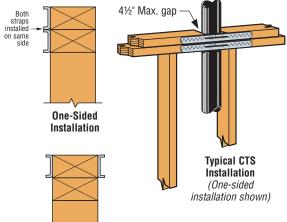
 Tested specifically for top/bottom plate repair with various multi-strap configurations

MATERIAL: 14 gauge FINISH: Galvanized

INSTALLATION: • Use all specified fasteners. See General Notes.

- One-sided installations install one or two CTS straps on the same side of the member.
- Two-sided installation install CTS straps on opposite sides of member. For three-part installations, install two parts on one side, one part on opposite side.





These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

					Factor	ed Resist	ance (K _D =1.15)
	Model	Ctron		Footomore	D.Fir-l	_	S-P-F	
	Model No.	Strap Qty.	Installation	Fasteners (per strap)	Compression	Tension	Compression	Tension
	140.	Qty.		(por strup)	lbs	lbs	lbs	lbs
					kN	kN	kN	kN
		1	One Sided		1485	1985	1055	1985
		'	One Sided		6.61	8.83	4.69	8.83
		2	One Sided		2970	3970	2110	3970
			One Sided		13.21	17.66	9.39	17.66
		2	Two Sided	24-10dx1½	3440	3970	2445	3970
			Z TWO Slued		15.30	17.66	10.88	17.66
		3	Two Sided		5405	5955	3840	5955
		J	TWO Sided		24.04	26.49	17.08	26.49
		4	Two Sided		6880	7940	4890	7940
函	CTS218		TWO Olded		30.60	35.32	21.75	35.32
9	010210	1	One Sided		1705	1985	1210	1985
			One olded		7.58	8.83	5.38	8.83
		2	One Sided		3410	3970	2420	3970
			One olded		15.17	17.66	10.77	17.66
		2	Two Sided	24-SD#9x1½	3970	3970	2820	3970
			TWO Olded	24 00// 3/172	17.66	17.66	12.54	17.66
		3	Two Sided		5995	5955	4255	5955
			1 440 01000		26.67	26.49	18.93	26.49
		4	Two Sided	A /\ A /	7940	-7940	5640	7940
		-T	I WO Sided	. / \ / \ / \ /	25 32	35 32	25 00	35 32

 Factored resistances have been increased 15% for wind or seismic with no further increase allowed. Reduce where other loads govern.
 Fastener quantities are for a single strap.

Two-Sided Installation

- 3. Maximum gap between wood members is 4½".
- 4. FASTENERS: 10dx1½ = 0.148" dia. x 1½", SD #9x1½ = 0.131" dia. x 1½" ong See pages 2-23 for other nail sizes and information.

PSCL/PSCA Panel Sheathing Clips

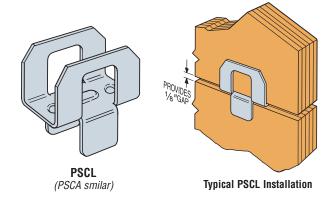
Simpson Strong-Tie® Panel Sheathing Clips are used to brace unsupported sheathing edges. The PSCA is a new version of the PSCL with less material for a more cost effective solution. Model sizes include: PSCL3/8, PSCA3/16, PSCL7/16, PSCA15/32, PSCL15/32, PSCL1/2, PSCL5/8, PSCL19/32, PSCL3/4.

MATERIAL: 20 gauge FINISH: Galvanized

INSTALLATION: • Use the same size sheathing clip as the panel thickness.

Model No.	Panel Thickness
PSCL%	3/8
PSCL7/16, PSCA7/16	7/16
PSCL ¹⁵ / ₃₂ , PSCA ¹⁵ / ₃₂	15/32
PSCL½	1/2
PSCL ⁵ %	5/8
PSCL ¹⁹ / ₃₂	19/32
PSCL¾	3/4

1. PSCLs and PSCAs meet the requirements of 9.23.16.6 NBC 2015 for required edge support of panel type sheathing.



RR Ridge Rafter Connector

An interlock provides alignment control and correct nailing locations. For a rafter-to-face connector, flatten the top flange into the face plane. The RR may be used with any rafter sloped up to 30°.

MATERIAL: 18 gauge FINISH: Galvanized

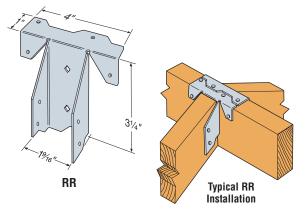
INSTALLATION: • Use all specified fasteners. See General Notes.

These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

		Fasteners		Factored Resistance				
				D.F	ir-L	S-I	P-F	
Model	Joist			Uplift	Normal	Uplift	Normal	
No.	Size	Header	Joist	$(K_D=1.15)$	$(K_D=1.00)$	$(K_D=1.15)$	$(K_D=1.00)$	
					lbs	lbs	lbs	lbs
				kN	kN	kN	kN	
RR	2x6	4-10dx11/2	4-10dx1½	185	685	130	490	
nn	2.00	4-10UX172	4-10UX172	0.82	3.05	0.58	2.18	



2. **NAILS:** $10dx1\frac{1}{2} = 0.148$ " dia. x $1\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.



DS Drywall Stop

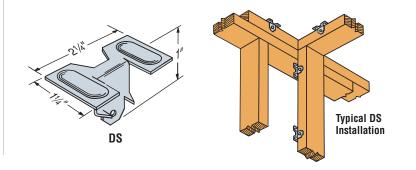
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Eliminates costly blocking at top plate, end walls, and corners. A typical residence will use several hundred of these inexpensive clips with a substantial savings in blocking and labour.

The installation prongs provide even more labour savings.

MATERIAL: 20 gauge FINISH: Galvanized INSTALLATION:

- 16" on center or less, using 8d commons.
- DS should not be used where gypsum board is used for structural loads.

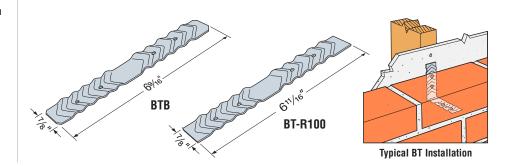


Brick Ties provide a connection between the wood structure and brick façade.

MATERIAL: 22 gauge FINISH: Galvanized INSTALLATION:

- Holes sized for 10d commons.
- See code for spacing and installation requirements.

TO ORDER: BT-R100 = retail pack of 100 BTB = bulk carton of 500



MP Mending Plates

Versatile and easy-to-use mending plates for wood-to-wood connections. No nails or notching of wood required. For non-structural applications only; not for truss applications.

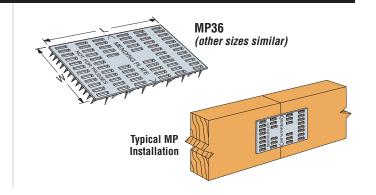
MATERIAL: 20 gauge FINISH: Galvanized

INSTALLATION:

- Place plate over two pieces of aligned wood with arrows aligned at joint.
- · Hammer the plate to embed the prongs.

Model	Dimens	ions (in)
No.	W	L
MP14	1	4
MP24	2	4
MP36	3	6

1. Connectors are not load rated.



TP/TPA Tie Plates

TPs are nail-on tie plates. TPAs are flanged for added support.

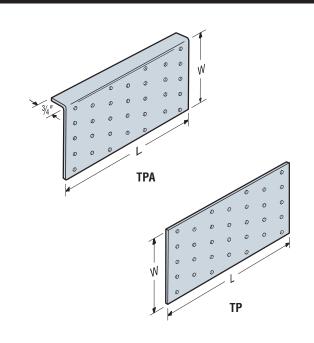
MATERIAL: 20 gauge FINISH: Galvanized

INSTALLATION: • Holes are sized for 8d common or 8dx11/2" nails.

These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

Model	Dimens	ions (in)	Number of
No.	W	L	Nail Holes
TP15	1 13/16	5	13
TPA37	3½	7	32
TPA39	3½	9	41
TP35	31//8	5	23
TP37	31//8	7	32
TP39	31//8	9	41
TP311	31//8	11	50
TP45	41//8	5	30
TP47	41/8	7	42
TP49	41//8	9	54
TP411	41/8	11	66
TP57	5¾	7	60
TPA57	5	7	49

1. Connectors are



Miscellaneous

Simpson Strong-Tie® Wall Bracing products offer effective options to resist racking during construction. Not designed to replace structural panel shearwall load-carrying component.

The WBC (coiled WB) multiple product dispenser pack weighs less than 40 pounds, making storage and transportation easy. WB106C—15 pieces per roll, WB126C—12 pieces per roll, WB143C-10 pieces per roll.

The RCWB features a rolled edge (the TWB has two rolled edges) for extra strength and safety.

MATERIAL: WB and WBC—16 gauge; TWB—22 gauge; RCWB-20 gauge

FINISH: Galvanized

INSTALLATION: • Use all specified fasteners. See General Notes.

- Install in "X" pairs or in opposing "V" fashion.
- Use with 16" or 24" o.c. 2x4 (min.) studs.

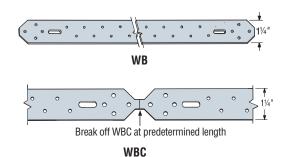
RCWB and TWB:

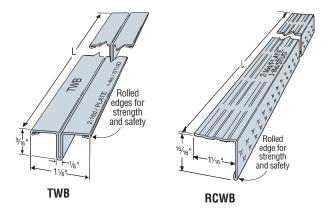
- Use with 16" o.c. studs.
- Use minimum of 2x4 studs with TWB.
- . Use minimum of 2x6 studs with RCWB (2x4 min. for interior, non-bearing wall).
- · Establish a run-line using the bracing as a straight edge. Single cut a saw kerf %" deep (TWB) or 11/8" deep (RCWB) along the run line. If the wall is pre-framed on the floor, place the part into the saw kerf, and put one nail into the top plate. Tilt the wall up and plumb before nailing off top plate, bottom plate and studs according to the nailing schedule.

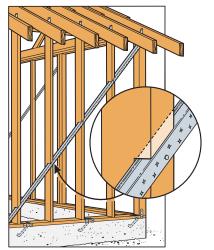
Model	L	Angle and	Faste	eners
No.	L	Wall Height	Plates	Studs
WB106	9'-5%"	8' @ 60	2-16d	1-8d
WB106C	9'-6"	8' @ 60	2-16d	1-8d
TWB10	9'-9"	8' @ 55	2-16d	1-8d
RCWB12	11'-4"	8' @ 45	2-16d	1-8d
WB126	11'-4%"	8' @ 45	2-16d	1-8d
WB126C	11'-4¾"	8' @ 45	2-16d	1-8d
TWB12	11'-4"	8' @ 45	2-16d	1-8d
RCWB12	11'-4"	9' @ 53	2-16d	1-8d
WB126	11'-4%"	9' @ 53	2-16d	1-8d
WB126C	11'-4¾"	9' @ 53	2-16d	1-8d
TWB12	11'-4"	9' @ 53	2-16d	1-8d
WB143C	14'-3"	10' @ 45	2-16d	1-8d
RCWB14	14'-2"	10' @ 45	2-16d	1-8d
TWB14	14'-2"	10' @ 45	2-16d	1-8d

1. **NAILS:** 16d = 0.162" dia. $\times 3\frac{1}{2}$ " long, 8d = 0.131" dia. $\times 2\frac{1}{2}$ " long. See pages 22-23 for other nail sizes and information.

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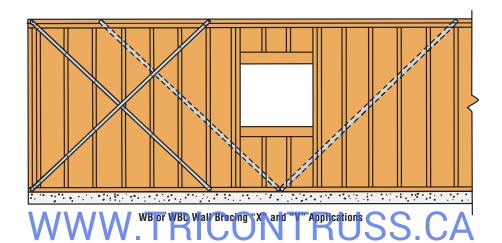






The WBC Handy Carry Carton is convenient to store. transport and use.

Typical RCWB Installation



NCA/TB/LTB Bridging



NCA—Nailless installation eliminates callbacks for nail squeaks. Designed for secure grip before the drive-home blow, and deeper prong penetration. Precision-formed into a rigid "V" section.

TB—Tension-type bridging with maximum nailing flexibility. Use just two of the seven nail holes at each end.

LTB—Staggered nail pattern accommodates 2x8 and 2x10 joists. Use just two of the six nail holes at each end. LTB40 has rigid prongs that install easily into the joist, and embossments that allow crisp bends.

MATERIAL: LTB—22 gauge; NCA and TB—20 gauge (except NCA2x12-16—18 gauge).

FINISH: Galvanized

INSTALLATION: • Support floor joists with a depth-to-thickness ratio of six or more with bridging at intervals not exceeding 8'. If span is greater than 8', install on 2x8 or larger joists. If span is greater than 16', use more than one pair.

- Tension bridging works only in tension, so must be used in cross pairs.
- Install bridging tightly: loose installation may allow floor movement.
- NCA may be installed before or after sheathing, from the top or bottom. Simply locate the bend line approximately 1" from the joist edge.
- NCA has nail holes in one end for use if a prong is bent during installation. Fully seat nails if they are used; otherwise, they may lead to squeaks.
- TB requires two 10dx1½" fasteners per end.
- LTB requires two 6d commons per end.
- Nail Bridging Only—When installation for the connection to the top of the stud wall instead of the joist underside, use a strap one size smaller than shown in the table.

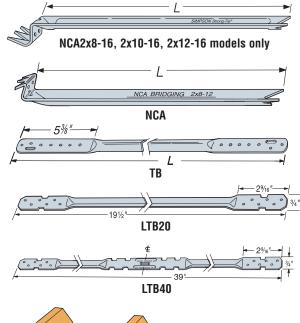
TENSION BRIDGING FOR I-JOISTS

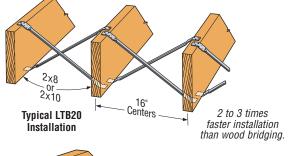
Joist				Jois	t Spacing	g (in)			
Height (in)	12	16	19.2	24	30	32	36	42	48
9½	TB20	TB27	TB27	TB30	TB36	TB36	TB42	TB48	TB54
10	TB20	TB27	TB27	TB30	TB36	TB36	TB42	TB48	TB54
11%	TB20	TB27	TB27	TB30	TB36	TB36	TB42	TB48	TB54
12	TB20	TB27	TB27	TB30	TB36	TB36	TB42	TB48	TB54
14	TB27	TB27	TB27	TB36	TB36	TB42	TB42	TB48	TB54
16	TB27	TB27	TB30	TB36	TB42	TB42	TB42	TB48	TB54
18	TB27	TB30	TB30	TB36	TB42	TB42	TB48	TB54	TB56
20	TB30	TB30	TB36	TB36	TB42	TB42	TB48	TB54	TB56
22	TB30	TB36	TB36	TB36	TB42	TB42	TB48	TB54	TB56
24	TB36	TB36	TB36	TB42	TB42	TB48	TB48	TB54	TB56
26	TB36	TB36	TB36	TB42	TB48	TB48	TB48	TB54	TB60
28	TB36	TB36	TB42	TB42	TB48	TB48	TB54	TB54	TB60
30	TB36	TB42	TB42	TB42	TB48	TB48	TB54	TB56	TB60
32	TB42	TB42	TB42	TB42	TB48	TB48	TB54	TB56	TB60

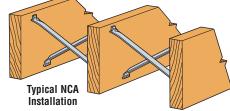
TENSION BRIDGING FOR SOLID SAWN LUMBER

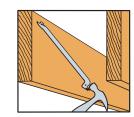
laist		NCA		ТВ	LTB	
Joist Size	Spacing	Model No.	L (in)	Model No.	L (in)	Model No.
2x10	12" o.c.	NCA2x10-12	12½	TB20	20	_
2x12	12" o.c.	NCA2x12-12	13%	TB20	20	_
2x14	12" o.c.	NCA2x8-16	151/4	TB27	27	_
2x16	12" o.c.	NCA2x10-16	15 ¹³ ⁄ ₁₆	TB27	27	_
2x8	16" o.c.	NCA2x8-16	151/4	TB27	27	LTB20 or 40
2x10	16" o.c.	NCA2x10-16	15 ¹³ ⁄ ₁₆	TB27	27	LTB20 or 40
2x12	16" o.c.	NCA2x12-16	167//8	TB27	27	

Space bridging to avoid contact noises.



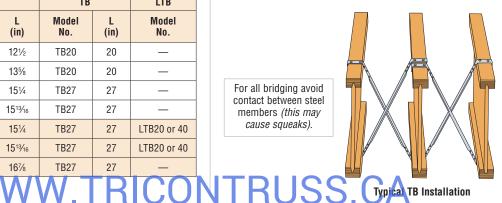






Install from below as shown, or from above. Drive upper end into joist approximately 1" from the top.

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Moisture

Barrier

Not Shown

The Architectural Products Group consists of aesthetically pleasing, pre-finished connectors and innovative concealed joist ties designed for exposed wood applications. These connectors provide structural performance and, at the same time, add a unique appearance feature to a project. Refer to Simpson Strong-Tie® C-APG catalogue.

• ARCHITECTURAL FINISHES

Eliminate time consuming prep work and costly field painting. Available finishes include textured flat black powder-coat, gray paint and hot-dip galvanized coating.

AVAILABILITY

Select products are in stock and readily available. Contact Simpson Strong-Tie for product availability and lead times for non-stocked items.

• PRE-ENGINEERED AND TESTED

Load-rated products are verified to perform to design loads, unlike custom designed and fabricated connectors.

QUALITY ASSURANCE

No-Equal quality-controlled manufacturing ensures product consistency and high quality.



Products shown in this section come with textured flat black powder-coat unless otherwise noted. Most are also available with a galvanized coating or gray primer. Contact Simpson Strong-Tie for availability.

www.strongtie.com/apg

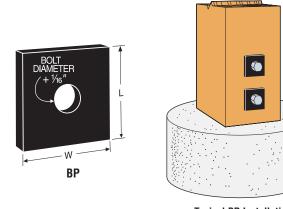
BP - BEARING PLATES

Bearing Plates give greater bearing surface than standard cut washers, and help distribute the load at these critical connections.

MATERIAL: See table

FINISH: Textured flat black powder-coat INSTALLATION: See General Notes.

Model	Thickness	Dimens	Bolt Dia.	
No.	(in)	W	L	(in)
BP½PC	3/16	2	2	1/2
BP%-2PC	3/16	2	2	5/8
BP%PC	1/4	21/2	21/2	5/8
BP%PC	5/16	23/4	23/4	3/4
BP%PC	5/16	3	3	7/8
BP1PC	3/8	3½	3½	1



Typical BP Installation

SPECIAL ORDER PLATES

Simpson Strong-Tie can make a variety of flat and bent steel shapes, which include gusset plates for heavy timber trusses, custom ornamental shapes and retaining plates.

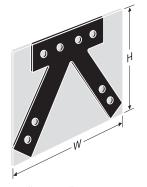
MATERIAL: 3 gauge maximum

FINISH: Galvanized, textured powder-coated flat black, Simpson Strong-Tie® gray paint, stainless steel. Contact Simpson Strong-Tie for availability.

TO OBTAIN A QUOTE:

Catalogue C-C-CAN2015 © 2015 SIMPSON STRONG-TIE COMPANY INC

- · Supply a CAD drawing in .dxf format complete with plate dimensions, hole diameter and locations, steel thickness, desired coating (Simpson Strong-Tie Gray Paint, Black Powder-Coat, HDG or raw steel).
- . Total plate shape and size up to max. dimensions of 48"x48" (approx. 1/16" tolerance).
- · Simpson Strong-Tie does not provide product engineering or load values for Custom Steel Plates.
- · Contact Simpson Strong-Tie for pricing information



"W" and "H" indicate the envelope size of the steel shape.



Typical Installation (Plate shown has black powder-coat)

SIMPSON

CLASSIC COLLECTION

MATERIAL: As noted in tables

FINISH: Textured powder-coated flat black paint

INSTALLATION: • Use all specified fasteners. See General Notes.

STRAP TIES

Model	Ga	Dimens	ions (in)	Bolts		
No.	ua	W L		Qty.	Dia.	
HST2PC	7	2½	211/4	6	5/8	
HST5PC	7	5	211/4	12	5/8	
HST3PC	3	3	25½	6	3/4	
HST6PC	3	6	25½	12	3/4	
PS218PC	7	2	18	4	3/4	
PS418PC	7	4	18	4	3/4	
PS720PC	7	6¾	20	8	1/2	

BEAM TO COLUMN TIES

Model No.	Ga	Dimensions (in)			End &	ım Bolt Edge ces (in)	Bolts		
		W	Н	L	d ₁	d_2	Qty.	Dia.	
1212HLPC	7	2½	12	12	2½	4%	5	5/8	
1616HLPC	7	2½	16	16	21/2	4%	5	5/8	
1212HTPC	7	2½	12	12	2½	4%	6	5/8	
1616HTPC	7	2½	16	16	21/2	43/8	6	5/8	

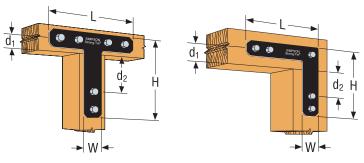
1. 1212HL, 1616HL, 1212HT and 1616HT are to be installed in pairs with machine bolts in double shear.

COLUMN BASES

INSTALLATION: • Minimum side cover is 3" for CB's.

- · Install with bottom of base flush with concrete.
- · Post bases do not provide adequate resistance to prevent members from rotating about the base and therefore are not recommended for non top-supported installations (such as fences or unbraced carports).

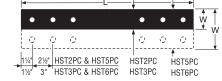
Model	Ga	Dimens	ions (in)	Bolts		
No.	ua	W ₁	W ₂	Qty.	Dia.	
CB44PC	7	3%16	3½	2	5/8	
CB46PC	7	3%16	5½	2	5/8	
CB48PC	7	3%16	71/2	2	5/8	
CB66PC	7	5½	5½	2	5/8	
CB68PC	7	5½	71/2	2	5/8	
CB88PC	3	7½	71/2	2	3/4	
CB810PC	3	7½	9½	2	3/4	



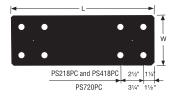
Typical 1212HTPC Installation

(1616HTPC similar)

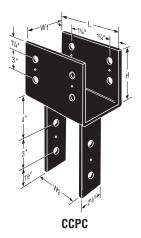
Typical 1212HLPC Installation (1616HLPC similar)

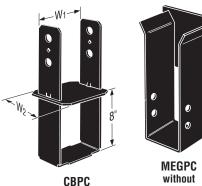


HSTPC

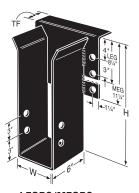


PSPC





without **Top Flange**



LEGPC/MEGPC

Catalogue C-C-CAN2015 © 2015 SIMPSON STRONG-TIE COMPANY INC

BEAM HANGERS

MATERIAL: Top flange-7 ga, Stirrups-7 ga.

	Dim	ensions	s (in) Bolts				
Model No.	w	Min.	TF	Header		Joist	
1101	VV	Н		Qty.	Dia.	Qty.	Dia.
LEG3PC	31/4	9	2½	4	3/4	2	3/4
LEG5PC	51/4	9	21/2	4	3/4	2	3/4
MEG5PC	51/4	9	21/2	6	3/4	2	3/4
LEG7PC	67/8	9	21/2	4	3/4	2	3/4
MEG7PC	67/8	9	21/2	6	3/4	2	3/4

1. See Glulam Connectors section of this catalogue for

COLUMN CAPS

INSTALLATION: • Bolt holes shall be a minimum of \mathcal{V}_{32} " to a maximum of 1/16" larger than the specified bolt's diameter (12.4.1.2 CSA 086-14).

			Dimer	nsions			Во	Its	
Model No.	Ga		(in)			Be	am	Po	st
		W ₁	W ₂	L	Н	Qty.	Dia.	Qty.	Dia.
CC44PC	7	3%	3%	7	4	2	5/8	2	5/8
CC46PC	7	3%	5½	11	6½	4	5/8	2	5/8
CC66PC	7	5½	5½	11	6½	4	5/8	2	5/8
CC68PC	7	5½	71/2	11	6½	4	5/8	2	5/8
CC88PC	3	71/2	71/2	13	8	4	3/4	2	3/4



RUSTIC COLLECTION

MATERIAL: As noted in tables

FINISH: Textured powder-coated flat black paint

INSTALLATION: • Use all specified fasteners. See General Notes.

STRAP TIES

Model			ions (in)	Bolts		
No.	ua	W L		Qty.	Dia.	
OS	12	2	12	4	1/2	
OHS	7	21/2	12	4	5/8	
0HS135	7	6	13½	4	3/4	
OHS195	7	6	19½	8	3/4	

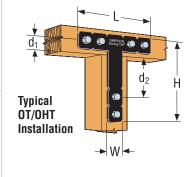
BEAM TO COLUMN TIES

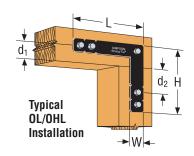
Model No.	Ga	Dimensions (in)			End &	ım Bolt Edge inces	Bolts		
		W	Н	L	d ₁	d ₂	Qty.	Dia.	
OL	12	2	12	12	2	3½	5	1/2	
OHL	7	2½	12	12	2½	4%	5	5/8	
OT	12	2	12	12	2	3½	6	1/2	
OHT	7	2½	12	12	2½	43/8	6	5/8	

1.OL, OHL, OT and OHT must be installed in pairs with machine bolts in double shear.

HEAVY ANGLES

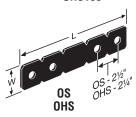
Model	Ga	Dimens	ions (in)	Bolts		
No.	ua	W	L	Qty.	Dia.	
OHA33	7	31//8	3	2	3/4	
OHA36	7	31//8	6	4	3/4	

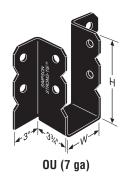


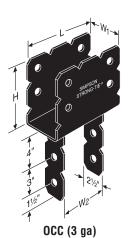












COLUMN BASES

Model	Ga	Dimens	ions (in)	Bolts		
No.	ua	W ₁	W ₂	Qty.	Dia.	
OCB44	3	3%16	3½	2	5/8	
OCB46	3	3%16	5½	2	5/8	
OCB48	3	3%16	71/2	2	5/8	
OCB66	3	5½	5½	2	5/8	
OCB68	3	5½	71/2	2	5/8	
OCB88	3	7½	7½	2	3/4	
OCB810	3	7½	9½	2	3/4	

1. Minimum side cover is 3" for OCB's.

COLUMN CAPS

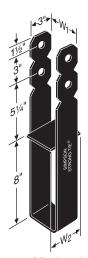
			Dimensions				Bolts				
Model No.	Ga		(in)			Be	am	Post			
110.		W ₁	W ₂	L	Н	Qty.	Dia.	Qty.	Dia.		
OCC44	3	3%	35/8	9	41/2	2	5/8	2	5/8		
OCC46	3	3%	5½	12	71/2	4	5/8	2	5/8		
00066	3	5½	5½	12	71/2	4	5/8	2	5/8		
00068	3	5½	71/2	12	71/2	4	5/8	2	5/8		
00088	3	71/2	7½	15	71/2	4	3/4	2	3/4		

1. For end conditions specify OECC.

JOIST HANGERS

Model	Dimens	ions (in)	Во	Its
No.	W	Н	Header	Joist
0U46	3%16	5	2-3/4	1-3/4
OU48	3%16	7	4-3/4	2-3/4
0U410	3%16	9	4-3/4	2-3/4
0U412	3%16	11	6-3/4	3-3/4
0U414	3%16	13	6-3/4	3-3/4
OU68	5½	7	4-3/4	2-3/4
OU610	5½	9	4-3/4	2-3/4
OU612	5½	11	6-3/4	3-3/4
0U614	5½	13	6-3/4	3-3/4
OU810	7½	9	4-3/4	2-3/4
OU812	7½	11	6-3/4	3-3/4
0U814	7½	13	6-¾	3-¾

1. Glulam beam sizes are available. Add an "X" to the name and specify width, i.e. OU68X, W = 5.25".



OCB (3 ga)

Architectural Products Group



STANDOFF BASES

FEATURES: • Designed for increased concrete surface area

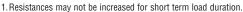
- Corrosion resistant
- · Sized for 10d nails
- Can be used with rough lumber

MATERIAL: Engineered composite plastic.

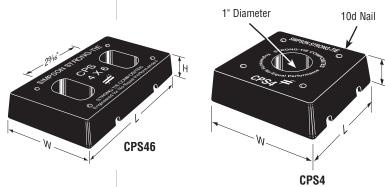
INSTALLATION: • See General Notes.

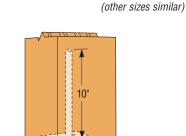
- Not recommended for non-top-supported installations such as fences.
- Attach to post before installation using four nail holes.
- Embed minimum ½" diameter rod into concrete and extend into wood member (2 rods required for CPS46).
- For nominal or rough sawn lumber.

Model	Post or Column	Dimensions (in)		Factored Compressive Resistance	
No.	Size		н	lbs	
		-	VV	"	kN
CPS4	4x4	31/4	31/4	1	5685
UF34	484	374	374	'	25.32
CPS46	546 4x6 55/16 35/16 1	-1	8065		
UF340	480	3 916	3916	'	35.92
CPS5	5x5	41/8	41/8	1	6945
6255	SXS	4 78	4 78	ı	30.94
CPS6	eve.	E5/	E5/	4	10655
UP30	6x6 55/16 55/16 1		47.46		
CPS7	67 8x8 7¼ 7¼ 1¼	11/	11430		
UF3/		1 74	1 74	1 74	50.91



2. Resistance is calculated based on the CPS bearing area and concrete strength of 20 MPa.







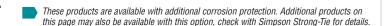
HL - HEAVY ANGLES & GUSSETS

Versatile angle gussets and heavy angles promote standardization and construction economy, and are compatible with Simpson Strong-Tie structural hardware.

FINISH: Textured flat black powder-coat, Simpson Strong-Tie® gray paint and also available galvanized

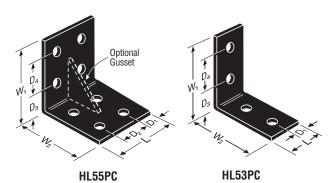
TO ORDER: All products with PC suffix are textured powder-coated flat black paint. 7 gauge products without the PC suffix are galvanized. 3 gauge products without the PC suffix are Simpson Strong-Tie gray paint.

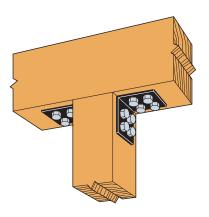
OPTIONS: Gussets may be added to HL models when $L \ge 5$ ". Specify G after numbers in model number as in HL46GPC.



Dimensions (in)

Model	0.		Bolts (Total)						
No.	Ga	W ₁ & W ₂	L	D ₁	D ₂	D ₃	D ₄	Qty.	Dia.
HL33PC	7	31/4	21/2	11/4	_	2	_	2	1/2
HL35PC	7	31/4	5	11/4	21/2	2	_	4	1/2
HL37PC	7	31/4	7½	11/4	21/2	2	_	6	1/2
HL53PC	7	5¾	2½	11/4	_	2	2½	4	1/2
HL55PC	7	5¾	5	11/4	21/2	2	21/2	8	1/2
HL57PC	7	5¾	7½	11/4	21/2	2	21/2	12	1/2
HL43PC	3	41/4	3	1½	_	2¾	_	2	3/4
HL46PC	3	41/4	6	1½	3	2¾	_	4	3/4
HL49PC	3	41/4	9	1½	3	23/4	_	6	3/4
HL73PC	3	71/4	3	1½		2¾	3	4	3/4
HL76PC	3	71/4	6	1½	3	23/4	3	8	3/4
HL79PC	3	71/4	1 9	11/2	13/	23/4	3	12	3/4





Typical HL55PC Installation



ORNAMENTAL – JOIST HANGER

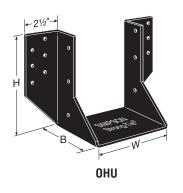
The OHU Ornamental Joist Hangers are heavy duty, load-rated joist hangers that are attached with Simpson Strong-Tie® ¼"x3" Strong-Drive® double-barrier coating SDS Heavy-Duty Connector screws (supplied with product).

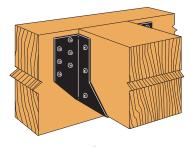
MATERIAL: 12 gauge

Catalogue C-C-CAN2015 © 2015 SIMPSON STRONG-TIE COMPANY INC.

FINISH: Textured powder-coated flat black paint

OPTIONS: No modifications.





Typical OHU Installation

						No	. of	Factored Resistance					
			0	imensior (in)	IS	SDS 1	/4"x3"	D.F	ir-L	S-P-F			
Joist	Model	Ga		. ,		Wood	orews	Uplift	Normal	Uplift	Normal		
Size	No.	ua						(K _D =1.15)	(K _D =1.00)	(K _D =1.15)	(K _D =1.00)		
			W	Н	В	Face	Joist	lbs	lbs	lbs	lbs		
								kN	kN	kN	kN		
4x6	OHU46-SDS3	12	3%16	5	4	6	4	2415	3885	2080	2795		
470	011040-3033	12	3716	3	4	0	4	10.74	17.28	9.25	12.43		
4x8	OHU48-SDS3	12	3%16	63/4	4	8	6	2890	3885	2080	2795		
470	011040-3033	12	J / 16	074	4	0	0	12.86	17.28	9.25	12.43		
4x10	0HU410-SDS3	12	3%16	83/4	4	12	6	3620	8175	3275	5885		
4810	0110410-3033	12	3 / 16	074	4	12	0	16.10	36.37	14.57	26.18		
4x12	0HU412-SDS3	12	3%16	10¾	4	12	8	4755	8175	3425	5885		
4812	0110412-3033	12	3716	1074	4	12	0	21.15	36.37	15.24	26.18		
4x14	0HU414-SDS3	12	3%16	12¾	4	14	10	4755	8175	3425	5885		
4814	0110414-3033	12	3716	1274	4	14	10	21.15	36.37	15.24	26.18		
6x6	OHU66-SDS3	12	5½	5	4	6	4	2415	3885	2080	2795		
000	OHU00-3D33	12	372	5	4	0	4	10.74	17.28	9.25	12.43		
6x8	OHU68-SDS3	12	5½	7	4	12	6	3620	8175	3275	5885		
000	UHU00-3D33	12	372	'	4	12	0	16.10	36.37	14.57	26.18		
6x10	0HU610-SDS3	12	5½	9	4	14	6	3620	8175	3275	5885		
OXIU	000010-5053	12	372	9	4	14	0	16.10	36.37	14.57	26.18		
6x12	0HU612-SDS3	12	5½	11	4	16	0	4830	9435	4370	6795		
UXIZ	0110012-3033	12	372	''	4	10	8	21.49	41.97	19.44	30.23		
6x14	0HU614-SDS3	12	5½	13	4	18	10	6035	9435	5360	6795		
0X14	0110014-3033	12	372	13	4	10	10	26.85	41.97	23.84	30.23		

^{1/4&}quot;x3" SDS HEAVY-DUTY CONNECTOR Screw U.S. Patent 6,109,850; 5,897,280; 5,044,853

Strong-Drive®



^{1.} Factored resistances have been increased 15% for wind or earthquake loading with no further increase allowed.



The CJT is a concealed connector. It can be installed three ways: with no routing of header/post or beam; a routed header/post, or a routed beam.

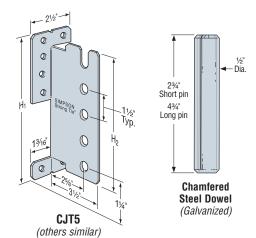
MATERIAL: 12 gauge FINISH: Galvanized

INSTALLATION: • Use all specified fasteners.

See General Notes.

- The CJT Pack is supplied with all dowels and screws required. Screws require a hex head driver.
- Router end of beam for screw heads for flush installation.
- To provide maximum beam width for use with short dowels, center in beam.
- The joist/beam may be sloped to 45° with full tabulated resistances.
- Request F-CJT flier for installation instructions and ordering information

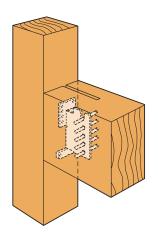
OPTIONS: Order short or long dowels, eg. CJT3S or CJT3L.



WARNING:

This connector requires special attention to ensure correct installation. The beam must be installed perpendicular to the support member. The connection's components may be damaged if the beam is rotated from its opposite end during or after installation. Damaged components may not be noticeable and may reduce the connector's load carrying capacity.

		Dime	nsions	Fasteners		Factored Resistance						
	N/I:	(i	n)	Faste	eners	Short	Dowels	Long [Dowels			
Model	Min. Joist					Uplift	Normal	Uplift	Normal			
No.	Size			SDS	1/2"	(K _D =1.15)	(K _D =1.00)	(K _D =1.15)	(K _D =1.00)			
	(in)	H ₁	H ₂	1/4"x3" Screws	Dia. Dowels	lbs	lbs	lbs	lbs			
				SULEMS	DOMEIS	kN	kN	kN	kN			
					D.Fir-l							
					J	2580	2510	2580	2510			
CCJT3	4x8	5%16	47/16	6	3	11.48	11.17	11.48	11.17			
						3935	3470	3935	4200			
CJT4	4x10	7	515/16	8	4	17.50	15.44	17.50	18.68			
						4535	3945	4940	5065			
CJT5	4x12	8%16	71/16	10	5	20.17	17.55	21.98	22.53			
								5900	5130			
CJT6	6x12	10	815/16	12	6	_	_	26.25	22.82			
).Fir-L Gl	ulam		20.20	LL.UL			
				<u>.</u>	7.1 II-L UII	2460	2140	2580	2660			
CJT3	31/8x71/2	5%16	47/16	6	3		9.52					
						10.94 3055	2655	11.48 3935	11.83			
CJT4	31/8x9	7	5 ¹⁵ / ₁₆	8	4	13.59	11.81	17.50	4200 18.68			
						3635	3160	4940	5215			
CJT5	31/8x101/2	8%16	71/16	10	5	16.17	14.06	21.98	23.20			
							4190	3640	6910	6005		
CJT6	31/x12	10	815/16	12	6	18.64	16.19	30.74	26.71			
				Cnr	uce-Pine		10.13	30.74	20.71			
				əhr	uce-Pine		4075	1055	1015			
CJT3	31/8x71/2	5%16	47/16	6	3	1855	1875	1855	1915			
						8.25	8.34	8.25	8.52			
CJT4	31/8x9	7	5 ¹⁵ / ₁₆	8	4	2670	2325	2830	3505			
						11.88	10.34	12.59	15.59			
CJT5	31/8x101/2	8%16	77/16	10	5	3180	2765	3555	4560			
						14.15	12.30	15.81	20.28			
CJT6	31/8x12	10	815/16	12	6	3665 16.30	3185 14.17	6045 26.89	5255 23.38			
							14.17	26.89	23.38			
					Paralla		0.1	0.00	0.1==			
CJT3	3½x9½	5%16	47/16	6	3	2580	3150	2580	3150			
					-	11.48	14.01	11.48	14.01			
CJT4	3½x9½	7	515/16	8	4	3935	4085	3935	4085			
						17.50	18.17	17.50	18.17			
CJT5	3½x11%	8%16	71/16	10	5	4940	5250	4940	5250			
						21.98	23.35	21.98	23.35			
CJT6	3½x11%	10	815/16	12	6	7245	6300	7245	6300			
						32.23	28.02	32.23	28.02			



Typical CJT Installation (Note that dowels should be centered within beam)



Strong-Drive® 1⁄4"x3" SDS HEAVY-DUTY CONNECTOR Screw

U.S. Patent 6,109,850; 5,897,280; 5,044,853

 Center dowel in beam. Short dowel (2¾") for use with 3¼ GLB, 4x sawn lumber or 3½" wide PSL. Long dowel (4¾") for use with 5½ GLB, 6x sawn lumber or greater widths.

WWW.TRICONTRUSS.CA



ETB - HIDDEN CONNECTOR KIT

The ETB hidden connector provides a load-tested beam connection without any visible hardware. Interlocking plates are fastened onto each member and lock together for a secure structural connection.

MATERIAL: Plates—Aluminum 6082-T6

FINISH: Plates—none; Screws—Dacromet® corrosion resistant coating

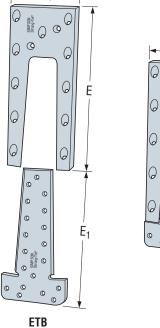
INSTALLATION: • Use all specified fasteners. See General Notes

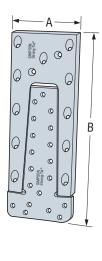
- Rout a %" (10mm) deep pocket into the side of the supporting beam as shown for the lower plate.
- Install lower plate with 16d hot dip galvanized nails (not included in kit).
- Install horseshoe plate onto end of supported beam using SCRB screws supplied in kit. Screws are installed at a downward angle (approx. 45°).
- Templates are available to make accurate installation more efficient.

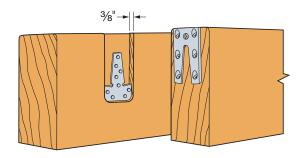
Dimensional and Fabrication Information

Model	Dimensions (in)									
No.	A	В	E	E ₁	t ₁	t ₂				
ETB90	2.36	3.62	2.72	2.26	0.236	0.393				
ETB160	2.36	6.61	5.12	3.74	0.236	0.393				
ETB230	2.95	9.13	7.87	5.43	0.236	0.393				

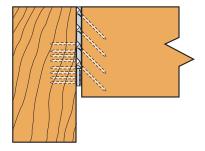
	Гол	ntonoro		Factored	Resistance (I	(_D = 1.00)
	Fas	steners		D.Fir-L	S-P-F	Northern
Model No.			Joist Size	$(K_D=1.00)$	$(K_D = 1.00)$	$(K_D=1.00)$
NO.	Header	Joist	3126	lbs	lbs	lbs
				kN	kN	kN
			40	1870	1525	1325
			4x6	8.34	6.78	5.89
			4x8	1675	1325	1145
				7.45	5.89	5.09
ETB90			4,40	1545	1220	1060
			4x10	6.87	5.43	4.72
			4x12	1415	1120	970
	6-16dHDG	5-SPAX 5x80	4812	6.29	4.98	4.31
	0-10UHDG	SCRB screw	6x6	1870	1640	1385
			OXO	8.32	7.30	6.16
			6x8	1870	1640	1385
			UXO	8.32	7.30	6.16
			6x10	1870	1535	1280
			UXIU	8.32	6.83	5.69
			6x12	1755	1410	1175
			0.112	7.81	6.27	5.23
			4x8	3430	2965	2545
			4x8	15.26	13.19	11.32
			4x10	3430	2740	2375
			4710	15.26	12.19	10.56
			4x12	3175	2510	2175
FTR160	11-16dHDG	10-SPAX 5x80	77.12	14.12	11.17	9.68
LIBIOO	TT TOUTIDG	SCRB screw	6x8	3430	2985	2545
			0.00	15.26	13.28	11.32
			6x10	3430	2985	2545
			3710	15.26	13.28	11.32
			6x12	3430	2985	2545
			0A12	15.26	13.28	11.32
			4x12	4925	3890	3375
ETR230	19-16dHDG	16-SPAX 5x80	7/17	20.48	17.30	15.01
L10230	13-1001100	SCRB screw	6x12	5930	4420	3770
		1 1 / 1	X17	23.24	19.66	677







Typical ETB Installation



ETB Installation with Non-Routed Header (Side View)

- Factored resistances assume standard term load duration. Reduce value where other load durations govern. Do not increase value for short term load duration.
- Factored resistances have been calculated in accordance with CSA 086-14
 assuming dry service condition (Ks = 1.00) and seasoned lumber (moisture
 content < 15%) at time of fabrication. For unseasoned lumber (moisture
 content > 15%) multiply tablulated values by 0.80. For wet service condition
 multiply tablulated values by 0.67.
- 3. Do not use ETB connectors with preservative-treated woods.
- Factored resistances shown are the lower of the test value, the fasterner capacity or the effective shear capacity of the joist assuming joist and headers are same species.
- 5. Substitution for fasteners is not permitted. All fasteners must be used
- as specified. 5. NAILS: 16dH DG = 0.162" dia. x 3½" long hot-dig galvanized.

FACE-MOUNT HANGER OPTION MATRIX



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		Н	ANGER MODIFI	CATION OPTION	S		APPLICATIONS	
	SKEWE	D SEAT						
BASE MODEL SERIES	ALLOWABLE SKEW	SQUARE CUT JOIST ALLOWED	SLOPED SEAT	SKEWED & SLOPED SEAT	CONCEALED FLANGE(S)	ALTERNATE WIDTHS	UPLIFT WELDABILITY	HANGER OPTION PAGE(S)
			FA	CE MOUNT HAI	NGERS			
HGU	≤ 45°	7" wide			0	•	U	238
HGUM	≤ 45°	7" wide			•	•	U	238
HGUS	≤ 45°	0					U	234
HHGU					•	•	U	238
HHUS	≤ 45°		≤ 45°	•			U	234
HSUL/HSUR	45° Std.	•			0		U	_
HSULC/HSURC	45° Std.	•			Std.		U	_
HTU	≤ 67½°	•					U	234
HU ³	≤ 67½°	•	≤ 45°	•	0	0	U, W	233
HUC	≤ 45°	•	≤ 45°		Std.		U, W	233
HUCQ					Std.		U	_
HUSC					Std.		U	_
IUS							U	_
LGU	≤ 45°	•			•	•	U	238
LGUM	≤ 45°	•					U	238
LSU/LSSU	Field skewable	and slopeable to	o 45° available fo	r some models			U	_
LTHJA							U	_
LU							U	_
LUC					Std.		U	_
LUS							U	
MGU	≤ 45°	•			0	•	U	238
MIU							U	_
MTHM							U	
SUL/SUR	45° Std.	•					U	_
SULC/SURC	45° Std.	•			Std.		U	_
THGB/THGBH/ THGBV/THGBHV	≤ 45°	7" wide					U	239
THGQH	45°	•					U	239
THJA							U	_
THJU						•	U	234
U ³	≤ 67½°	•	≤ 45°	•			U	233

^{1.} Refer to the specific product pages for uplift, nailer, and weld information.

^{2.} Refer to the listed pages for each model series for restrictions, required load reductions, and

additional information regarding the hanger modifications.

3. See page 233 for limitations on skew angles for U/HU hangers.

TOP-FLANGE HANGER OPTION MATRIX



				Н	ANGER N	ODIFICA	TION OP	TIONS					APPLICATIONS	
	SKEWED	SEAT											<u>U</u> PLIFT	
BASE MODEL SERIES	SKEW	SQUARE CUT JOIST ALLOWED	SLOPED SEAT	SKEWED & SLOPED SEAT	CONCEALED FLANGE(S)	ALTERNATE WIDTHS	SLOPED TOP FLANGE	OPEN TOP FLANGE	CLOSED TOP FLANGE	OFFSET TOP FLANGE	SADDLE HANGER	RIDGE HANGER	MAILERS WELDABILITY	HANGER OPTION PAGE(S)
	Skewable	Butt Cut	Slopeable	Slopeable & Skewable	Concealed	ALTE	Sloped Top Flange	Open Top Flange	Closed Top Flange	Offset Top Flange	Saddle Hanger	Ridge Hanger	Nailer Weldable	HAI
					T	OP FLAN	IGE HAN	GERS						
В	≤ 45°		≤ 45°	•		•	•	•	•		•		U, N, W	235
BA													U, N, W	_
EG	≤ 45°		≤ 45°										_	237
EGQ	≤ 45°		≤ 45°										U	239
GB			≤ 45°								•		U, W	235
GLS	≤ 50°		≤ 45°	•			•			•	•		U, W	237
GLT	≤ 50°		≤ 45°	•			•			•			U, W	237
GLTV	≤ 50°		≤ 45°	•			•			•			U, N, W	237
НВ	≤ 45°		≤ 45°	•		•	•	•	•		•		U, N, W	235
HGB			≤ 45°								•		U	235
HGLS	≤ 50°		≤ 45°				•			•	•		U, W	237
HGLT	≤ 50°		≤ 45°				•			•			U, W	237
HGLTV	≤ 50°		≤ 45°				•			•			U, W	237
ННВ			≤ 45°			•					•		U, W	235
HIT													U, N	_
HUSCTF					Std.								_	_
HW/HWI	≤ 84°	0	≤ 45°	•			•	•		•	•	0	N, W	236
HWU	≤ 45°		≤ 45°	0									U, N, W	236
ITS													U, N	_
LB													U, N, W	_
LBV	≤ 45°		≤ 45°	•		•	•	•	•		•		U, N, W	235
LEG	≤ 45°	•	≤ 45°							•			_	237
MEG	≤ 45°	7" wide	≤ 45°							•			_	237
MIT													U, N	_
MSC	20°-45° O	•	≤ 45°	•		0							_	_
THA					0								U, N	_
THAC					Std.								U, N	_
THAI													N	_
THAR/L	45° Std.	•											U, N	_
THASR/L	22°- <mark>75°</mark> Field Skewable	•											U	_
W/WI	≤ 84°	0	≤ 45°	•			•	•	•	•	•	0	N, W	236
WM/WMI	≤ 45°		≤ 45°	•						•			_	233
WNP/WP/WPI	≤ 84°	0	≤ 45°	•			•	•	•	•	•	0	N, W	236
WPU/WNPU	≤ 45° o		≤ 45°	0									U, N, W	236

See foonotes on page 230.

SIMPSON Strong-Tie

HANGER MODIFICATION OPTIONS AND APPLICATIONS

The Hanger Options Matrix for Face Mount and Top Flange Hangers on pages 230-231 shows hanger modifications and special applications (*uplift, nailers and weldability*) that are available for each model series. Modifications may not be available for all models in the series, and some combinations of hanger options are not available. Many hanger modifications result in load reductions. For all modifications, refer to the listed hanger option pages for additional information regarding the availability of each modification, associated load reductions, and installation requirements. For more information regarding the applications, refer to the individual product pages throughout the catalogue.

HANGER OPTIONS GENERAL NOTES

This information applies only to the hangers manufactured by Simpson Strong-Tie and installed per our instructions. Some combinations of these options on a single hanger have not been evaluated. In some cases, combinations of these options cannot be manufactured. A qualified Designer must always evaluate each connection, including header and joist limitations, before specifying the product.

Testing is performed using a standardized hanger test method. The joist in the test setup may include the minimum amount of structural stability where appropriate. For example, the sloped down hanger tests are assembled with a joist cut on the lower end to lie flush with a wood member attached with three 8d common toenails. Header and other attached structural members are assumed fixed in actual installations. Horizontal loads induced by sloped joists must be resisted by other members in the structural system.

MATERIAL: Gauge may vary from that specified depending on the manufacturing process used. U, HU, HUTF, W and B hangers normally have single-piece stirrups; occasionally, the seat may be welded. Hanger configurations, height and fastener schedules may vary from the tables depending on the joist size, skew and slope.

FINISH: See specific hanger tables. Welded specials: Simpson Strong-Tie® gray paint.

Specials that are not galvanized before fabrication can be hot-dip galvanized after fabrication; specify HDG.

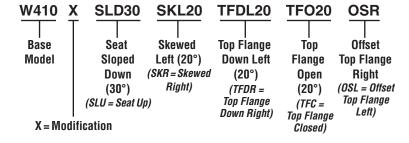
CODES: Modified hangers, due to their numerous variations, are not on code reports.

RESISTANCES: For multiple modifications on the same connector, use the single multiplier factor that yields the lowest factored resistance.

TO ORDER: Use the abbreviations below to order specials. The example shows a W410 hanger and illustrates most available options; most special hangers have only a few of these features. For assistance, contact Simpson Strong-Tie.

INSTALLATION:

- Fastener quantities may be increased beyond the amount specified in the standard hanger table.
- Fill all holes with the table-specified fastener types.
- Some skewed hangers require bevel cut joists; refer to the specific notes provided for each product.



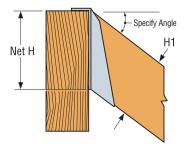
HEIGHT FOR SLOPED HANGERS

Height 1 (H1) is the joist height before the slope cut has been made.

Net Height (Net H) is the joist height after the slope cut has been made.

Provide **H1** when ordering a connector. Connectors are made assuming dry lumber is being used in continuously dry conditions.

Simpson Strong-Tie will calculate the **Net H** dimension based on the mathematical formula of H1/cos angle.



HANGER OPTIONS

U/HU

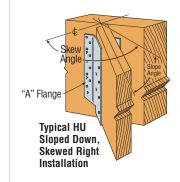
See Hanger Options General Notes. Not all slope and skew combinations are available.

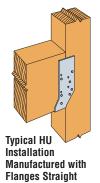
SLOPED, SKEWED, AND SLOPED/SKEWED

- For low-cost, 45° skews, see SUR/SUL and HSUR/HSUL. See also LSU/LSSU connectors.
- These options only apply to wood-to-wood connections.
- U/HU may be skewed to a maximum of 45° and sloped to a maximum of 45°. Hangers 5½" or less in width may be skewed to 671/2°. Hangers skewed 51°- 67½° require a square cut.
- HU1.81/5 can be skewed to a maximum of 50°.
- For all options, uplift resistances are 0.75 of
- For skew only or combined slopes and skews, the factored down resistance is 65% of the table value.
- For slope only, the factored down resistance is 100% of the table value.
- · Skewed hangers may have joist nails on one side.

STRAIGHT OR CONCEALED FLANGE

- HU is available with the A flanges straight at 0.70 of the table values if W $\geq 3\frac{1}{2}$ ". If W < 3", use N10 nails at 0.50 of the table value. If $W \ge 3$ ", use 10d nails at 0.50 of the table value. Do not use with end grain
- · HU is available with A flanges concealed, provided the W dimension is $25\!\!/_{16}{}^{\!\!\!\!\text{\tiny IB}}$ or greater, at 100% of the table value. Specify HUC.
- HU is available with one flange concealed when the W dimension is less than 25/16" at 100% of the table value
- For skewed only HUC hangers, the flange on the acute side can be concealed at 0.65 of the table value. See table for skew limitations.
- For sloped only hangers, flanges can be concealed at 100% of the table value.
- For sloped and skewed hangers, the flange on the acute side can be concealed at 0.65 of the table value. Contact Simpson Strong-Tie for skew limitations.
- When nailing into the carrying member's end grain, the factored resistance is 0.67 of the table value for an unmodified product or 0.67 of the reduced capacity for a modified product.
- For welding see technical bulletin T-HUHUC-W for details.





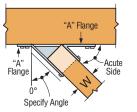


Installed on a Beam

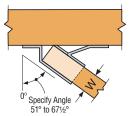
Maximum Skew Degree for Skewed HUC Hangers

Hanger Width	Maximum Skew
25/16"	26°
23/8"	26°
29/16"	29°
2¾"	29°
31/8"	37°
31/4"	38°
35/16"	39°
39/16"	42°
41/8"	42°
49/32"	42°

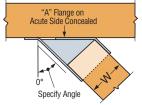
1. Widths greater than 41/32" maximum skew is 45°.



Top View U Hanger Skewed Right < 51° (Square Cut)



Top View U Hanger Skewed Right ≥ 51° (Square Cut)



Top View HUC Concealed Hanger Skewed Right (Square Cut)

WM/WMI/WMU (Midwall Installation Only)

See Hanger Options General Notes.

INSTALLATION: • Bevel-cut the joist for skewed hangers (see illustration).

· For hanger heights exceeding the joist height, the factored resistance is 0.50 of the table value.

SLOPED AND/OR SKEWED SEAT

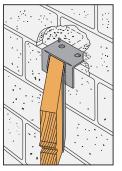
- WM/WMI may be skewed and/or sloped to 45° maximum.
- The factored resistance is 100% of the table value.

OFFSET TOP FLANGE

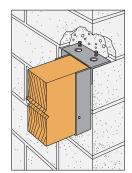
• The top flange may be offset left or right for placement at the end of a header. The factored resistance is 0.50 of the table value.

UPLIFT (WMU Only)

· WMU cannot be modified.



Typical WM Sloped Down, **Skewed Right Block Wall Installation**



Typical WM Top Flange Offset Left

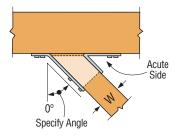


See Hanger Options General Notes. **SLOPED AND/OR SKEWED SEAT**

- - HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°.
 - HHUS skew only, maximum factored down resistance is 0.85 of the table value.
 - For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 for HHUS.
 - Not all slope and skew combinations are available; consult the factory for information.
 - HHUS, the joist must be bevel-cut to allow for double shear nailing.
 - Uplift resistances for sloped/skewed conditions are 0.62 of the table value for HHUS hangers.

HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

Models	Down Load	Uplift
W < 2" bevel or square	0.62 of table values	0.46 of table values
2" < W < 6" beveled	0.67 of table values	0.41 of table values
2" < W < 6" square cut	0.46 of table values	0.41 of table values
W > 6" bevel or square	0.40 of table values	0.41 of table values

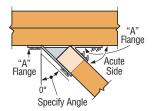


Top View HHUS Hanger Skewed Right (joist must be bevel cut) All joist nails installed on the outside angle (non-acute side).

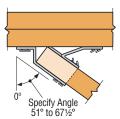
HTU

See Hanger Options General Notes. **SKEWED SEAT**

- Skewable up to 671/2°.
- Available in single and 2-ply size.
- No bevel cut required.



Top View HTU Hanger Skewed Right < 51°



Top View HTU Hanger Skewed Right ≥ 51°

Factored Resistances for Skewed HTU Hangers

		Fas	teners		Factored F	Resistance	
	_			D.F	ir-L	S-I	P-F
Model	Skew Angle			Uplift	Normal	Uplift	Normal
No.	(Degrees)	Header	Joist	(K _D =1.15)	$(K_D=1.00)$	(K _D =1.15)	(K _D =1.00)
	(===			lbs	lbs	lbs	lbs
				kN	kN	kN	kN
	< 51	20-16d	14-10dx1½	1835	4110	1300	2905
HTU26	< 31	20-10u	14-10ux 1 72	8.16	18.28	5.78	12.92
птого	51-67½	20-16d	12-10dx1½	1350	3620	955	2560
	31-07 /2	20-10u	12-10UX 1 72	6.01	16.10	4.25	11.39
	< 51	26-16d	20-10dx1½	2810	4270	1985	3030
HTU28	< 31	20-10u	20-100X172	12.50	18.99	8.83	13.48
птиго	51-67½	26-16d	17-10dx1½	2075	3930	1465	2780
	31-07 72	20-10u	17-10UX 1 72	9.23	17.48	6.52	12.37
	< 51	32-16d	26-10dx1½	3785	4430	2675	3135
HTU210	< 31	32-10u	20-10ux172	16.84	19.71	11.90	13.95
1110210	51-67½	32-16d	22-10dx1½	2795	4240	1980	3000
	31-07 72	32-10u	22-10ux172	12.43	18.86	8.81	13.35
	< 51	20-16d	14-10d	2140	3715	1515	2625
HTU26-2	< 31	20-10u	14-10u	9.52	16.53	6.74	11.68
111020-2	51-67½	20-16d	12-10d	1610	3920	1140	2785
	31-07 /2	20-10u	12-10u	7.16	17.44	5.07	12.39
	< 51	26-16d	20-10d	3960	5425	2815	3855
HTU28-2	< 31	20-10u	20-10u	17.62	24.13	12.52	17.15
111020-2	51-67½	26-16d	17-10d	2385	5425	1695	3855
	31-07/2	20-10u	17-10u	10.61	24.13	7.54	17.15
	< 51	32-16d	26-10d	5025	6890	3570	4890
HTU210-2	()	32-10U	20-10u	22.35	30.65	15.88	21.75
1110210-2	51-67½	36-16d	22-10d	3145	6680	2225	4745
	J1-01-72	30-10d	22-10u	13.99	29.72	9.90	21.10

- 1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
- 2. Reduced heel heights are not permitted for skewed HTU's.

THJU

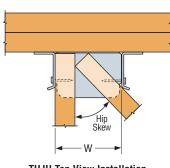
See Hanger Options General Notes.

HANGER WIDTHS

- THJU is available in intermediate seat widths between 51/8" (THJU26 width) and 81/8" (THJU26-W width).
- Factored download and uplift resistances for all intermediate widths is 100% of the THJU26-W table values.
- For double hip installation, divide the total factored resistance by 2 to determine the factored resistance for each hip.
- Order as THJU26X and specify width; see table for reference.

THJU Intermediate Width Options

	Carried Member Combination	Hip Skew	Width (W)
	2-Ply Hip and Single-Ply Jack	45-degree	63/8
	Single-Ply Hip and 2-Ply Jack	45-degree	63/4
	Double (Terminal) Hip	45-degree	7%
	2-Ply Hip and 2-Ply Jack	45-degree	THJU26-W
		44-46	THJU26
		47-49	5½
		50-52	5¾
	Cinala Dhullin	53-55	6
	Single-Ply Hip and Single-Ply Jack	56-57	6%
	and onigic 1 ly back	58-59	65/8
		60-61	7
,	TDIOOL	62-63	73/8
7		C / CF	TILLII OC M



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[HJU_Top View Installation

HANGER OPTIONS

B/LBV/HB/HHB/GB/HGB

See Hanger Options General Notes.

MATERIAL:

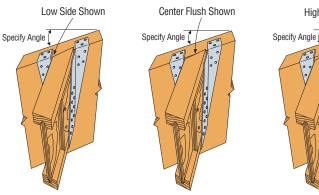
· Gauge may vary from that specified depending on the manufacturing process used. Hanger configurations, height and fastener schedules may vary from the tables depending on the joist size, skew and slope.

RESISTANCES:

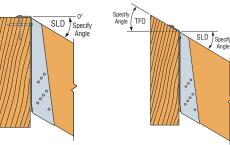
• For multiple modifications on the same connector, use the single multiplier factor that yields the lowest factored resistance.

INSTALLATION:

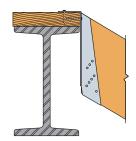
- · Fastener quantities will typically increase beyond the amount specified in the standard hanger tables.
- · Web stiffeners are required for I-joists.
- Fill all holes with the table-specified fastener types.
- · Bevel cut the carried member for skewed applications.



B Hanger Sloped Down and Skewed Left with Sloped Top Flange Installation When ordering, specify Low Side Flush, Center Flush or High Side Flush



Typical LBV Sloped Typical LBV Sloped Down Installation with Full Backing Down with Top Flange Open



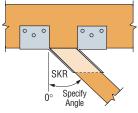
High Side Shown

Typical LBV Sloped Down on Nailer Non-Backed

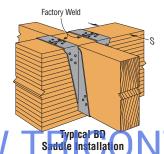
Reduction Factors for Modified Hangers¹

Hanger Series	Cond	ition	Sloped Down	Sloped Up	Skewed Only		d Down ewed	Sloped Up & Skewed		TF Down	TF Open/ Closed
061163	Angle Limit		45	45	45	4	45		45		30
	Minimum Height		6	6	6	91/4	14	91/4	14	111/4	91/4
LBV	All Widths	Download	0.98	0.68	1.00	0.97	1.00	1.00	0.68	(90-x)/90	(90-x)/90
	All Wiulis	Uplift	1.00	1.00	1.00	1.00	1.00	0.86	0.86	1.00	1.00
	Minimur	n Height	6	6	6	91/4	14	91/4	14	14	91/4
	Less than	Download	0.64	0.49	0.70	0.64	0.86	0.49	0.49	(90-x)/90	(90-x)/90
В	2½" Wide	Uplift	1.00	1.00	0.95	1.00	1.00	0.76	0.76	1.00	1.00
	2½"	Download	0.80	0.97	0.81	0.75	1.00	0.97	0.49	(90-x)/90	(90-x)/90
	and Wider	Uplift	1.00	1.00	0.95	1.00	1.00	0.76	0.76	1.00	1.00
	Minimur	n Height	8	8	8	111/4	14	111/4	14	14	111/4
	Less than	Download	0.69	0.51	0.95	0.55	0.52	0.51	0.51	(90-x)/90	(90-x)/90
НВ	2½" Wide	Uplift	1.00	1.00	0.53	0.82	1.00	0.53	0.53	1.00	1.00
	2½"	Download	0.87	0.79	0.95	0.60	1.00	0.79	0.79	(90-x)/90	(90-x)/90
	and Wider	Uplift	1.00	1.00	0.53	1.00	1.00	0.53	0.53	1.00	1.00
ННВ	Minimur	n Height	91/4	_	_	_	_	_	_	_	_
GB	All Widths	Download	0.60	_	_	_	_	_	_	_	
HGB	All WIUIIIS	Uplift	1.00	_	_	_	_	_	_	_	_

- 1. Use this table to calculate factored resistances for modified hangers. Apply the reduction factor to the appropriate factored resistance for the header condition including headers.
- 2. B and HB hangers less than 21/2" wide are assumed to use 10dx11/2 joist nails.
- 3. B and HB hangers 21/2" or wider are assumed to use 16dx21/2 or 16d common nails in the joist.
- 4. For B and HB hangers with TF Down that are less than 51/8" in width, minimum hanger height is 111/4 inches.
- 5. In the table the term "x" refers to the angle of the modification.
- 6. For top flange closed option, install upper nails slightly angled downward to avoid interference with top flange.







SADDLE HANGER

Saddle hangers are made to order; add "D" to model (e.g. BD412); specify S (for saddle) dimension. They may be used for most conditions except at end wall locations and are preferred for nailer applications. Minimum S dimension (saddle width) is 3%6". Minimum supporting member width is 3½". Minimum nailer thickness apply (see page 83 and 114). Saddle hangers achieve factored resistances listed. Saddle hangers on stud walls do not achieve factored esistances listed

SIMPSON

W/WI/WNP/WNPU/WP/WPI/WPU/HW/HWI/HWU

See Hanger Options General Notes.

INSTALLATION: • Some models are available in Type A (Bevel Cut) and Type B (Butt Cut) styles; all models are available in Type B style. Check Simpson Strong-Tie when ordering.

- · Bevel-cut the joist for skewed Type A hangers (see illustration). Square-cut the joist for Type B hangers.
- Hangers with a skew greater than 15° may have all the joist nails on the outside angle.
- Skewed HWs have face nails and require a minimum header thickness of 3½".

HANGER HEIGHT

- For hanger heights exceeding the joist height by more than ½", the factored resistance is 0.50 of the table value.
- · Minimum hanger height may increase significantly with modification. Check availability when ordering.

SLOPED AND/OR SKEWED SEAT

- Non-skewed hangers can carry the design load when the seat slope is within 2° of the joist slope. Designer must check that wood bearing is not limiting
- W/WNP/WP/HW series may be skewed to a maximum of 84° and/or sloped to a maximum of 45°. For slope only, skew only, or slope and skew combinations, the factored resistance is 100% of the table value.
- WPU/WNPU/HWU series may be skewed to a maximum of 45° and/or sloped to a maximum of 45° for joist widths less than or equal to 3%6". Multiply the table values by 0.50.

UPLIFT RESISTANCES (WPU/WNPU/HWU only)

- Hangers can be sloped to 45° and/or skewed 45° at 100% of the uplift resistance.
- Skew option is only on hangers with "W" 31/16" or less.
- Specify the slope up or down in degrees from the horizontal plane and/or the skew right or left in degrees from the perpendicular vertical plane. Specify whether low side, high side or centre of joist will be flush with the top of the header (see illustration).
- · Uplift loads are not available for open/closed TF, TF sloped, and offset options.

SLOPED TOP FLANGE

· A top flange may be ordered sloped down left or down right to 35° with or without a sloped and/or skewed seat (see illustration). Reduce tabulated factored resistances using straight-line interpolation (see open/closed top flange).

OFFSET TOP FLANGE

- The top flange may be offset left or right for placement at the end of a header (see illustration). The factored resistance is 0.50 of the table value.
- For skewed and offset top flange hangers, the maximum factored resistance is 0.42 of the table value or 2905 lbs (12.92 kN), whichever is lower.

OPEN/CLOSED TOP FLANGE

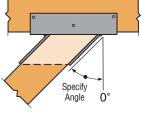
- The top flange may be opened more or closed less than the standard 90° (see illustration) to a maximum of 30°, except the HW which cannot be closed. Reduce factored resistances using straight-line interpolation.
- Example: For a top flange open 30°, reduce resistance to [(90-30)/90] x table value.

SADDLE HANGER

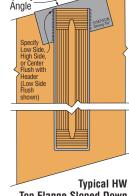
• To order, add D to model and specify S dimension (see illustration).

RIDGE HANGER (not available for uplift models)

• Top flange may be sloped to a maximum of 35° to accommodate a ridge (see illustration). Specify angle of the slope. Reduce factored resistance using straight-line interpolation. See Open/Closed example.



Typical W Top View Skewed Left Type A Hanger (Bevel Cut Joist Shown)

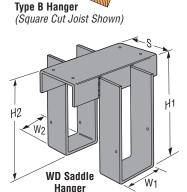


Specify

Top Flange Sloped Down Left with Low Side Flush

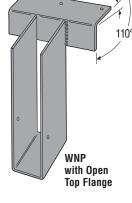
Top Flange

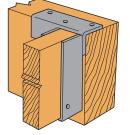
open 20°

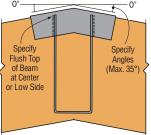


Typical W

Skewed Left

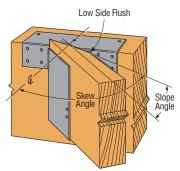


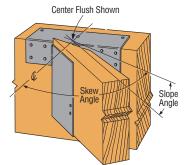


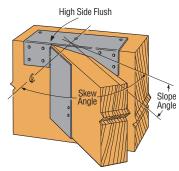


Typical W Top Flange Offset Left

Typical W Ridge Installation







rpical H.W. Sloped Down, Slewed Right with Trpe A Hanger (*Joist end must be bevel cut* When ordering, specify Low Side Flush, Center Flush or High Side Flush

HANGER OPTIONS

GLT/HGLT/GLS/HGLS/GLTV/HGLTV

See Hanger Options General Notes.

INSTALLATION: • Bevel-cut the carried beam for skewed hangers.

HANGER HEIGHT

• For hangers exceeding the joist height by 1/2", the factored resistance is 50% of the table value.

SLOPED AND/OR SKEWED SEAT

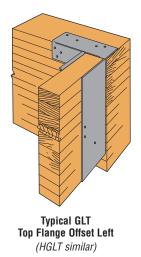
- GLT/GLTV/HGLT/HGLTV and GLS/HGLS series may be skewed to a maximum of 50° or sloped to a maximum of 45°.
- For skews greater than 15°, multiply the tabulated factored uplift resistance by 0.50
- For sloped only, multiply the table value by 0.78 for GLT/GLS/GLTV to a maximum of 8135 lbs. For HGLT/HGLS/HGLTV multiply the table value by 0.85 to a maximum of 12,605 lbs.
- For skewed only, multiply the table value by 0.87 for GLT/GLS/GLTV to a maximum of 9510 lbs. For HGLT/HGLS/HGLTV multiply the table value by 0.73 to a maximum of 10,890 lbs.
- For sloped and skewed GLT/GLS/GLTV configurations, multiply the table values by 0.78 to a maximum of 8130 lbs. Sloped and skewed combinations are not allowed for the HGLT/HGLS/HGLTV.

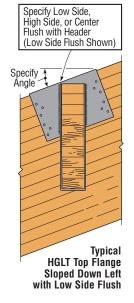
SLOPED TOP FLANGE

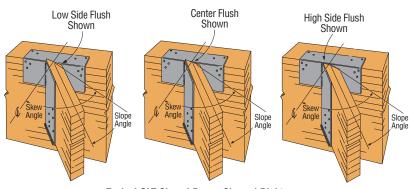
- A top flange may be sloped down left or down right to 30° with or without a sloped and/or skewed seat (see illustration). Reduce tabulated factored resistances using straight-line interpolation.
- Example: For a top flange sloped 30°, reduce resistance to [(90-30)/90] x table value.

OFFSET TOP FLANGE

- The top flange may be offset left or right for placement at the end of a header. Minimum seat width 31/4". The maximum factored resistance is 0.50 of the table value for the GLT/GLTV/GLS and 0.45 for the HGLT/HGLTV/HGLS.
- For skewed and offset top flange hangers, the maximum factored resistance is 5085 lbs.
- No uplift resistance.







Typical GLT Sloped Down, Skewed Right When ordering, specify Low Side Flush, Center Flush or High Side Flush

LEG/MEG/EG

See Hanger Options General Notes.

SKEWED SEAT - TOP FLANGE MODELS ONLY

• The LEG/MEG/EG series can be skewed up to 45°. The maximum factored resistance is 13,750 lbs D.Fir-L Glulam and 12,090 lbs Spruce-Pine Glulam for LEG and MEG, 19,710 lbs D.Fir-L Glulam and 18,005 lbs Spruce-Pine Glulam for EG.

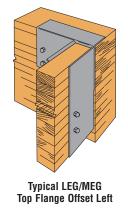
SLOPED SEAT - TOP FLANGE MODELS ONLY

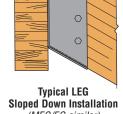
• The LEG/MEG/EG series can be sloped up to 45°. The maximum factored resistance is 15,835 lbs D.Fir-L Glulam and 13,920 lbs Spruce-Pine Glulam; see illustration.

NO SLOPED AND SKEWED COMBO AVAILABLE.

OFFSET TOP FLANGE

- The LEG/MEG (only) top flange may be offset left or right for placement at the end of a header (see illustration). The maximum factored resistance is 9,280 lbs D.Fir-L Glulam and 8,160 lbs. Spruce-Pine Glulam (Min. H = 11" for MEG, 9" for LEG)
- · No skews allowed on offset hangers.





Specify

Anale

(MEG/EG similar)

LGU/MGU/HGU/HHGU

See Hanger Options General Notes.

CONCEALED FLANGE

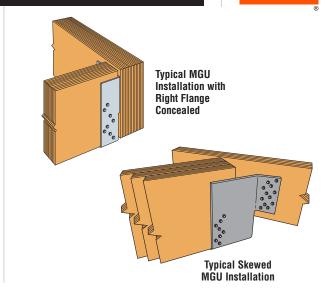
- · LGU, MGU, HGU and HHGU hangers are available with one flange concealed. Specify flange to conceal.
- · Factored resistances for one flange-concealed option:
 - LGU 0.83 of published value - HGU 0.70 of published value
 - MGU 0.65 of published value - HHGU 0.84 of published value
- MGU with W \leq 4" and HGU with W \leq 411/16" flanges cannot be concealed.

SKEWED

- LGU, MGU and HGU hangers are available skewed up to 45°.
- Concealed flanges are not available with skewed models.
- Apply the following reduction factors to table values:

Reduction Factors for Skewed LGU, MGU, HGU

Model	Beam Cut	Download	Uplift
LGU	Square Cut	0.90	0.60
LGU	Bevel Cut	0.90	0.60
MGU/HGU	Square Cut	0.75	0.65
less than 6" wide	Bevel Cut	0.80	0.65
MGU/HGU	Square Cut < 7" wide	0.75	0.55
6" and wider	Bevel Cut	0.80	0.55



3" min. to top of wall (typ.)

Typical Concealed

Flange HGUM Installation

at Outside Corner

Min. one 15M rebar top course (typ.)

LGUM/HGUM

See Hanger Options General Notes.

CONCEALED FLANGE

· HGUM hangers are available with one flange concealed. Specify flange to conceal.

Table 1 - HGUM Factored Resistances for One Flange Concealed Applications

		Dimensions Fasteners		Factored Resistance								
				газі	IGHGIS	End of Wall				Outside Corner		
Mode	Model	C			CMU/ Joist		CMU Wall		Concrete Wall		CMU or Concrete Wall	
	No.		Concrete	30121	Uplift	Normal	Uplift	Normal	Uplift	Normal		
	NU.	W	Н	H Titen HD	SDS Screws	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$	
						lbs	lbs	lbs	lbs	lbs	lbs	
					IID Sciews	SCIEWS	kN	kN	kN	kN	kN	kN
	ПСПМ	5¼ to 9	9 11 to 30	1 to 30 8-%"x5"	24-1/4"x21/2"	1690	7355	4495	9660	3880	9890	
	ndulvi					7.52	32.72	20.00	42.97	17.26	43.99	

Min. one 15M rebar

Vertical rebar

1. Factored uplift resistances shown are for D.Fir-L joist. For S-P-F joist, multiply the value x 0.72.

See Table 2 for additional notes.

Table 2 - LGUM/HGUM Factored Resistances for Skewed Applications

	Eac	teners	Factored Resistance				
	газ	tellel 2	Up	Normal			
Model	CMU/	Joist	D.Fir-L	S-P-F	Concrete/CMU		
No.	Concrete	(KD = 1.1		$(K_D = 1.15)$	$(K_D = 1.00)$		
	Titen HD	SDS Screws	lbs	lbs	lbs		
			kN	kN	kN		
LGUM26-2X			875	630	2855		
LGUM26-3X	4-%"x4"	4-1/4"x21/2"	073	000	2000		
LGUM26-4X	70 77		3.89	2.80	12.70		
LGUM46X			0.00	2.00	12.70		
LGUM28-2X	6-¾"x4"	6-1/4"x21/2"	1410	1015	4470		
LGUM28-3X			1410	1013	1110		
LGUM28-4X			6.27	4.52	19.88		
LGUM48X			0.21	7.02	10.00		
LGUM210-2X		8-1/4"x21/2"	1950	1405	6085		
LGUM210-3X	8-3/8"x4"		1000	1100	0000		
LGUM210-4X	0 78 84	0 /4 XL/2	8.67	6.25	27.07		
LGUM410X							
HGUM5.25X	8-%"x5"	8-1/4"x21/2"	2390	1720	9370		
HGUM5.50X	0 /0 /0	0 /4 XZ/2	10.63	7.65	41.68		
HGUM7.00X	8-%"x5"	8-1/4"x21/2"	2350	1690	8450		
HGUM7.25X			10.45	7.52	37.59		
HGUM9.00X	8-%"x5"	8-1/4"x21/2"	2310	1660	7530		
TIGOWIS.OUX	U 78 AU	0-74 XZ72	10.28	7.38	33.50		

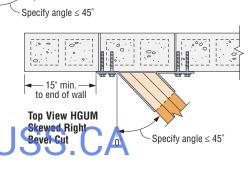
Typical Concealed Flange HGUM Installation at **Outside Corner** Typical Concealed Flange HGUM Installation at Top View LGUM 12" min. **End of Wall** to end of Skewed Right **Square Cut**

one block

Shaded cells grouted and reinforced per Designer (min.)

1. Factored uplift values have been increased 15% for wind or earthquake loading with no further increase allowed.

Factored uplift values have been increased 15% for wind of earthquake loading with no further increase allowed. Reduce where other load durations govern.
 Factored resistances assume Type S mortar with f'_m = 1087 psi (7.5 MPa) for 15 MPa concrete block masonry as per Table 4 CSA S304.1-14. For values of f'_m < 1085 psi (7.5 MPa) multiply the tabulated values by (f'_m/1085)^{6.5}.
 Factored resistances assume a 28 day concrete compressive strength of f'_C = 2500 psi (17.25 MPa). For values of f'_C < 2500 psi (17.25 MPa) multiply the tabulated values by (f'_C/2500)^{0.5}.
 Factored resistances for concrete block masonry assumes minimum 8° (190 mm) block grouted solid as per CSA A179-14. Specifier to design block wall rein orcing per CSA S304 1-11 to carry the applied load.
 Factored resistances for concrete assumes minimum 8° (203 mm) concrete vall. Specifier to design concrete wall reinforcing as per CSA A23.3° 14 to carry the applied load.



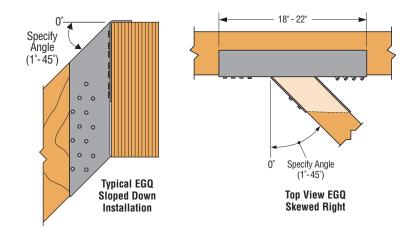
SKEWED SEAT

- The EGQ can be skewed a maximum of 45°.
- The factored down resistance is 0.69 of the table value to a maximum of 22,435 lbs (99.76 kN).
- The factored uplift resistance is 100% of the table value.

SLOPED SEAT

- The EGQ can be sloped down a maximum of 45°.
- The factored down resistance is 0.78 of the table value to a maximum of 25,160 lbs (111.92 kN)
- The factored uplift resistance is 100% of the table value.
- Sloped seat installation requires an additional 14 joist screws (supplied with the connector).

NO SLOPED AND SKEWED COMBO AVAILABLE.



THGB/THGBH/THGBV/THGBHV

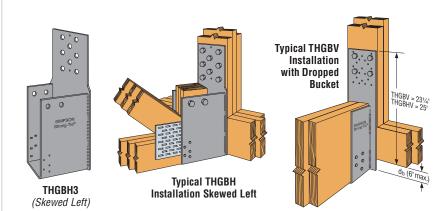
See Hanger Options General Notes.

SKEWED SEAT, 45° MAXIMUM

- Multiply the tabulated factored resistances for uplift and download by the following:
- THGB2/THGB3 0.74 - THGBH3 0.71 - THGBH4 0.56

DROPPED BUCKET

- THGBV/THGBHV backplates can be extended to allow for up to a 6" dropped bucket.
- Factored resistances are 100% of the table values.
- Order as "X" version, specify the total backplate height, BK_PLT, equal to the hanger height (H) plus the dropped bucket amount (db).
 Ex: a THGBV3.62/9 with a 4" dropped bucket would have a total backplate height of 271/4".



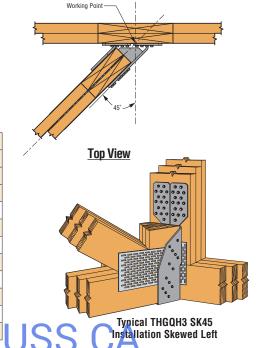
THGQH

See Hanger Options General Notes.

SKEWED SEAT

- THGQH may be skewed 45° for the models shown. Carried members may be bevel cut.
- Align centreline of joist and centreline of connecting web at the face of the girder.

				Fasteners		Factored Resistance					
		Max.	B#1	rasie	ellers	D.F	ir-L	r-L S-P			
	Model Girder		Girder Vert		Min. Vert.			Uplift	Normal	Uplift	Normal
	No.	No. Truss B.C. Web Size	B C	R C Web	Header	Handay Iniat	$(K_D = 1.15)$	$(K_D = 1.00)$	$(K_D = 1.15)$	$(K_D = 1.00)$	
			3126	пеаиег	der Joist	lbs	lbs	lbs	lbs		
						kN	kN	kN	kN		
	THGQH2 SK45	2x10	2x12	28-SDS 1/4"x3"			18-SDS	6275	15440	4520	11115
	IIIduiiz 3K43	2310	2.8.12				1/4"x3" 1/4"x3"	27.91	68.68	20.11	49.44
	THGQH3 SK45	2x10	2x12	36-SDS ½"x4½"		18-SDS	5345	15440	3845	11115	
	IIIdulio okao	2310	2.8.1.2			1/4"x41/2"	1/4"x41/2" 1/4"x41/2	1/4"x41/2"	23.78	68.68	17.10
	THGQH4 SK45	2x12	2×12	40-SDS	40-SDS 18-SDS	5345	20310	3845	14625		
	THUWHA SKAD	2.8.1.2			1/4"x6"	1/4"x6"	1⁄4"x6"	23.78	90.35	17.10	65.06



Hanger Options

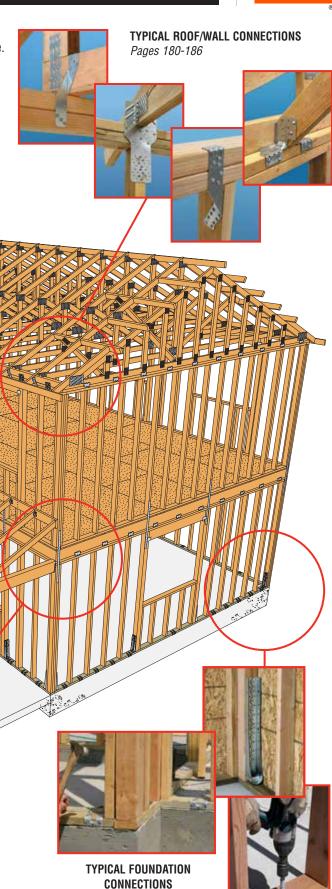
CONTINUOUS LOAD TRANSFER PATH

Catalogue C-C-CAN2015 © 2015 SIMPSON STRONG-TIE COMPANY INC

This drawing shows the connection points for a continuous load transfer path from the rafters to the foundation of a two-story house.

Building with a continuous load path is an essential part of creating a structure better able to withstand the forces of mother nature.

This drawing is for illustrative purposes only and should not be considered an engineered system. Refer to the page numbers for the full range of Simpson Strong-Tie® connectors. Consult a qualified Designer to ensure that correct connector quantities and installation methods are used to achieve the full design values.



SYSTEMSPages 74-76

LATERAL

TYPICAL
FLOOR-TO-FLOOR
CONNECTIONS
Pages 187-194

TRICONTRUSS.CA

Pages 28-56

VALUE-ADDED SOFTWARE

Simpson Strong-Tie Apps Designed to **Help You Do Your Job Faster and Easier**

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Need to find out how many Simpson Strong-Tie® adhesive cartridges you require for a job? Perhaps you're looking for some literature to learn more about a new product? Simpson Strong-Tie now delivers exciting mobile apps to your iPhone™, iPad™ or Android™ mobile device. Simply download and easily access information about our most popular products and even find the closest Simpson Strong-Tie dealer to purchase them.

Match Your Anchor with the Right Connector

Our Connector-Anchor selector connector helps discern the required embedment depth of our anchoring products when used with a Simpson Strong-Tie holdown or post base product.

Selecting the Right Holdown

The Holdown Selector is a quick and easy tool that selects the most cost-effective holdown connector based on the type of installation, demand load and wood species.

Here's Just Some of Many Other Software Solutions **Designed to Help You Save Time**

- Adhesive Cartridge Estimator
- AHEP Design Calculator
- Code Report Finder
- Coil Strap Calculator
- Connector-Anchor Selector
- Dealer Locator
- Rod Elongation Calculator

- Slope & Skew Calculator
- Steel Deck Diaphragm Calculator
- Anchor Designer Software
 Strong Frame[®] Moment Frame Selector
 - Strong-Wall[®] Selector
 - Wall-Bracing-Length Calculator
 - Wood Shrinkage Calculator





NOTES

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